



Estimating Near-Bank Stress ( NBS )									
Stream: <b>Eccleston</b>					Location: <b>Right Bank 10</b>				
Station: <b>119+64.19 to 121+21.11</b>			Stream Type:		Valley Type:			Date: <b>5/4/2018</b>	
Observers:					Date: <b>5/4/2018</b>				
Methods for Estimating Near-Bank Stress (NBS)									
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme							
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)				
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)				
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)				
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)				
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)					
Converting Values to a Near-Bank Stress (NBS) Rating									
Near-Bank Stress (NBS) ratings	Method number								
	(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50		
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00		
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60		
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00		
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40		
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40		
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Moderate</b>			

Method	5
Dominant	
Near-Bank Stress	
Moderate	



Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Left Bank 10</b>											
Station: <b>120+25.66 to 121+23.56</b>		Stream Type:			Valley Type:								
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td>Dominant Near-Bank Stress</td><td></td></tr> <tr><td>Moderate</td><td></td></tr> </table>		Method	5	Dominant Near-Bank Stress		Moderate	
	Method	5											
	Dominant Near-Bank Stress												
Moderate													
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)									
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)									
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		0.67	0.44	1.5227273	Moderate								
<b>Level III</b>	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
<b>Overall Near-Bank Stress (NBS) rating</b>					<b>Moderate</b>								



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Right Bank 11</b>										
Station: <b>116+39.48 to 119+64.19</b>			Stream Type:		Valley Type:			Date: <b>5/4/2018</b>							
Observers:					Date: <b>5/4/2018</b>										
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.56</b>	<b>0.39</b>	<b>1.4358974</b>	<b>Low</b>										
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)						
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 11</b>						
Station: <b>114+61.16 to 120+25.66</b>		Stream Type:		Valley Type:				
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

Method	5
Dominant Near-Bank Stress	
Low	





Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Right Bank 12</b>											
Station: <b>115+14.22 to 116+39.48</b>		Stream Type:			Valley Type:								
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
		<table border="1"> <tr> <td>Method</td> <td>5</td> </tr> <tr> <td colspan="2">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2">Low</td> </tr> </table>						Method	5	Dominant Near-Bank Stress		Low	
Method	5												
Dominant Near-Bank Stress													
Low													
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		0.56	0.39	1.4358974	Low								
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						Low							



Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Left Bank 12</b>											
Station: <b>111+99.76 to 114+61.16</b>		Stream Type:			Valley Type:								
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		<b>0.41</b>	<b>3.97</b>	<b>0.1032746</b>	<b>Extreme</b>								
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		<table border="1" style="margin-left: auto; margin-right: auto;"> <tr><td>Method</td><td>2</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Extreme</td></tr> </table>						Method	2	Dominant Near-Bank Stress		Extreme	
Method	2												
Dominant Near-Bank Stress													
Extreme													
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)									
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)					
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
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Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Extreme</b>							



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 13</b>						
Station: <b>113+44.63 to 115+14.22</b>		Stream Type:			Valley Type:			
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
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(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>					<b>Moderate</b>			

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Moderate</b>	



Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Left Bank 13</b>											
Station: <b>110+41.23 to 111+99.76</b>		Stream Type:		Valley Type:									
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
		<table border="1"> <tr> <td>Method</td> <td>5</td> </tr> <tr> <td colspan="2">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2">Moderate</td> </tr> </table>						Method	5	Dominant Near-Bank Stress		Moderate	
Method	5												
Dominant Near-Bank Stress													
Moderate													
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		<b>0.73</b>	<b>0.48</b>	<b>1.5208333</b>	<b>Moderate</b>								
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Near-Bank Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						<b>Moderate</b>							





Estimating Near-Bank Stress ( NBS )									
Stream: <b>Eccleston</b>					Location: <b>Right Bank 14</b>				
Station: <b>112+84.76 to 113+44.63</b>			Stream Type:			Valley Type:			
Observers:					Date: <b>5/4/2018</b>				
Methods for Estimating Near-Bank Stress (NBS)									
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme							
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)				
		<b>0.21</b>	<b>4.93</b>	<b>0.0425963</b>	<b>Extreme</b>				
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)				
<b>Level III</b>	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)				
	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)				
<b>Level IV</b>	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)
	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating									
Near-Bank Stress (NBS) ratings	Method number								
	(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50		
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00		
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60		
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00		
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40		
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40		
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Extreme</b>			

Method	<b>2</b>
<b>Dominant</b>	
<b>Near-Bank Stress</b>	
<b>Extreme</b>	



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 14</b>						
Station: <b>108+67.52 to 110+41.23</b>		Stream Type:		Valley Type:				
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Moderate</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Moderate</b>	



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 15</b>						
Station: <b>112+3.47 to 112+84.76</b>		Stream Type:			Valley Type:			
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Moderate</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Moderate</b>	



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 15</b>						
Station: <b>107+61.02 to 108+67.52</b>		Stream Type:		Valley Type:				
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

Method	5
Dominant Near-Bank Stress	
Low	





Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Right Bank 16</b>											
Station: <b>110+65.05 to 112+3.47</b>		Stream Type:		Valley Type:									
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
		<table border="1"> <tr> <td>Method</td> <td>5</td> </tr> <tr> <td colspan="2">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2">Moderate</td> </tr> </table>						Method	5	Dominant Near-Bank Stress		Moderate	
Method	5												
Dominant Near-Bank Stress													
Moderate													
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		<b>0.45</b>	<b>0.25</b>	<b>1.8</b>	<b>Moderate</b>								
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						<b>Moderate</b>							



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Left Bank 16</b>										
Station: <b>106+23.76 to 107+61.02</b>			Stream Type:		Valley Type:										
Observers:					Date: <b>5/4/2018</b>										
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.89</b>	<b>0.65</b>	<b>1.3692308</b>	<b>Low</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									



Estimating Near-Bank Stress ( NBS )														
Stream: <b>Eccleston</b>		Location: <b>Right Bank 17</b>												
Station: <b>107+61.59 to 110+65.05</b>		Stream Type:			Valley Type:									
Observers:		Date: <b>5/4/2018</b>												
Methods for Estimating Near-Bank Stress (NBS)														
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme												
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			<table border="1"> <tr><td>Method</td><td>1</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Very High</td></tr> </table>	Method	1	Dominant Near-Bank Stress		Very High	
	Method	1												
	Dominant Near-Bank Stress													
Very High														
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)										
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)										
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)									
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)					
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating														
Near-Bank Stress (NBS) ratings	Method number													
	(1)	(2)	(3)	(4)	(5)	(6)	(7)							
<b>Very Low</b>	N / A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50							
<b>Low</b>	N / A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00							
<b>Moderate</b>	N / A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60							
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00							
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40							
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40							
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Very High</b>								



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 17</b>						
Station: <b>102+41.73 to 106+23.76</b>		Stream Type:		Valley Type:				
Observers:		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N / A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N / A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N / A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>High</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>High</b>	





Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Right Bank 18</b>											
Station: <b>106+3.93 to 107+61.59</b>		Stream Type:		Valley Type:									
Observers:		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
		<table border="1"> <tr> <td>Method</td> <td>5</td> </tr> <tr> <td colspan="2">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2">Low</td> </tr> </table>						Method	5	Dominant Near-Bank Stress		Low	
Method	5												
Dominant Near-Bank Stress													
Low													
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		0.75	0.64	1.171875	Low								
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						Low							



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 18</b>						
Station: <b>202+64.02 to 205+15.89</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Low</b>	



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 19</b>						
Station: <b>102+40.18 to 106+3.93</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

Method	5
Dominant Near-Bank Stress	
Low	



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 19</b>						
Station: <b>205+15.89 to 205+62.34</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Near-Bank Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Low</b>	



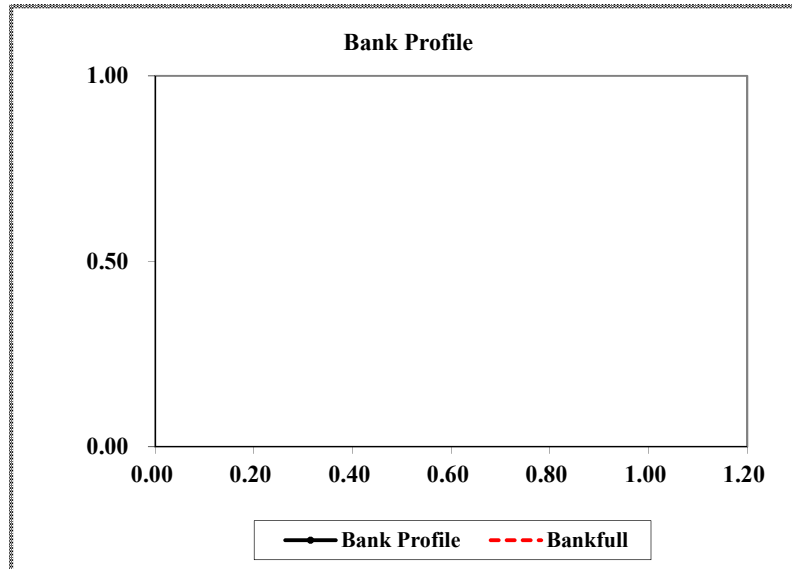
### BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	41.36					
Reach:	202+64.58 to 205+11.94	Comments:						Very High						
Location:	Right Bank 20	Bank Length	244					Total Score	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018						Values:	5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
3.00	1.00	3.00	10.00	Extreme	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.25	3.00	0.08	8.63	Very High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
30.00	0.08	2.50	10.00	Extreme	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
50.00			3.41	Low	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
50.00			4.32	Moderate	
			Adjustment		Notes
<b>Bank Materials</b>			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>			0.00		
<b>TOTAL SCORE</b>			<b>41.36</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Bank Material	Adjustments						
		<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
<b>Boulders</b>		Boulder banks have a low erosion potential.						
<b>Cobble</b>		Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
<b>Clay/Silt Loam</b>		Add 5 points.						
<b>Gravel</b>		Add 5-10 points depending on percentage of bank material composed of sand.						
<b>Sand</b>		Add 10 points.						
<b>Silt / Clay</b>		No adjustment.						
Stratification								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



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Estimating Near-Bank Stress ( NBS )									
Stream: <b>Eccleston</b>		Location: <b>Right Bank 20</b>							
Station: <b>202+64.58 to 205+11.94</b>		Stream Type:		Valley Type:					
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>							
Methods for Estimating Near-Bank Stress (NBS)									
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance				
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction				
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction				
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction				
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction				
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction				
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation				
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High							
		Extensive deposition (continuous, cross-channel).....NBS = Extreme							
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme							
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)				
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)				
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)				
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)				
		<b>0.64</b>	<b>0.2</b>	<b>3.2</b>	<b>Extreme</b>				
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)					
Converting Values to a Near-Bank Stress (NBS) Rating									
Near-Bank Stress (NBS) ratings	Method number								
	(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50		
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00		
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60		
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00		
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40		
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40		
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Extreme</b>			

Method	5
Dominant Near-Bank Stress	
Extreme	

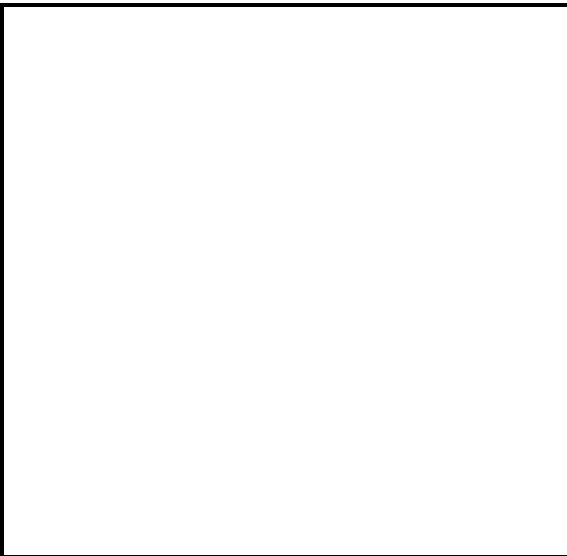
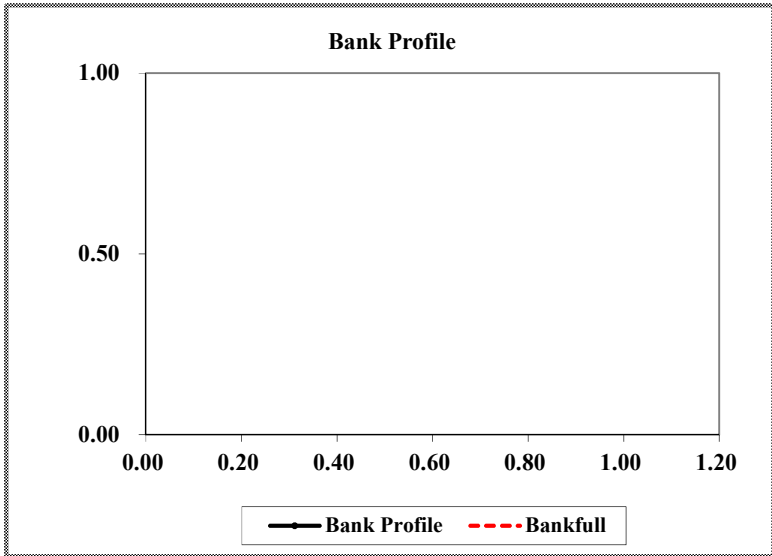
**BANK EROSION HAZARD INDEX**

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	37.77						
Reach:	404+4.57 to 405+60.53	Comments:								High					
Location:	Left Bank 20	Bank Length	155					Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme	
Date:	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
0.50	0.50	1.00	1.00	Very Low	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.42	0.50	0.84	2.24	Low	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
10.00	0.84	8.40	8.62	Very High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
90.00			7.90	High	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
15.00			8.00	Very High	
			Adjustment		Notes
<b>Bank Materials</b>					
			10.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>37.77</b>		

Bank Erosion Potential							
		Very Low	Low	Moderate	High	Very High	Extreme
<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Adjustments							
<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
<b>Boulders</b>	Boulder banks have a low erosion potential.						
<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
<b>Clay/Silt Loam</b>	Add 5 points.						
<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.						
Stratification							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Left Bank 20</b>										
Station: <b>404+4.57 to 405+60.53</b>			Stream Type:			Valley Type:									
Observers: <b>PVC &amp; SH</b>					Date: <b>5/4/2018</b>										
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope S	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.72</b>	<b>0.55</b>	<b>1.3090909</b>	<b>Low</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope S	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									

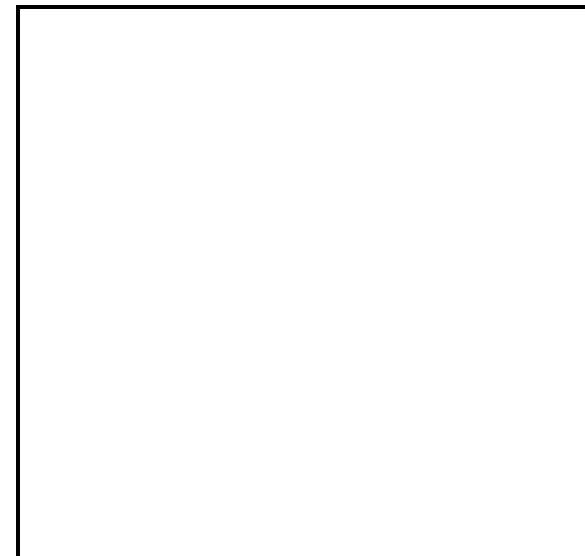
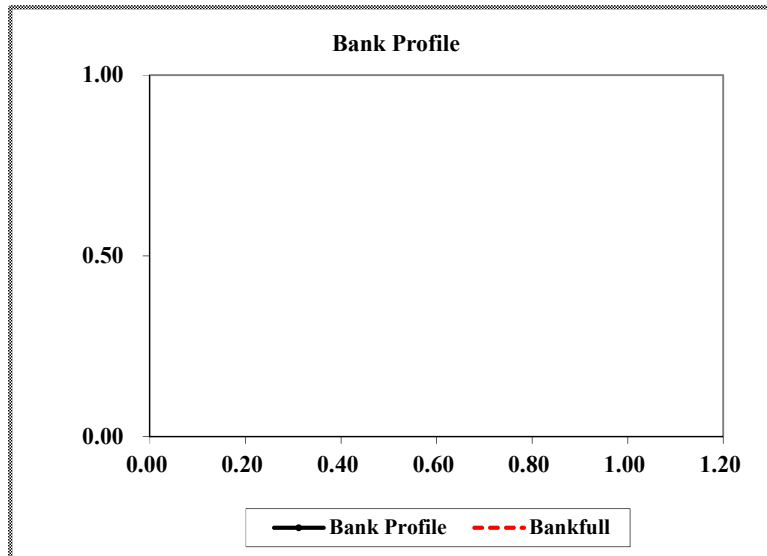
**BANK EROSION HAZARD INDEX**

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	41.23					
Reach:	205+11.94 to 205+58.48	Comments:							Very High					
Location:	Right Bank 21	Bank Length	47					Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.00	1.00	2.00	7.90	High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.17	2.00	0.09	8.61	Very High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
30.00	0.09	2.55	10.00	Extreme	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
75.00			5.40	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
50.00			4.32	Moderate	
			Adjustment		Notes
<b>Bank Materials</b>					
			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>41.23</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Adjustments</b>							
	Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
		<b>Boulders</b>	Boulder banks have a low erosion potential.					
<b>Cobble</b>		Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
<b>Clay/Silt Loam</b>		Add 5 points.						
<b>Gravel</b>		Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
	<b>Silt / Clay</b>	No adjustment.						
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )			
Stream: <b>Eccleston</b>	Location: <b>Right Bank 21</b>		
Station: <b>205+11.94 to 205+58.48</b>	Stream Type:	Valley Type:	
Observers: <b>PVC &amp; SH</b>	Date: <b>5/4/2018</b>		

Methods for Estimating Near-Bank Stress (NBS)		
(1) Channel pattern, transverse bar or split channel/central bar creating NBS	Level I	Reconnaissance
(2) Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )	Level II	General prediction
(3) Ratio of pool slope to average water surface slope ( $S_p / S$ )	Level II	General prediction
(4) Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )	Level II	General prediction
(5) Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )	Level III	Detailed prediction
(6) Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )	Level III	Detailed prediction
(7) Velocity profiles / Isovels / Velocity gradient	Level IV	Validation

**Level I** (1) Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High  
 Extensive deposition (continuous, cross-channel).....NBS = Extreme  
 Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme

<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Extreme</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Extreme	
	Method	5													
	Dominant Near-Bank Stress														
Extreme															
(3)	Pool Slope $S_p$	Average Slope S	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)	0.64	0.2	3.2	Extreme						
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope S	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)						
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)											

Converting Values to a Near-Bank Stress (NBS) Rating							
Near-Bank Stress (NBS) ratings	Method number						
	(1)	(2)	(3)	(4)	(5)	(6)	(7)
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Extreme</b>	

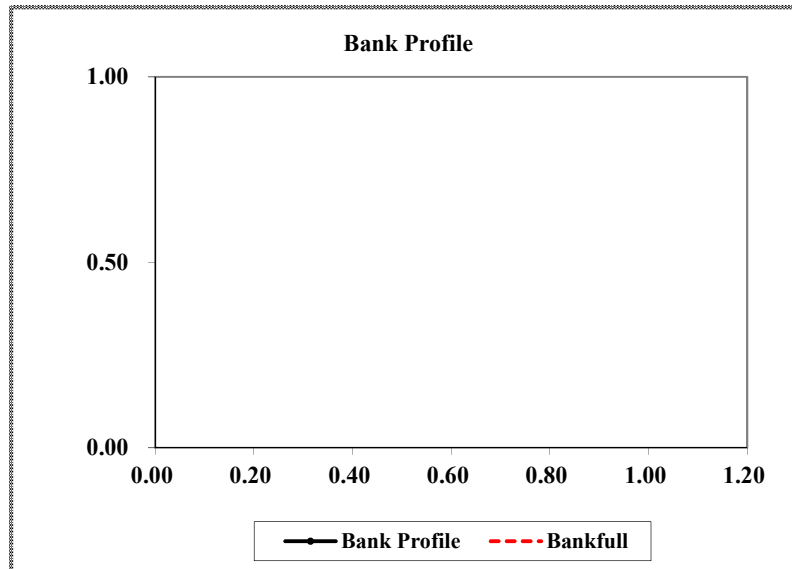
# BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:	Total Score: <b>36.05</b>							
Reach:	405+60.53 to 410+65.09	Comments:						<b>High</b>						
Location:	Left Bank 21	Bank Length	517					Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.80	1.00	2.80	9.00	Very High	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.80	0.21	7.12	High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
50.00	0.21	10.36	8.40	Very High	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
50.00			3.41	Low	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
65.00			3.11	Low	
			Adjustment		Notes
			5.00		
Bank Materials					
			Adjustment		Notes
			0.00		
Bank Stratification					
			TOTAL SCORE	<b>36.05</b>	

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
Adjustments								
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
	<b>Boulders</b>	Boulder banks have a low erosion potential.						
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	<b>Clay/Silt Loam</b>	Add 5 points.						
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
Stratification								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes




Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Left Bank 21</b>										
Station: <b>405+60.53 to 410+65.09</b>			Stream Type:		Valley Type:										
Observers: <b>PVC &amp; SH</b>			Date: <b>5/4/2018</b>												
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.72</b>	<b>0.55</b>	<b>1.3090909</b>	<b>Low</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									



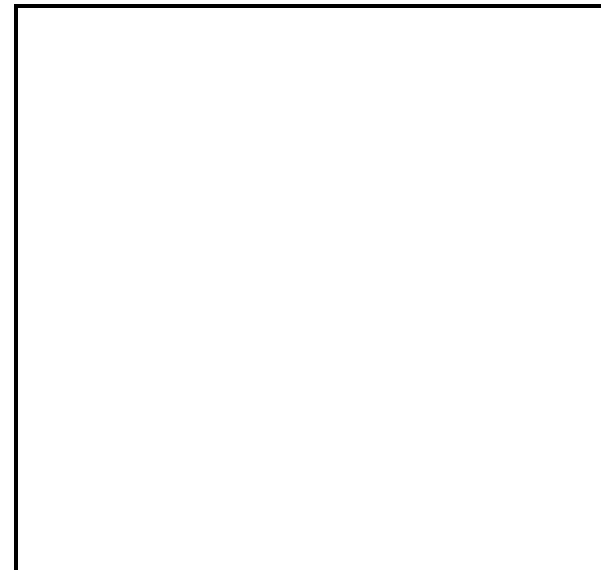
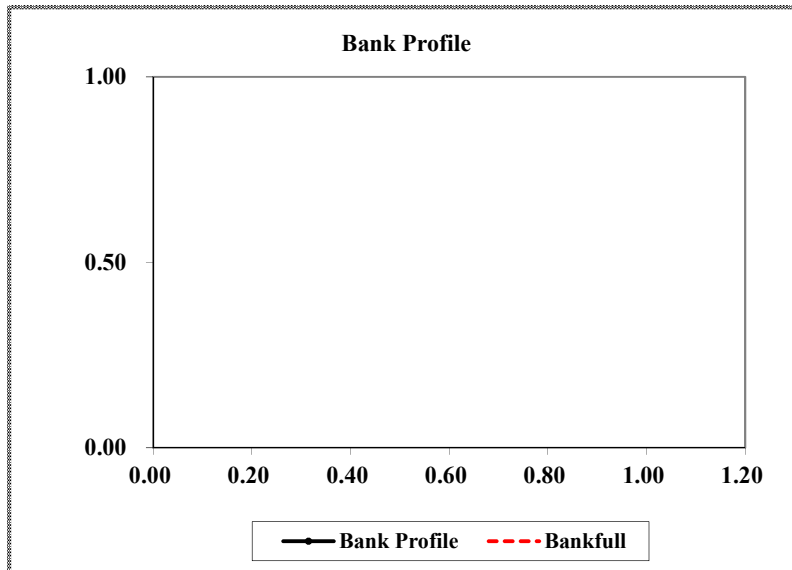
**BANK EROSION HAZARD INDEX**

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	37.77						
Reach:	404+7.41 to 405+56.34	Comments:							High						
Location:	Right Bank 22	Bank Length:	147						Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
0.50	0.50	1.00	1.00	Very Low	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.42	0.50	0.84	2.24	Low	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
10.00	0.84	8.40	8.62	Very High	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
90.00			7.90	High	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
15.00			8.00	Very High	
			Adjustment		Notes
			10.00		
Bank Materials					
			Adjustment		Notes
Bank Stratification					
			0.00		
<b>TOTAL SCORE</b>			<b>37.77</b>		

Bank Erosion Potential									
		Very Low	Low	Moderate	High	Very High	Extreme		
Erodibility Variables	Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Adjustments								
	Bank Material	Bedrock	Bedrock banks have a very low erosion potential.						
		Boulders	Boulder banks have a low erosion potential.						
		Cobble	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
		Clay/Silt Loam	Add 5 points.						
		Gravel	Add 5-10 points depending on percentage of bank material composed of sand.						
		Sand	Add 10 points.						
	Silt / Clay	No adjustment.							
	Stratification								
	Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Right Bank 22</b>										
Station: <b>404+7.41 to 405+56.34</b>			Stream Type:			Valley Type:									
Observers: <b>PVC &amp; SH</b>			Date: <b>5/4/2018</b>												
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Extreme</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Extreme	
	Method	5													
	Dominant Near-Bank Stress														
Extreme															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.64</b>	<b>0.2</b>	<b>3.2</b>	<b>Extreme</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Extreme</b>									

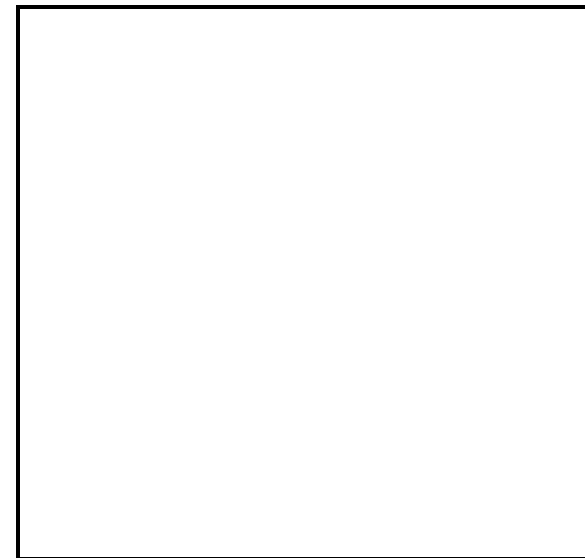
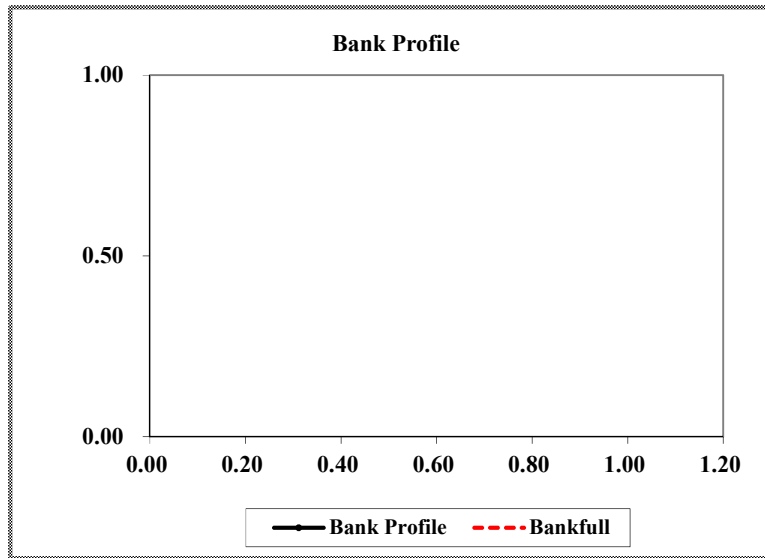
### BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	46.13						
Reach:	309+20.39 to 311+1.38	Comments:							Extreme						
Location:	Left Bank 22	Bank Length:	178						Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.42	2.50	0.17	7.66	High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
20.00	0.17	3.36	10.00	Extreme	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
80.00			5.90	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
10.00			9.00	Very High	
			Adjustment		Notes
<b>Bank Materials</b>					
			0.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			5.00		
<b>TOTAL SCORE</b>			<b>46.13</b>		

Bank Erosion Potential							
		Very Low	Low	Moderate	High	Very High	Extreme
<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Adjustments							
<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
<b>Boulders</b>	Boulder banks have a low erosion potential.						
<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
<b>Clay/Silt Loam</b>	Add 5 points.						
<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.						
Stratification							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 22</b>						
Station: <b>309+20.39 to 311+1.38</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Low</b>	

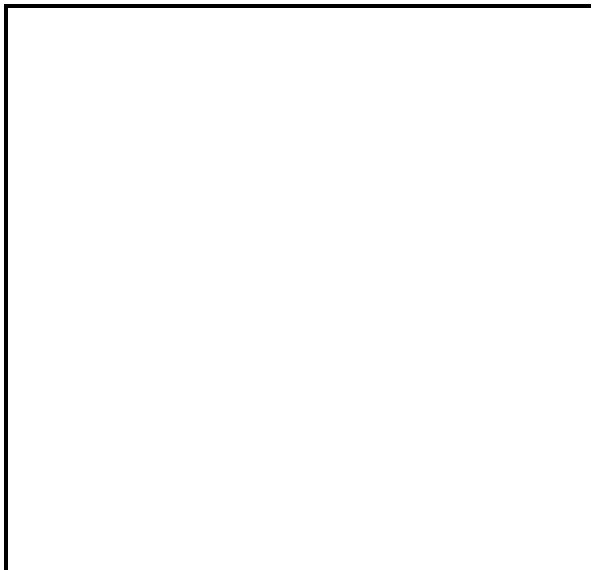
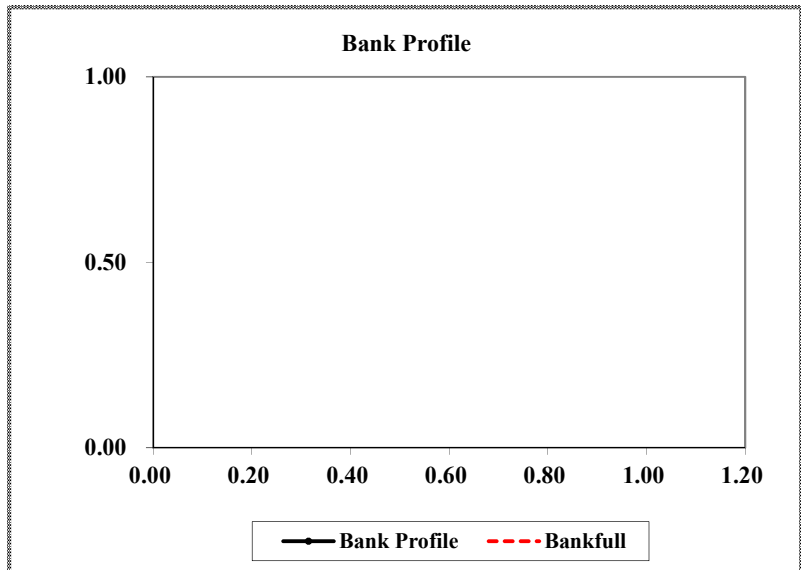
# BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	33.39					
Reach:	405+56.34 to 410+65.81	Comments:						High						
Location:	Right Bank 23	Bank Length	516					Total Score	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018							Values:	5-10	10-20	20-30	30-40	40-45	45-50

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.00	1.00	2.00	7.90	High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.00	0.29	6.00	High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
50.00	0.29	14.50	7.97	High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
50.00			3.41	Low	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
65.00			3.11	Low	
			Adjustment		Notes
<b>Bank Materials</b>					
			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>33.39</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Adjustments</b>								
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
	<b>Boulders</b>	Boulder banks have a low erosion potential.						
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	<b>Clay/Silt Loam</b>	Add 5 points.						
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )							
Stream: <b>Eccleston</b>		Location: <b>Right Bank 23</b>					
Station: <b>405+56.34 to 410+65.81</b>		Stream Type:		Valley Type:			
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>					
Methods for Estimating Near-Bank Stress (NBS)							
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance		
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction		
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction		
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction		
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction		
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction		
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation		
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High					
		Extensive deposition (continuous, cross-channel).....NBS = Extreme					
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)		
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)		
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)		
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)		
		0.64	0.2	3.2	Extreme		
Level IV	(7)	Velocity Gradient ( ft / sec / ft )	Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating							
Near-Bank Stress (NBS) ratings	Method number						
	(1)	(2)	(3)	(4)	(5)	(6)	(7)
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40
Overall Near-Bank Stress (NBS) rating						Extreme	

Method	5
Dominant Near-Bank Stress	
Extreme	

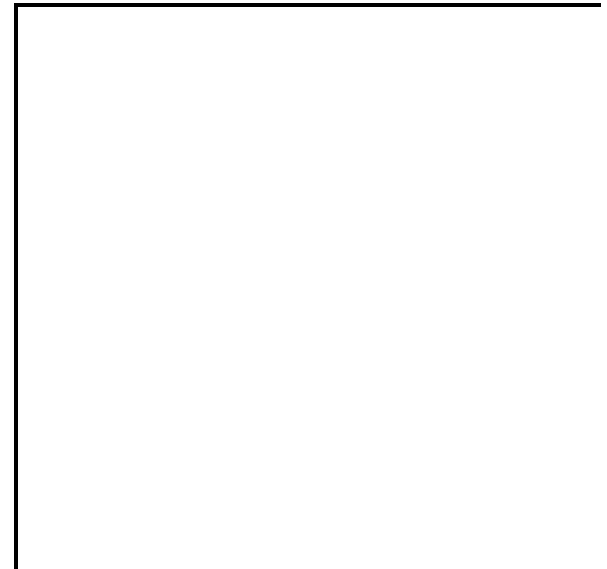
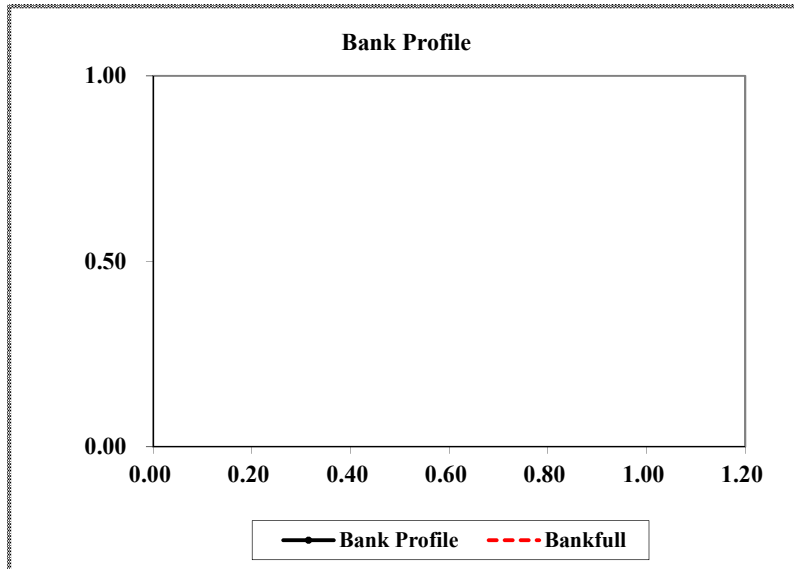
# BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	40.65					
Reach:	307+55.25 to 309+20.39	Comments:						Very High						
Location:	Left Bank 23	Bank Length	163					Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.17	2.50	0.07	8.80	Very High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
40.00	0.07	2.72	10.00	Extreme	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
45.00			3.17	Low	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
40.00			5.11	Moderate	
			Adjustment		Notes
<b>Bank Materials</b>					
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>40.65</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Bank Material	<b>Adjustments</b>						
		<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
		<b>Boulders</b>	Boulder banks have a low erosion potential.					
		<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.					
<b>Clay/Silt Loam</b>		Add 5 points.						
<b>Gravel</b>		Add 5-10 points depending on percentage of bank material composed of sand.						
<b>Sand</b>		Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 23</b>						
Station: <b>307+55.25 to 309+20.39</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N / A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N / A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N / A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Low</b>	



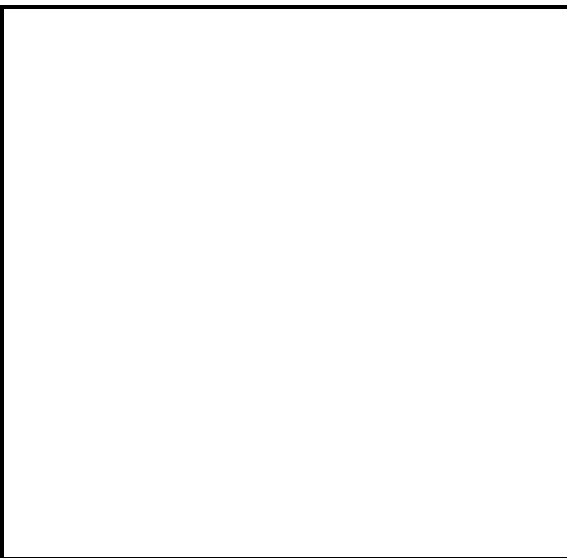
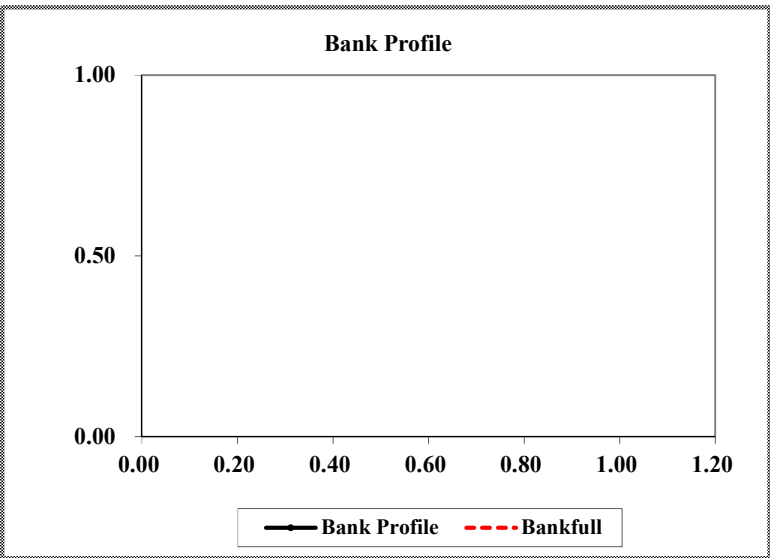
**BANK EROSION HAZARD INDEX**

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	<b>36.77</b>					
<b>Reach:</b>	306+16.05 to 310+97.33	<b>Comments:</b>						<b>Total Score</b>	<b>High</b>					
<b>Location:</b>	Right Bank 24	<b>Bank Length</b>	486					<b>Values:</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.50	0.23	6.79	High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
60.00	0.23	13.92	8.01	Very High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
70.00			4.90	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
60.00			3.50	Low	
			Adjustment		Notes
<b>Bank Materials</b>			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>			0.00		
<b>TOTAL SCORE</b>			<b>36.77</b>		

Bank Erosion Potential							
		Very Low	Low	Moderate	High	Very High	Extreme
<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Adjustments							
<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
<b>Boulders</b>	Boulder banks have a low erosion potential.						
<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
<b>Clay/Silt Loam</b>	Add 5 points.						
<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.						
Stratification							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Right Bank 24</b>											
Station: <b>306+16.05 to 310+97.33</b>		Stream Type:		Valley Type:									
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High											
		Extensive deposition (continuous, cross-channel).....NBS = Extreme											
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)								
		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
		<table border="1"> <tr> <td>Method</td> <td>5</td> </tr> <tr> <td colspan="2">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2">Low</td> </tr> </table>						Method	5	Dominant Near-Bank Stress		Low	
Method	5												
Dominant Near-Bank Stress													
Low													
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
		0.55	0.43	1.2790698	Low								
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						Low							

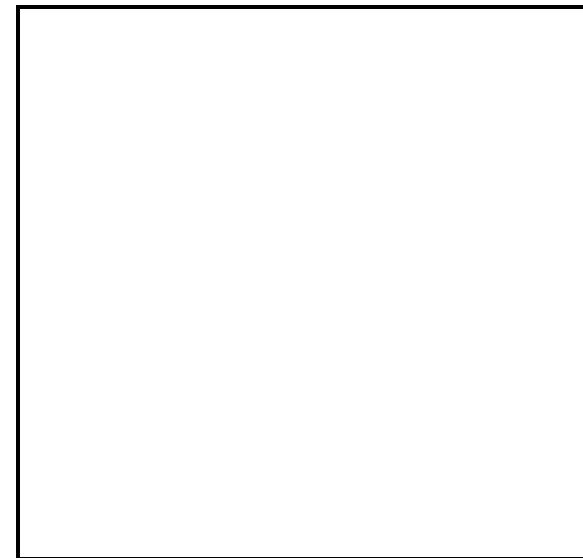
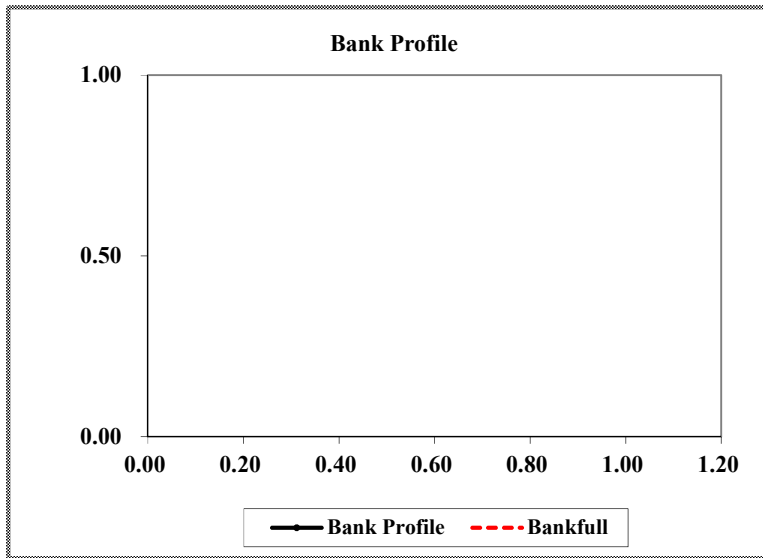
**BANK EROSION HAZARD INDEX**

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	<b>36.77</b>						
<b>Reach:</b>	306+18.16 to 307+55.25	<b>Comments:</b>								<b>High</b>					
<b>Location:</b>	Left Bank 24	<b>Bank Length</b>	137						<b>Total Score Values:</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

<b>Erodibility Variables</b>					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.50	0.23	6.79	High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
60.00	0.23	13.92	8.01	Very High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
70.00			4.90	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
60.00			3.50	Low	
			Adjustment		Notes
<b>Bank Materials</b>					
			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>36.77</b>		

<b>Bank Erosion Potential</b>								
			Very Low	Low	Moderate	High	Very High	Extreme
<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Adjustments</b>								
<b>Bedrock</b>	Bedrock banks have a very low erosion potential.							
<b>Boulders</b>	Boulder banks have a low erosion potential.							
<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.							
<b>Clay/Silt Loam</b>	Add 5 points.							
<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.							
<b>Sand</b>	Add 10 points.							
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

<b>Bank Profile</b>		
Horizontal Distance	Vertical Height	Notes
<b>Bankfull</b>		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Left Bank 24</b>						
Station: <b>306+18.16 to 307+55.25</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N / A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N / A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N / A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

Method	5
Dominant Near-Bank Stress	
Low	

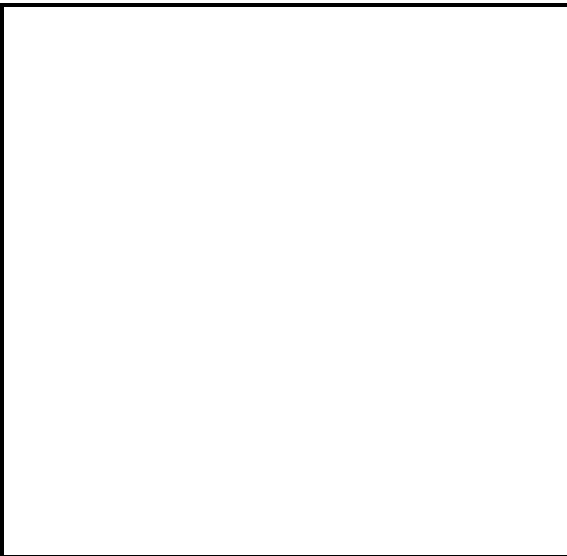
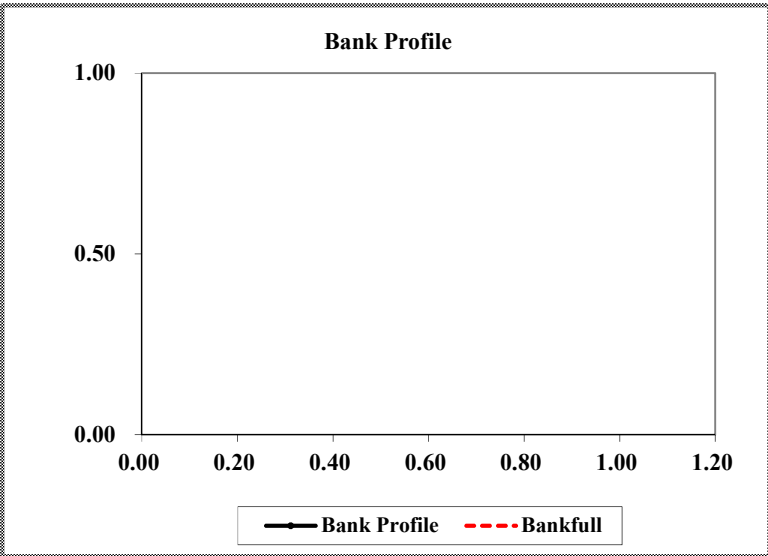
**BANK EROSION HAZARD INDEX**

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	55.27				
Reach:	305+52.82 to 306+16.05	Comments:						Extreme					
Location:	Right Bank 25	Bank Length		62				Total Score Values:					
Date:	5/4/2018							Very Low	Low	Moderate	High	Very High	Extreme
								5-10	10-20	20-30	30-40	40-45	45-50

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.17	2.50	0.07	8.80	Very High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
5.00	0.07	0.34	10.00	Extreme	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
90.00			7.90	High	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
5.00			10.00	Extreme	
			Adjustment		Notes
			10.00		
			Adjustment		Notes
			0.00		
<b>TOTAL SCORE</b>			<b>55.27</b>		

Bank Erosion Potential							
		Very Low	Low	Moderate	High	Very High	Extreme
Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Adjustments							
Bedrock		Bedrock banks have a very low erosion potential.					
Boulders		Boulder banks have a low erosion potential.					
Cobble		Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.					
Clay/Silt Loam		Add 5 points.					
Gravel		Add 5-10 points depending on percentage of bank material composed of sand.					
Sand		Add 10 points.					
Silt / Clay		No adjustment.					
Stratification							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 25</b>						
Station: <b>305+52.82 to 306+16.05</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

Method	5
Dominant Near-Bank Stress	
Low	

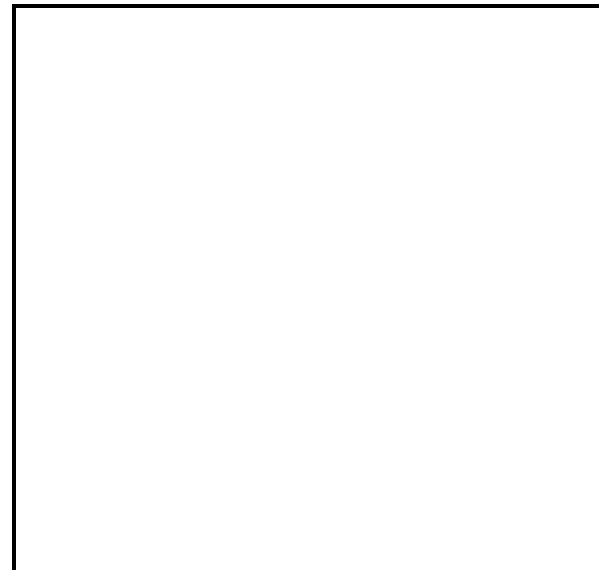
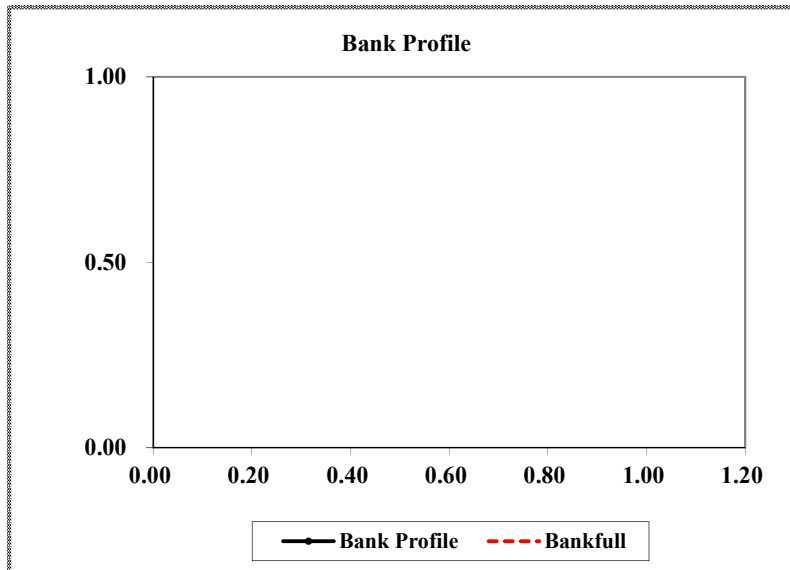
### BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	47.27				
Reach:	305+51.43 to 306+18.16	Comments:					Extreme						
Location:	Left Bank 25	Bank Length:	65				Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018					5-10	10-20	20-30	30-40	40-45	45-50		

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.17	2.50	0.07	8.80	Very High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
10.00	0.07	0.68	10.00	Extreme	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
20.00			1.90	Very Low	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
15.00			8.00	Very High	
			Adjustment		Notes
			10.00		
Bank Materials					
			Adjustment		Notes
Bank Stratification					
			0.00		
<b>TOTAL SCORE</b>			<b>47.27</b>		

Bank Erosion Potential								
			Very Low	Low	Moderate	High	Very High	Extreme
			Value	Index	Value	Index	Value	Index
Erodibility Variables	Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Adjustments							
	Bank Material	Bedrock	Bedrock banks have a very low erosion potential.					
Boulders		Boulder banks have a low erosion potential.						
Cobble		Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
Clay/Silt Loam		Add 5 points.						
Gravel		Add 5-10 points depending on percentage of bank material composed of sand.						
Sand		Add 10 points.						
Silt / Clay	No adjustment.							
Stratification								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Left Bank 25</b>										
Station: <b>305+51.43 to 306+18.16</b>			Stream Type:		Valley Type:										
Observers: <b>PVC &amp; SH</b>			Date: <b>5/4/2018</b>												
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.53</b>	<b>0.38</b>	<b>1.3947368</b>	<b>Low</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									



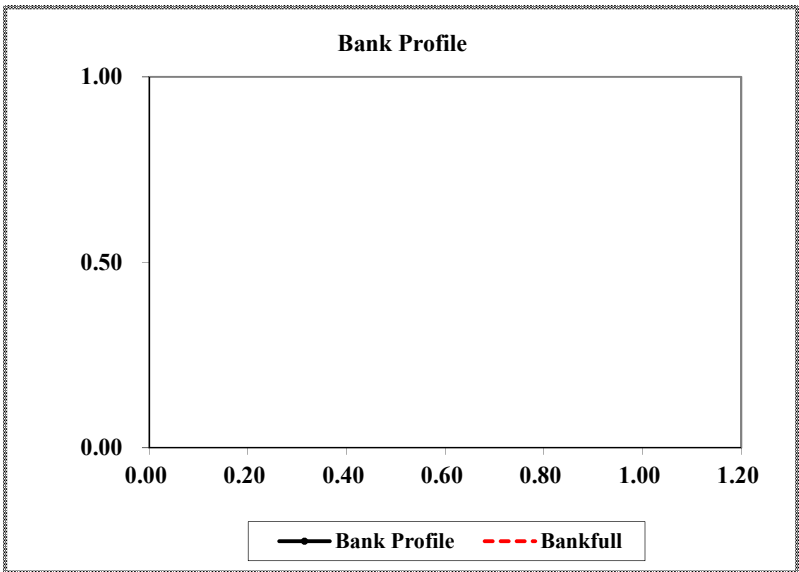
# BANK EROSION HAZARD INDEX

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	36.77						
<b>Reach:</b>	302+31.22 to 305+52.82	<b>Comments:</b>							High						
<b>Location:</b>	Right Bank 26	<b>Bank Length</b>	335						<b>Total Score</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018								<b>Values:</b>	5-10	10-20	20-30	30-40	40-45	45-50

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.50	0.23	6.79	High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
60.00	0.23	13.92	8.01	Very High	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
70.00			4.90	Moderate	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
60.00			3.50	Low	
			Adjustment		Notes
Bank Materials					
			5.00		
			Adjustment		Notes
Bank Stratification					
			0.00		
<b>TOTAL SCORE</b>			<b>36.77</b>		

Bank Erosion Potential							
Erodibility Variables		Very Low	Low	Moderate	High	Very High	Extreme
		<b>Bank Height / Bankfull Height</b>	Value 1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80
	Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Root Depth / Bank Height</b>	Value 1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05	
	Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Weighted Root Density</b>	Value 100-80	79-55	54-30	29-15	14-5	<5	
	Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Bank Angle</b>	Value 0-20	21-60	61-80	81-90	91-119	>119	
	Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Surface Protection</b>	Value 100-80	79-55	54-30	29-15	14-10	<10	
	Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
Adjustments							
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
	<b>Boulders</b>	Boulder banks have a low erosion potential.					
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.					
	<b>Clay/Silt Loam</b>	Add 5 points.					
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.					
	<b>Sand</b>	Add 10 points.					
	<b>Silt / Clay</b>	No adjustment.					
Stratification							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



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Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 26</b>						
Station: <b>302+31.22 to 305+52.82</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>		

<b>Method</b>	<b>5</b>
<b>Dominant Near-Bank Stress</b>	
<b>Low</b>	

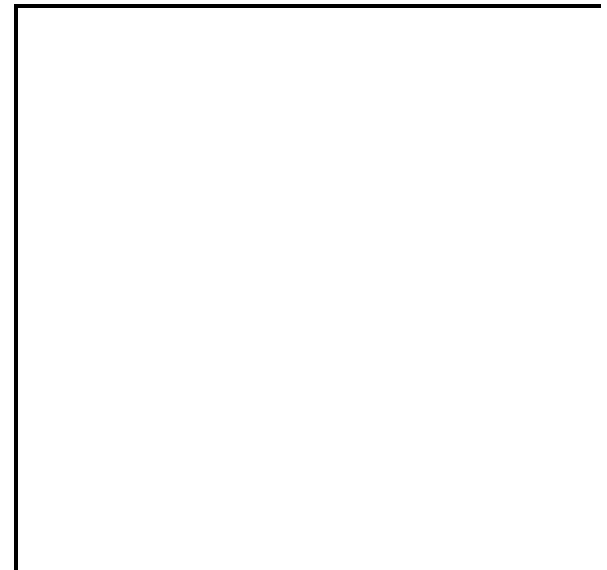
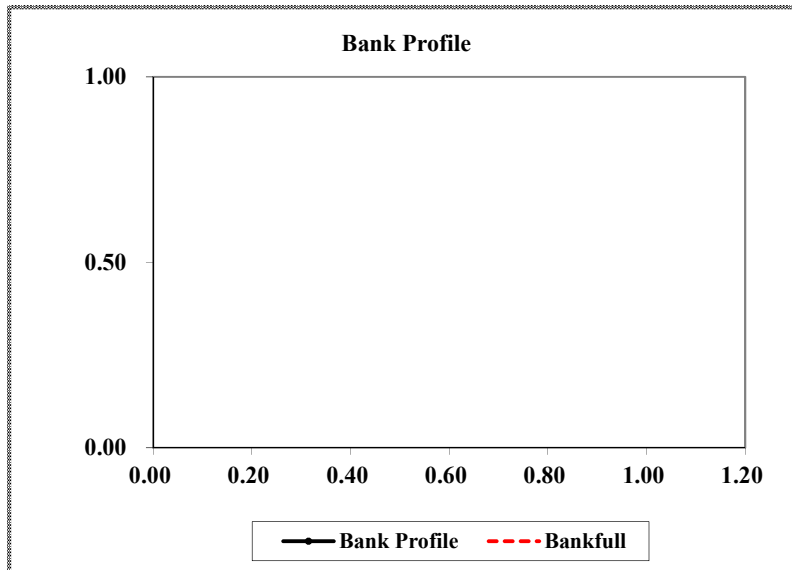
**BANK EROSION HAZARD INDEX**

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	<b>36.77</b>					
<b>Reach:</b>	302+30.62 to 305+51.43	<b>Comments:</b>						<b>High</b>						
<b>Location:</b>	Left Bank 26	<b>Bank Length</b>	322					<b>Total Score Values:</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.50	1.00	2.50	8.57	Very High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.58	2.50	0.23	6.79	High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
60.00	0.23	13.92	8.01	Very High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
70.00			4.90	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
60.00			3.50	Low	
			Adjustment		Notes
<b>Bank Materials</b>					
			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>36.77</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Adjustments</b>								
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
	<b>Boulders</b>	Boulder banks have a low erosion potential.						
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	<b>Clay/Silt Loam</b>	Add 5 points.						
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )															
Stream: <b>Eccleston</b>					Location: <b>Left Bank 26</b>										
Station: <b>302+30.62 to 305+51.43</b>			Stream Type:		Valley Type:										
Observers: <b>PVC &amp; SH</b>			Date: <b>5/4/2018</b>												
Methods for Estimating Near-Bank Stress (NBS)															
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS				Level I	Reconnaissance									
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )				Level II	General prediction									
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )				Level II	General prediction									
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )				Level II	General prediction									
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )				Level III	Detailed prediction									
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )				Level III	Detailed prediction									
(7)	Velocity profiles / Isovels / Velocity gradient				Level IV	Validation									
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme													
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td colspan="2">Dominant Near-Bank Stress</td></tr> <tr><td colspan="2">Low</td></tr> </table>				Method	5	Dominant Near-Bank Stress		Low	
	Method	5													
	Dominant Near-Bank Stress														
Low															
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)											
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)											
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)										
		<b>0.53</b>	<b>0.38</b>	<b>1.3947368</b>	<b>Low</b>										
(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$	Near-Bank Stress (NBS)							
<b>Level IV</b>	(7)	Velocity Gradient ( $ft / sec / ft$ )			Near-Bank Stress (NBS)										
Converting Values to a Near-Bank Stress (NBS) Rating															
Near-Bank Stress (NBS) ratings	Method number														
	(1)	(2)	(3)	(4)	(5)	(6)	(7)								
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50								
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00								
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60								
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00								
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40								
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40								
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>									

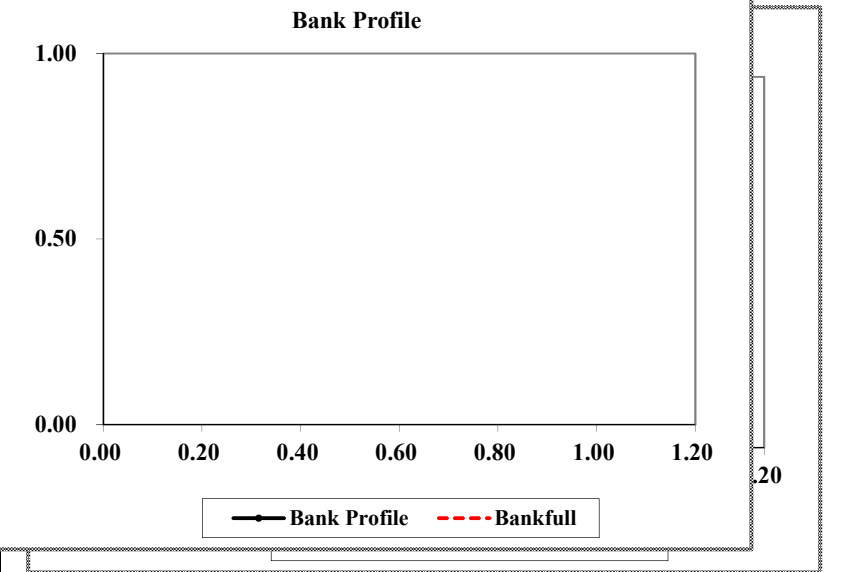
## BANK EROSION HAZARD INDEX

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	<b>30.90</b>						
<b>Reach:</b>	301+33.97 to 302+31.22	<b>Comments:</b>							<b>High</b>						
<b>Location:</b>	Right Bank 27	<b>Bank Length</b>	111						<b>Total Score Values:</b>	Very Low	Low	Moderate	High	Very High	Extreme
<b>Date:</b>	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
4.00	1.00	4.00	10.00	Extreme	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.17	4.00	0.04	10.00	Extreme	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
0.00	0.04				
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
80.00			5.90	Moderate	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
0.00					
				Adjustment	Notes
				5.00	
Bank Materials					
				Adjustment	Notes
Bank Stratification					
				0.00	
<b>TOTAL SCORE</b>			<b>30.90</b>		

Bank Erosion Potential									
		Very Low	Low	Moderate	High	Very High	Extreme		
Erodibility Variables	Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10	
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Adjustments								
	Bank Material		<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
			<b>Boulders</b>	Boulder banks have a low erosion potential.					
			<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.					
		<b>Clay/Silt Loam</b>	Add 5 points.						
		<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
		<b>Sand</b>	Add 10 points.						
		<b>Silt / Clay</b>	No adjustment.						
Stratification									
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.									

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



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Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>				Location: <b>Right Bank 27</b>				
Station: <b>301+33.97 to 302+31.22</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
	Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)		
		(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)		
(4)		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
Overall Near-Bank Stress (NBS) rating						Low		

Method	5
Dominant Near-Bank Stress	
Low	

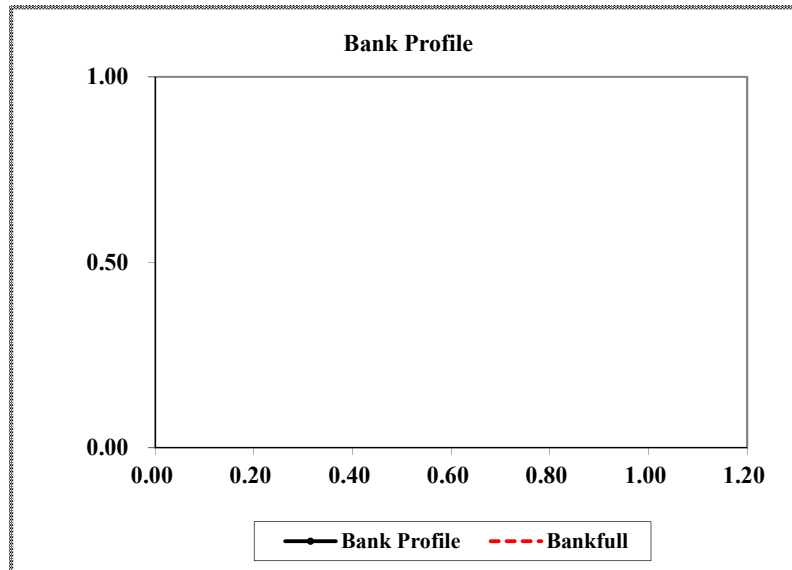
# BANK EROSION HAZARD INDEX

Stream:	Eccleston Mitigation Bank	Observer(s):	PVC	Data:	SH	QA/QC:		Total Score:	43.41						
Reach:	301+25.14 to 302+30.62	Comments:							Very High						
Location:	Left Bank 27	Bank Length	107						Total Score Values:	Very Low	Low	Moderate	High	Very High	Extreme
Date:	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50		

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
2.00	1.00	2.00	7.90	High	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.25	2.00	0.13	8.17	Very High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
40.00	0.13	5.00	9.00	Very High	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
30.00			2.44	Low	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
30.00			5.90	Moderate	
			Adjustment		Notes
			10.00		
<b>Bank Materials</b>					
			Adjustment		Notes
<b>Bank Stratification</b>					
			0.00		
<b>TOTAL SCORE</b>			<b>43.41</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Adjustments</b>								
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
	<b>Boulders</b>	Boulder banks have a low erosion potential.						
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	<b>Clay/Silt Loam</b>	Add 5 points.						
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes




Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Left Bank 27</b>											
Station: <b>301+25.14 to 302+30.62</b>		Stream Type:			Valley Type:								
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
	Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)							
		(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)							
(4)		Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)								
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
<table border="1" style="margin: auto;"> <tr> <td>Method</td> <td>1</td> </tr> <tr> <td colspan="2" style="text-align: center;">Dominant Near-Bank Stress</td> </tr> <tr> <td colspan="2" style="text-align: center;">Very High</td> </tr> </table>								Method	1	Dominant Near-Bank Stress		Very High	
Method	1												
Dominant Near-Bank Stress													
Very High													
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
Overall Near-Bank Stress (NBS) rating						Very High							



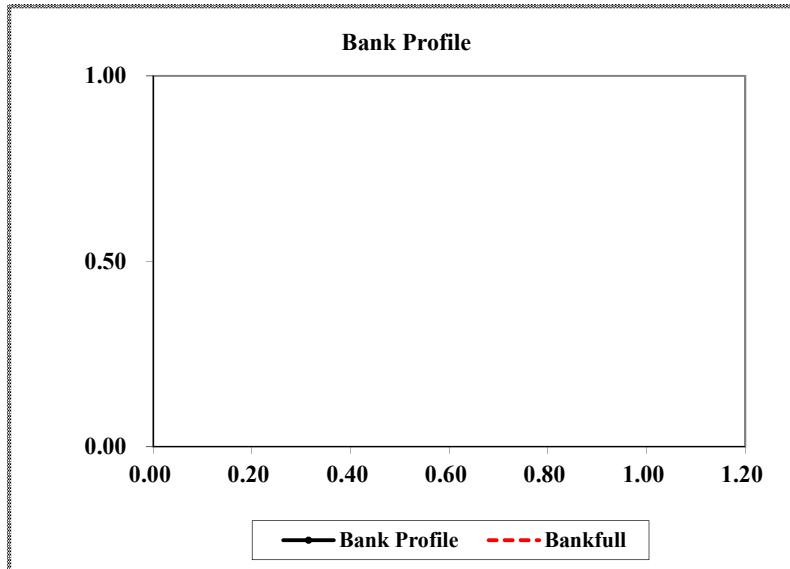
## BANK EROSION HAZARD INDEX

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	<b>45.43</b>					
<b>Reach:</b>	500+3.19 to 503+63.08	<b>Comments:</b>						<b>Very High</b>						
<b>Location:</b>	Right Bank 28	<b>Bank Length</b>	345					<b>Total Score</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018						<b>Values:</b>	5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
6.00	1.50	4.00	10.00	Extreme	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.50	6.00	0.08	8.63	Very High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
20.00	0.08	1.67	10.00	Extreme	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
80.00			5.90	Moderate	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
30.00			5.90	Moderate	
			Adjustment		Notes
<b>Bank Materials</b>			5.00		
			Adjustment		Notes
<b>Bank Stratification</b>			0.00		
<b>TOTAL SCORE</b>			<b>45.43</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	Bank Height / Bankfull Height	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Root Depth / Bank Height	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Weighted Root Density	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Bank Angle	Value	0-20	21-60	61-80	81-90	91-119	>119
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	Surface Protection	Value	100-80	79-55	54-30	29-15	14-10	<10
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
Adjustments								
Bank Material	Bedrock	Bedrock banks have a very low erosion potential.						
	Boulders	Boulder banks have a low erosion potential.						
	Cobble	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	Clay/Silt Loam	Add 5 points.						
	Gravel	Add 5-10 points depending on percentage of bank material composed of sand.						
	Sand	Add 10 points.						
Silt / Clay	No adjustment.							
Stratification								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>		Location: <b>Right Bank 28</b>						
Station: <b>500+3.19 to 503+63.08</b>		Stream Type:		Valley Type:				
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>						
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ (lb/ft <sup>2</sup> )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ (lb/ft <sup>2</sup> )	Ratio $\tau_{nb} / \tau_{bkf}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Very High</b>		

Method	1
Dominant Near-Bank Stress	
Very High	

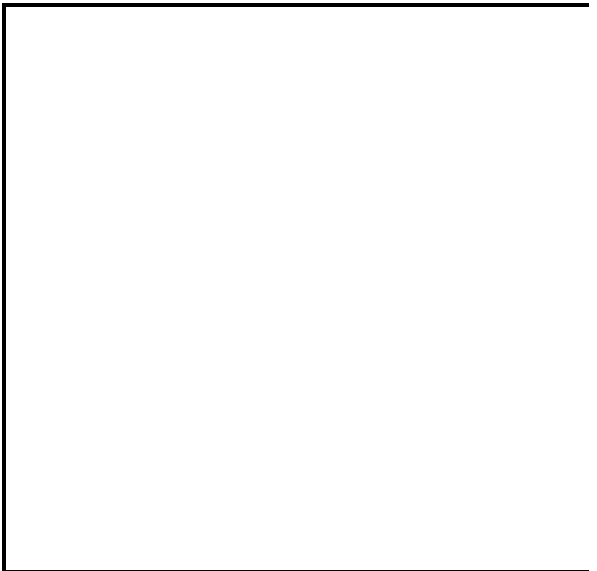
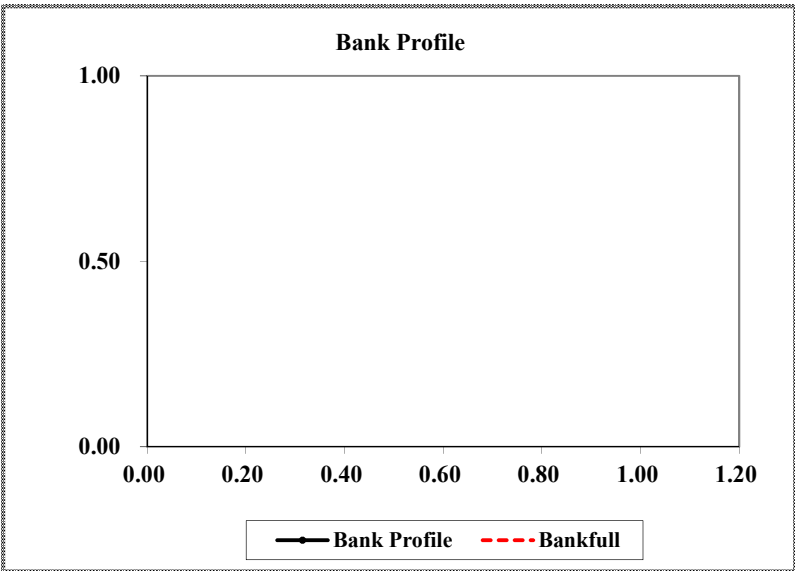
**BANK EROSION HAZARD INDEX**

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	44.64					
<b>Reach:</b>	500+2.38 to 503+65.54	<b>Comments:</b>						Very High						
<b>Location:</b>	Left Bank 28	<b>Bank Length</b>	367					<b>Total Score Values:</b>	Very Low	Low	Moderate	High	Very High	Extreme
<b>Date:</b>	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
Bank Height / Bankfull Height Ratio					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
6.00	1.50	4.00	10.00	Extreme	
Root Depth / Bank Height Ratio					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.50	6.00	0.08	8.63	Very High	
Weighted Root Density					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
30.00	0.08	2.50	10.00	Extreme	
Bank Angle					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
80.00			5.90	Moderate	
Surface Protection					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
40.00			5.11	Moderate	
		Adjustment			Notes
		5.00			
Bank Materials					
		Adjustment			Notes
		0.00			
Bank Stratification					
			0.00		
<b>TOTAL SCORE</b>			<b>44.64</b>		

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value 1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80	
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	<b>Root Depth / Bank Height</b>	Value 1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05	
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	<b>Weighted Root Density</b>	Value 100-80	79-55	54-30	29-15	14-5	<5	
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
Bank Material	<b>Bank Angle</b>	Value 0-20	21-60	61-80	81-90	91-119	>119	
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	<b>Surface Protection</b>	Value 100-80	79-55	54-30	29-15	14-10	<10	
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
	Adjustments							
	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
<b>Boulders</b>	Boulder banks have a low erosion potential.							
<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.							
<b>Clay/Silt Loam</b>	Add 5 points.							
<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.							
<b>Sand</b>	Add 10 points.							
<b>Silt / Clay</b>	No adjustment.							
Stratification								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )								
Stream: <b>Eccleston</b>				Location: <b>Left Bank 28</b>				
Station: <b>500+2.38 to 503+65.54</b>			Stream Type:		Valley Type:			
Observers: <b>PVC &amp; SH</b>			Date: <b>5/4/2018</b>					
Methods for Estimating Near-Bank Stress (NBS)								
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance			
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkr}$ )			Level II	General prediction			
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction			
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction			
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkr}$ )			Level III	Detailed prediction			
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkr}$ )			Level III	Detailed prediction			
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation			
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme						
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkr}$ (ft)	Ratio $R_c / W_{bkr}$	Near-Bank Stress (NBS)			
	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)			
	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)			
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkr}$ (ft)	Ratio $d_{nb} / d_{bkr}$	Near-Bank Stress (NBS)			
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkr}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkr}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkr}$
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)				
Converting Values to a Near-Bank Stress (NBS) Rating								
Near-Bank Stress (NBS) ratings	Method number							
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50	
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00	
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60	
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00	
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40	
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40	
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Moderate</b>		

<b>Method</b>	<b>5</b>
<b>Dominant</b>	
<b>Near-Bank Stress</b>	
<b>Moderate</b>	

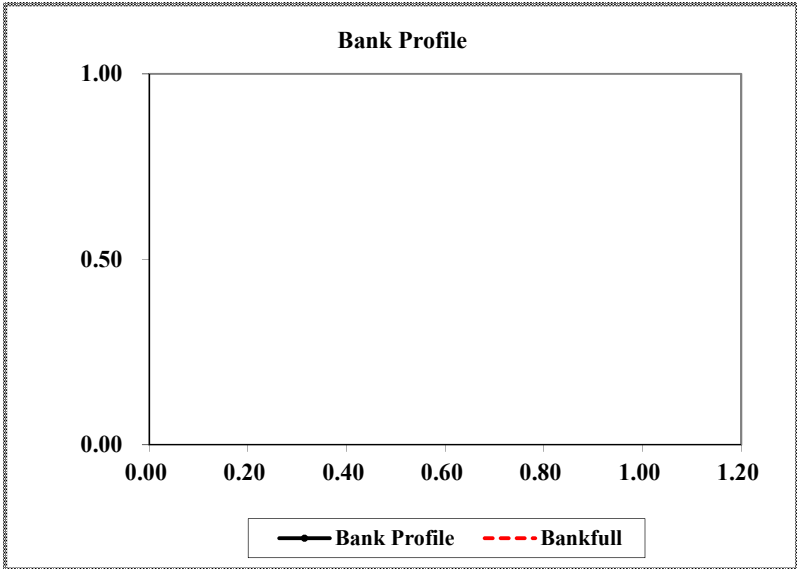
**BANK EROSION HAZARD INDEX**

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	43.66					
<b>Reach:</b>	503+63.08 to 507+15.6	<b>Comments:</b>						Very High						
<b>Location:</b>	Right Bank 29	<b>Bank Length</b>	358					<b>Total Score Values:</b>	Very Low	Low	Moderate	High	Very High	Extreme
<b>Date:</b>	5/4/2018							5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables					
<b>Bank Height / Bankfull Height Ratio</b>					
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes
5.00	1.50	3.33	10.00	Extreme	
<b>Root Depth / Bank Height Ratio</b>					
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes
0.50	5.00	0.10	8.44	Very High	
<b>Weighted Root Density</b>					
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes
20.00	0.10	2.00	10.00	Extreme	
<b>Bank Angle</b>					
Bank Angle (°)			Index	Bank Erosion Potential	Notes
80.00			5.90	Moderate	
<b>Surface Protection</b>					
Surface Protection (%)			Index	Bank Erosion Potential	Notes
50.00			4.32	Moderate	
<b>Bank Materials</b>					
			Adjustment		Notes
			5.00		
<b>Bank Stratification</b>					
			Adjustment		Notes
			0.00		
<b>TOTAL SCORE</b>			<b>43.66</b>		

Bank Erosion Potential							
		Very Low	Low	Moderate	High	Very High	Extreme
<b>Erodibility Variables</b>	<b>Bank Height / Bankfull Height</b>	Value 1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Erodibility Variables</b>	<b>Root Depth / Bank Height</b>	Value 1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Erodibility Variables</b>	<b>Weighted Root Density</b>	Value 100-80	79-55	54-30	29-15	14-5	<5
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Erodibility Variables</b>	<b>Bank Angle</b>	Value 0-20	21-60	61-80	81-90	91-119	>119
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Erodibility Variables</b>	<b>Surface Protection</b>	Value 100-80	79-55	54-30	29-15	14-10	<10
		Index 1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Adjustments</b>							
<b>Bank Material</b>	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.					
	<b>Boulders</b>	Boulder banks have a low erosion potential.					
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.					
	<b>Clay/Silt Loam</b>	Add 5 points.					
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.					
	<b>Sand</b>	Add 10 points.					
	<b>Silt / Clay</b>	No adjustment.					
<b>Stratification</b>							
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.							

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



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Estimating Near-Bank Stress ( NBS )									
Stream: <b>Eccleston</b>		Location: <b>Right Bank 29</b>							
Station: <b>503+63.08 to 507+15.6</b>		Stream Type:		Valley Type:					
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>							
Methods for Estimating Near-Bank Stress (NBS)									
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance				
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction				
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction				
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction				
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction				
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction				
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation				
Level I	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High							
		Extensive deposition (continuous, cross-channel).....NBS = Extreme							
		Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme							
Level II	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)				
Level II	(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)				
Level II	(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)				
Level III	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)				
		<b>0.83</b>	<b>0.62</b>	<b>1.3387097</b>	<b>Low</b>				
Level III	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)
Level IV	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)					
Converting Values to a Near-Bank Stress (NBS) Rating									
Near-Bank Stress (NBS) ratings	Method number								
	(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Very Low	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50		
Low	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00		
Moderate	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60		
High	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00		
Very High	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40		
Extreme	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40		
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Low</b>			

Method	5
Dominant Near-Bank Stress	
Low	

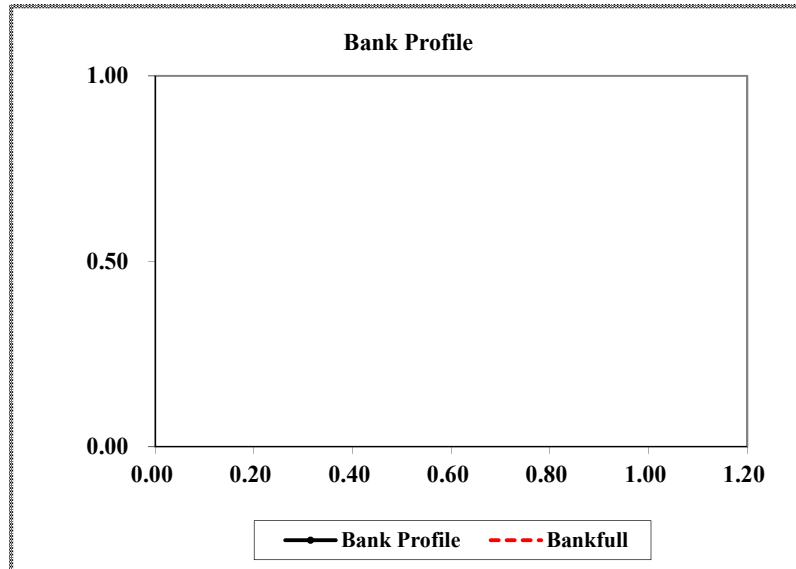
### BANK EROSION HAZARD INDEX

<b>Stream:</b>	Eccleston Mitigation Bank	<b>Observer(s):</b>	PVC	<b>Data:</b>	SH	<b>QA/QC:</b>		<b>Total Score:</b>	44.45						
<b>Reach:</b>	503+65.54 to 507+20.72	<b>Comments:</b>							Very High						
<b>Location:</b>	Left Bank 29	<b>Bank Length</b>	352						<b>Total Score Values:</b>	<b>Very Low</b>	<b>Low</b>	<b>Moderate</b>	<b>High</b>	<b>Very High</b>	<b>Extreme</b>
<b>Date:</b>	5/4/2018								5-10	10-20	20-30	30-40	40-45	45-50	

Erodibility Variables						
<b>Bank Height / Bankfull Height Ratio</b>						
Bank Height	Bankfull Height	Value	Index	Bank Erosion Potential	Notes	
5.00	1.50	3.33	10.00	Extreme		
<b>Root Depth / Bank Height Ratio</b>						
Root Depth	Bank Height	Value	Index	Bank Erosion Potential	Notes	
0.50	5.00	0.10	8.44	Very High		
<b>Weighted Root Density</b>						
Root Density (%)	Root Depth / Bank Height	Value	Index	Bank Erosion Potential	Notes	
30.00	0.10	3.00	10.00	Extreme		
<b>Bank Angle</b>						
Bank Angle (°)			Index	Bank Erosion Potential	Notes	
80.00			5.90	Moderate		
<b>Surface Protection</b>						
Surface Protection (%)			Index	Bank Erosion Potential	Notes	
40.00			5.11	Moderate		
			Adjustment		Notes	
<b>Bank Materials</b>			5.00		Notes	
			Adjustment		Notes	
<b>Bank Stratification</b>			0.00		Notes	
<b>TOTAL SCORE</b>			44.45			

Bank Erosion Potential								
		Very Low	Low	Moderate	High	Very High	Extreme	
Erodibility Variables	<b>Bank Height / Bankfull Height</b>	Value	1.00-1.10	1.11-1.19	1.20-1.50	1.60-2.00	2.10-2.80	>2.80
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Root Depth / Bank Height</b>	Value	1.00-0.90	0.89-0.50	0.49-0.30	0.29-0.15	0.14-0.05	<0.05
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
	<b>Weighted Root Density</b>	Value	100-80	79-55	54-30	29-15	14-5	<5
		Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10
<b>Bank Angle</b>	Value	0-20	21-60	61-80	81-90	91-119	>119	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Surface Protection</b>	Value	100-80	79-55	54-30	29-15	14-10	<10	
	Index	1.0-1.9	2.0-3.9	4.0-5.9	6.0-7.9	8.0-9.0	10	
<b>Adjustments</b>								
Bank Material	<b>Bedrock</b>	Bedrock banks have a very low erosion potential.						
	<b>Boulders</b>	Boulder banks have a low erosion potential.						
	<b>Cobble</b>	Subtract 10 points. No adjustment if sand/gravel compose greater than 50% of bank.						
	<b>Clay/Silt Loam</b>	Add 5 points.						
	<b>Gravel</b>	Add 5-10 points depending on percentage of bank material composed of sand.						
	<b>Sand</b>	Add 10 points.						
<b>Silt / Clay</b>	No adjustment.							
<b>Stratification</b>								
Add 5-10 points depending on position of unstable layers in relation to bankfull stage.								

Bank Profile		
Horizontal Distance	Vertical Height	Notes
Bankfull		
Horizontal Distance	Vertical Height	Notes



Estimating Near-Bank Stress ( NBS )													
Stream: <b>Eccleston</b>		Location: <b>Left Bank 29</b>											
Station: <b>503+65.54 to 507+20.72</b>		Stream Type:		Valley Type:									
Observers: <b>PVC &amp; SH</b>		Date: <b>5/4/2018</b>											
Methods for Estimating Near-Bank Stress (NBS)													
(1)	Channel pattern, transverse bar or split channel/central bar creating NBS			Level I	Reconnaissance								
(2)	Ratio of radius of curvature to bankfull width ( $R_c / W_{bkf}$ )			Level II	General prediction								
(3)	Ratio of pool slope to average water surface slope ( $S_p / S$ )			Level II	General prediction								
(4)	Ratio of pool slope to riffle slope ( $S_p / S_{rif}$ )			Level II	General prediction								
(5)	Ratio of near-bank maximum depth to bankfull mean depth ( $d_{nb} / d_{bkf}$ )			Level III	Detailed prediction								
(6)	Ratio of near-bank shear stress to bankfull shear stress ( $\tau_{nb} / \tau_{bkf}$ )			Level III	Detailed prediction								
(7)	Velocity profiles / Isovels / Velocity gradient			Level IV	Validation								
<b>Level I</b>	(1)	Transverse and/or central bars-short and/or discontinuous.....NBS = High / Very High Extensive deposition (continuous, cross-channel).....NBS = Extreme Chute cutoffs, down-valley meander migration, converging flow.....NBS = Extreme											
<b>Level II</b>	(2)	Radius of Curvature $R_c$ (ft)	Bankfull Width $W_{bkf}$ (ft)	Ratio $R_c / W_{bkf}$	Near-Bank Stress (NBS)	<table border="1"> <tr><td>Method</td><td>5</td></tr> <tr><td>Dominant Near-Bank Stress</td><td></td></tr> <tr><td>Moderate</td><td></td></tr> </table>		Method	5	Dominant Near-Bank Stress		Moderate	
	Method	5											
	Dominant Near-Bank Stress												
Moderate													
(3)	Pool Slope $S_p$	Average Slope $S$	Ratio $S_p / S$	Near-Bank Stress (NBS)									
(4)	Pool Slope $S_p$	Riffle Slope $S_{rif}$	Ratio $S_p / S_{rif}$	Near-Bank Stress (NBS)									
<b>Level III</b>	(5)	Near-Bank Max Depth $d_{nb}$ (ft)	Mean Depth $d_{bkf}$ (ft)	Ratio $d_{nb} / d_{bkf}$	Near-Bank Stress (NBS)								
	(6)	Near-Bank Max Depth $d_{nb}$ (ft)	Near-Bank Slope $S_{nb}$	Near-Bank Shear Stress $\tau_{nb}$ ( $lb/ft^2$ )	Mean Depth $d_{bkf}$ (ft)	Average Slope $S$	Bankfull Shear Stress $\tau_{bkf}$ ( $lb/ft^2$ )	Ratio $\tau_{nb} / \tau_{bkf}$	Near-Bank Stress (NBS)				
<b>Level IV</b>	(7)	Velocity Gradient ( ft / sec / ft )		Near-Bank Stress (NBS)									
Converting Values to a Near-Bank Stress (NBS) Rating													
Near-Bank Stress (NBS) ratings	Method number												
	(1)	(2)	(3)	(4)	(5)	(6)	(7)						
<b>Very Low</b>	N/A	> 3.00	< 0.20	< 0.40	< 1.00	< 0.80	< 0.50						
<b>Low</b>	N/A	2.21 – 3.00	0.20 – 0.40	0.41 – 0.60	1.00 – 1.50	0.80 – 1.05	0.50 – 1.00						
<b>Moderate</b>	N/A	2.01 – 2.20	0.41 – 0.60	0.61 – 0.80	1.51 – 1.80	1.06 – 1.14	1.01 – 1.60						
<b>High</b>	See	1.81 – 2.00	0.61 – 0.80	0.81 – 1.00	1.81 – 2.50	1.15 – 1.19	1.61 – 2.00						
<b>Very High</b>	(1)	1.50 – 1.80	0.81 – 1.00	1.01 – 1.20	2.51 – 3.00	1.20 – 1.60	2.01 – 2.40						
<b>Extreme</b>	Above	< 1.50	> 1.00	> 1.20	> 3.00	> 1.60	> 2.40						
<b>Overall Near-Bank Stress (NBS) rating</b>						<b>Moderate</b>							



indicates results to be Reported to the State (per Section 7.1.3)

**Protocol 1**

Credit for Prevented Sediment during Storm Flow

Step 1: Estimate stream sediment erosion rate

Erosion Rate =	2024.8 tons/yr*	-from Eccleston stream bank erosion rate (BEHI-NBS data)
Study Length =	12754 LF	-Combined right and left bank lengths
<hr/>		
Unit Erosion Rate =	0.158758 tons/ft/yr	* Unit Density of soil = 2286.74 lb/CY (per Bulk Density testing)

Existing Stream length to be restored =	5564 LF
x	0.158758 tons/ft/yr
<hr/>	
Total Erosion =	883.3 tons/yr

Step 2: Convert stream bank erosion to nutrient loading

Nitrogen Concentration =	1.35 lbs/ton	- from recommendations of the Expert Panel to Define Removal Rates for individual Stream Restoration Projects prepared by Tom Schueler and Bill Stack
Phosphorus Concentration =	0.445 lbs/ton	

Step 3: Estimate stream restoration efficiency

Load Reductions (50% efficiency)	
Sediment (TSS) =	883,330 lbs/yr
Nitrogen (TN) =	596.2 lbs/yr
Phosphorus (TP) =	196.5 lbs/yr

**Protocol 2**

Credit for in-stream and riparian nutrient processing within the hyporheic zone during base

Step 1: Determine the total post construction stream length that has been reconnected using the bank height ratio of 1.0 or less:

Restored (proposed) Stream Length =	6796 LF	length of restored stream where BHR = 1.0 or less
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Step 2: Determine the dimensions of the hyporheic box

Channel Width =	9 ft	where BHR = 1.0 or less	Channel Width = average of all stream reaches
Left Floodplain Width =	5 ft	(max. credit = 5 ft)	
Right Floodplain Width =	5 ft	(max. credit = 5 ft)	
<hr/>			
Width Hyporheic Box =	19 ft		

Depth Hyporheic Box =	5 ft	from guidance
x	6796 LF	

Total Vol. Hyporheic Zone =	645,620 ft <sup>3</sup>
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Mass of hyporheic box =	31085.4 tons*	* Unit Density of soil = 2600 lb/CY
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Step 3: Multiply the hyporheic box mass by the unit denitrification rate

Unit Denitrification Rate =	1.06E-04 lbs/ton/day
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Load Reduction	
Nitrogen (TN) =	1202.7 lbs/yr*

Watershed Area at Downstream POI = 1,728 acres

Impervious Area = 22.7%

\* <40% of watershed annual N load (full credit is applicable)

**Protocol 3**

Credit for Floodplain Reconnection Volume

*\*Not performed for this project*



## APPENDIX F Ecological Uplift

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME	LOCATION <u>Intersection Tr.b</u>	
STATION # _____ RIVERMILE _____	STREAM CLASS	
LAT _____ LONG _____	RIVER BASIN	
STORET #	AGENCY	
INVESTIGATORS		
FORM COMPLETED BY	DATE <u>5/4/18</u> TIME _____ AM PM	REASON FOR SURVEY



	Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor	
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b> Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of new fall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 <u>7</u> 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b> Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 <u>8</u> 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b> Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 <u>1</u> 0
<b>4. Sediment Deposition</b> Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.		
SCORE	20 19 18 17 16	<u>15</u> 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0	
<b>5. Channel Flow Status</b> Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.		
SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 <u>2</u> 1 0	

**HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)**

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.																				
	Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.																				
Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.																					
Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.																					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)																				
	The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.																				
The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.																					
Channel straight; waterway has been channelized for a long distance.																					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.																				
	Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.																				
Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.																					
Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.																					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.																				
	70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.																				
50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.																					
Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.																					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.																				
	Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.																				
Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.																					
Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.																					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			

**Total Score** \_\_\_\_\_

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME	LOCATION <u>Below Cypress</u>	
STATION # _____ RIVERMILE _____	STREAM CLASS	
LAT _____ LONG _____	RIVER BASIN	
STORET #	AGENCY	
INVESTIGATORS		
FORM COMPLETED BY	DATE <u>5/18</u> TIME _____ AM PM	REASON FOR SURVEY

	Habitat Parameter	Condition Category			
		Optimal	Suboptimal	Marginal	Poor
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b>	Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of newfall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.
	SCORE	20 19 18 17 16	15 (14) 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b>	Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.
	SCORE	20 19 18 17 (16)	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b>	Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.
	SCORE	20 19 18 17 16	15 (14) 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>4. Sediment Deposition</b>	Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.	
SCORE	20 19 18 17 16	15 14 13 (12) 11	10 9 8 7 6	5 4 3 2 1 0	
<b>5. Channel Flow Status</b>	Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.	
SCORE	20 19 18 17 16	15 14 (13) 12 11	10 9 8 7 6	5 4 3 2 1 0	

Below Cypress

HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.					Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.					Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.					Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					Channel straight; waterway has been channelized for a long distance.					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.					Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.					Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.					Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.					70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.					50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.					Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.					Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.					Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.					Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.					
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			

Total Score \_\_\_\_\_

**HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)**

STREAM NAME		LOCATION <u>Downstream Forested Jones</u>	
STATION # _____	RIVERMILE _____	STREAM CLASS _____	
LAT _____	LONG _____	RIVER BASIN _____	
STORET # _____		AGENCY _____	
INVESTIGATORS _____			
FORM COMPLETED BY _____		DATE <u>5/4/18</u> TIME _____ AM PM	REASON FOR SURVEY _____

Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor
<b>1. Epifaunal Substrate/ Available Cover</b>	Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of newfall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 (7) 6
<b>2. Pool Substrate Characterization</b>	Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.
	SCORE	20 19 18 17 16	15 (14) 13 12 11	10 9 8 7 6
<b>3. Pool Variability</b>	Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6
<b>4. Sediment Deposition</b>	Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 (8) 7 6
<b>5. Channel Flow Status</b>	Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 (8) 7 6



Downstream Forested Jones

**HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)**

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.																				
	Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.					Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.					Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.										
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)																				
	The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					Channel straight; waterway has been channelized for a long distance.										
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.																				
	Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.					Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.					Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.										
SCORE ___ (LB)	Left Bank		10	9		8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank		10	9		8	7	6			5	4	3			2	1	0			
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.																				
	70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.					50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.					Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.										
SCORE ___ (LB)	Left Bank		10	9		8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank		10	9		8	7	6			5	4	3			2	1	0			
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.																				
	Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.					Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.					Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.										
SCORE ___ (LB)	Left Bank		10	9		8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank		10	9		8	7	6			5	4	3			2	1	0			

**Total Score** \_\_\_\_\_

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME		LOCATION <u>Walnut</u>	
STATION # _____ RIVERMILE _____		STREAM CLASS	
LAT _____ LONG _____		RIVER BASIN	
STORET #		AGENCY	
INVESTIGATORS			
FORM COMPLETED BY		DATE <u>5/4/18</u> TIME _____ AM PM	REASON FOR SURVEY

	Habitat Parameter	Condition Category			
		Optimal	Suboptimal	Marginal	Poor
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b>  Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of new fall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b>  Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b>  Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.	
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>4. Sediment Deposition</b>  Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.		
SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0	
<b>5. Channel Flow Status</b>  Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.		
SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0	

Walnut

HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category																							
	Optimal					Suboptimal					Marginal					Poor								
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.					Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.					Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.					Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.								
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0			
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					Channel straight; waterway has been channelized for a long distance.								
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0			
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.					Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.					Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.					Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.								
SCORE ___ (LB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0
SCORE ___ (RB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.					70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.					50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.					Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.								
SCORE ___ (LB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0
SCORE ___ (RB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.					Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.					Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.					Width of riparian zone <6 meters: little or no riparian vegetation due to human activities.								
SCORE ___ (LB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0
SCORE ___ (RB)	Left Bank	10	9	8	7	6	5	4	3	2	1	0	Right Bank	10	9	8	7	6	5	4	3	2	1	0

Parameters to be evaluated broader than sampling reach

Total Score \_\_\_\_\_

**HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)**

STREAM NAME		LOCATION <u>Middle Jones</u>	
STATION # _____	RIVERMILE _____	STREAM CLASS	
LAT _____	LONG _____	RIVER BASIN	
STORET #		AGENCY	
INVESTIGATORS			
FORM COMPLETED BY		DATE <u>5/4/18</u>	REASON FOR SURVEY
		TIME _____ AM PM	

Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor
<b>1. Epifaunal Substrate/ Available Cover</b> Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient). SCORE	20 19 18 17 16	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of new fall, but not yet prepared for colonization (may rate at high end of scale). 15 14 13 12 11	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed. 10 9 8 7 6	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking. 5 4 3 2 1 0
	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>2. Pool Substrate Characterization</b> Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common. SCORE	20 19 18 17 16	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present. 15 14 13 12 11	All mud or clay or sand bottom; little or no root mat; no submerged vegetation. 10 9 8 7 6	Hard-pan clay or bedrock; no root mat or vegetation. 5 4 3 2 1 0
	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>3. Pool Variability</b> Even mix of large-shallow, large-deep, small-shallow, small-deep pools present. SCORE	20 19 18 17 16	Majority of pools large-deep; very few shallow. 15 14 13 12 11	Shallow pools much more prevalent than deep pools. 10 9 8 7 6	Majority of pools small-shallow or pools absent. 5 4 3 2 1 0
	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>4. Sediment Deposition</b> Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition. SCORE	20 19 18 17 16	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools. 15 14 13 12 11	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent. 10 9 8 7 6	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition. 5 4 3 2 1 0
	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
<b>5. Channel Flow Status</b> Water reaches base of both lower banks, and minimal amount of channel substrate is exposed. SCORE	20 19 18 17 16	Water fills >75% of the available channel; or <25% of channel substrate is exposed. 15 14 13 12 11	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed. 10 9 8 7 6	Very little water in channel and mostly present as standing pools. 5 4 3 2 1 0
	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0

HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.																				
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)																				
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.																				
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.																				
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.																				
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			

Total Score \_\_\_\_\_

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME	LOCATION <u>Braided Trib</u>	
STATION # _____ RIVERMILE _____	STREAM CLASS	
LAT _____ LONG _____	RIVER BASIN	
STORET #	AGENCY	
INVESTIGATORS		
FORM COMPLETED BY	DATE <u>5/4/18</u> TIME _____ AM PM	REASON FOR SURVEY

	Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor	
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b>	Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of newfall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b>	Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b>	Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>4. Sediment Deposition</b>	Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>5. Channel Flow Status</b>	Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0

Braided Trib

HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor
6. Channel Alteration	Channelization or dredging absent or minimal; stream with normal pattern.	Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.	Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.	Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.
SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
7. Channel Sinuosity	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)	The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line. <i>Braided</i>	The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.	Channel straight; waterway has been channelized for a long distance.
SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
8. Bank Stability (score each bank)	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.	Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.	Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.	Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.
SCORE ___ (LB)	Left Bank 10 9	8 7 6	5 4 3	2 1 0
SCORE ___ (RB)	Right Bank 10 9	8 7 6	5 4 3	2 1 0
9. Vegetative Protection (score each bank)  Note: determine left or right side by facing downstream.	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.	70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.	50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.	Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.
SCORE ___ (LB)	Left Bank 10 9	8 7 6	5 4 3	2 1 0
SCORE ___ (RB)	Right Bank 10 9	8 7 6	5 4 3	2 1 0
10. Riparian Vegetative Zone Width (score each bank riparian zone)	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.	Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.	Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.	Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.
SCORE ___ (LB)	Left Bank 10 9	8 7 6	5 4 3	2 1 0
SCORE ___ (RB)	Right Bank 10 9	8 7 6	5 4 3	2 1 0

Parameters to be evaluated broader than sampling reach

Total Score \_\_\_\_\_

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME	LOCATION <u>Railroad Trib</u>	
STATION # _____ RIVERMILE _____	STREAM CLASS	
LAT _____ LONG _____	RIVER BASIN	
STORET #	AGENCY	
INVESTIGATORS		
FORM COMPLETED BY	DATE _____ TIME _____ AM PM	REASON FOR SURVEY

	Habitat Parameter	Condition Category			
		Optimal	Suboptimal	Marginal	Poor
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b>	Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of newfall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b>	Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b>	Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>4. Sediment Deposition</b>	Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>5. Channel Flow Status</b>	Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0



# Railroad Trib

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.																				
	Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.					Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.					Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.										
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)																				
	The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					Channel straight; waterway has been channelized for a long distance.										
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.																				
	Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.					Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.					Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.										
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.																				
	70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.					50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.					Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.										
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.																				
	Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.					Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.					Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.										
SCORE ___ (LB)	Left Bank	10	9			8	7	6			5	4	3			2	1	0			
SCORE ___ (RB)	Right Bank	10	9			8	7	6			5	4	3			2	1	0			

Total Score \_\_\_\_\_

## HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (FRONT)

STREAM NAME	LOCATION <u>Cypress</u>	
STATION # _____ RIVERMILE _____	STREAM CLASS	
LAT _____ LONG _____	RIVER BASIN	
STORET #	AGENCY	
INVESTIGATORS		
FORM COMPLETED BY	DATE <u>5/4/18</u> TIME _____ AM PM	REASON FOR SURVEY

	Habitat Parameter	Condition Category			
	Optimal	Suboptimal	Marginal	Poor	
Parameters to be evaluated in sampling reach	<b>1. Epifaunal Substrate/ Available Cover</b>	Greater than 50% of substrate favorable for epifaunal colonization and fish cover; mix of snags, submerged logs, undercut banks, cobble or other stable habitat and at stage to allow full colonization potential (i.e., logs/snags that are <u>not</u> new fall and not transient).	30-50% mix of stable habitat; well-suited for full colonization potential; adequate habitat for maintenance of populations; presence of additional substrate in the form of newfall, but not yet prepared for colonization (may rate at high end of scale).	10-30% mix of stable habitat; habitat availability less than desirable; substrate frequently disturbed or removed.	Less than 10% stable habitat; lack of habitat is obvious; substrate unstable or lacking.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>2. Pool Substrate Characterization</b>	Mixture of substrate materials, with gravel and firm sand prevalent; root mats and submerged vegetation common.	Mixture of soft sand, mud, or clay; mud may be dominant; some root mats and submerged vegetation present.	All mud or clay or sand bottom; little or no root mat; no submerged vegetation.	Hard-pan clay or bedrock; no root mat or vegetation.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>3. Pool Variability</b>	Even mix of large-shallow, large-deep, small-shallow, small-deep pools present.	Majority of pools large-deep; very few shallow.	Shallow pools much more prevalent than deep pools.	Majority of pools small-shallow or pools absent.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>4. Sediment Deposition</b>	Little or no enlargement of islands or point bars and less than <20% of the bottom affected by sediment deposition.	Some new increase in bar formation, mostly from gravel, sand or fine sediment; 20-50% of the bottom affected; slight deposition in pools.	Moderate deposition of new gravel, sand or fine sediment on old and new bars; 50-80% of the bottom affected; sediment deposits at obstructions, constrictions, and bends; moderate deposition of pools prevalent.	Heavy deposits of fine material, increased bar development; more than 80% of the bottom changing frequently; pools almost absent due to substantial sediment deposition.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0
	<b>5. Channel Flow Status</b>	Water reaches base of both lower banks, and minimal amount of channel substrate is exposed.	Water fills >75% of the available channel; or <25% of channel substrate is exposed.	Water fills 25-75% of the available channel, and/or riffle substrates are mostly exposed.	Very little water in channel and mostly present as standing pools.
	SCORE	20 19 18 17 16	15 14 13 12 11	10 9 8 7 6	5 4 3 2 1 0

Cypress

HABITAT ASSESSMENT FIELD DATA SHEET—LOW GRADIENT STREAMS (BACK)

Habitat Parameter	Condition Category																				
	Optimal					Suboptimal					Marginal					Poor					
<b>6. Channel Alteration</b>	Channelization or dredging absent or minimal; stream with normal pattern.					Some channelization present, usually in areas of bridge abutments; evidence of past channelization, i.e., dredging, (greater than past 20 yr) may be present, but recent channelization is not present.					Channelization may be extensive; embankments or shoring structures present on both banks; and 40 to 80% of stream reach channelized and disrupted.					Banks shored with gabion or cement; over 80% of the stream reach channelized and disrupted. Instream habitat greatly altered or removed entirely.					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>7. Channel Sinuosity</b>	The bends in the stream increase the stream length 3 to 4 times longer than if it was in a straight line. (Note - channel braiding is considered normal in coastal plains and other low-lying areas. This parameter is not easily rated in these areas.)					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					The bends in the stream increase the stream length 1 to 2 times longer than if it was in a straight line.					Channel straight; waterway has been channelized for a long distance.					
SCORE	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	0
<b>8. Bank Stability (score each bank)</b>	Banks stable; evidence of erosion or bank failure absent or minimal; little potential for future problems. <5% of bank affected.					Moderately stable; infrequent, small areas of erosion mostly healed over. 5-30% of bank in reach has areas of erosion.					Moderately unstable; 30-60% of bank in reach has areas of erosion; high erosion potential during floods.					Unstable; many eroded areas; "raw" areas frequent along straight sections and bends; obvious bank sloughing; 60-100% of bank has erosional scars.					
SCORE ___ (LB)	Left Bank 10 9					8 7 6					5 4 3					2 1 0					
SCORE ___ (RB)	Right Bank 10 9					8 7 6					5 4 3					2 1 0					
<b>9. Vegetative Protection (score each bank)</b>	More than 90% of the streambank surfaces and immediate riparian zone covered by native vegetation, including trees, understory shrubs, or nonwoody macrophytes; vegetative disruption through grazing or mowing minimal or not evident; almost all plants allowed to grow naturally.					70-90% of the streambank surfaces covered by native vegetation, but one class of plants is not well-represented; disruption evident but not affecting full plant growth potential to any great extent; more than one-half of the potential plant stubble height remaining.					50-70% of the streambank surfaces covered by vegetation; disruption obvious; patches of bare soil or closely cropped vegetation common; less than one-half of the potential plant stubble height remaining.					Less than 50% of the streambank surfaces covered by vegetation; disruption of streambank vegetation is very high; vegetation has been removed to 5 centimeters or less in average stubble height.					
SCORE ___ (LB)	Left Bank 10 9					8 7 6					5 4 3					2 1 0					
SCORE ___ (RB)	Right Bank 10 9					8 7 6					5 4 3					2 1 0					
<b>10. Riparian Vegetative Zone Width (score each bank riparian zone)</b>	Width of riparian zone >18 meters; human activities (i.e., parking lots, roadbeds, clear-cuts, lawns, or crops) have not impacted zone.					Width of riparian zone 12-18 meters; human activities have impacted zone only minimally.					Width of riparian zone 6-12 meters; human activities have impacted zone a great deal.					Width of riparian zone <6 meters; little or no riparian vegetation due to human activities.					
SCORE ___ (LB)	Left Bank 10 9					8 7 6					5 4 3					2 1 0					
SCORE ___ (RB)	Right Bank 10 9					8 7 6					5 4 3					2 1 0					

Parameters to be evaluated broader than sampling reach

Total Score \_\_\_\_\_



## APPENDIX G

### Design Computations

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River Name: Proposed Design  
 Reach Name: Main Stem  
 Cross Section Name: Typical Section - B stream  
 Survey Date: 09/27/2018

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Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
11.75	0	1.3	
16.25	0	1	
20.75	0	1.3	
22.5	0	2	
32	0	2	

-----

Cross Sectional Geometry

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	Channel	Left	Right
Floodprone Elevation (ft)	3	3	3
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	32	-----	-----
Bankfull width (ft)	12.5	6.25	6.25
Entrenchment Ratio	2.56	-----	-----
Mean Depth (ft)	0.71	0.71	0.71
Maximum Depth (ft)	1	1	1
width/Depth Ratio	17.61	8.8	8.8
Bankfull Area (sq ft)	8.88	4.44	4.44
wetted Perimeter (ft)	12.79	7.39	7.39
Hydraulic Radius (ft)	0.69	0.6	0.6
Begin BKF Station	10	10	16.25
End BKF Station	22.5	16.25	22.5

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Entrainment Calculations

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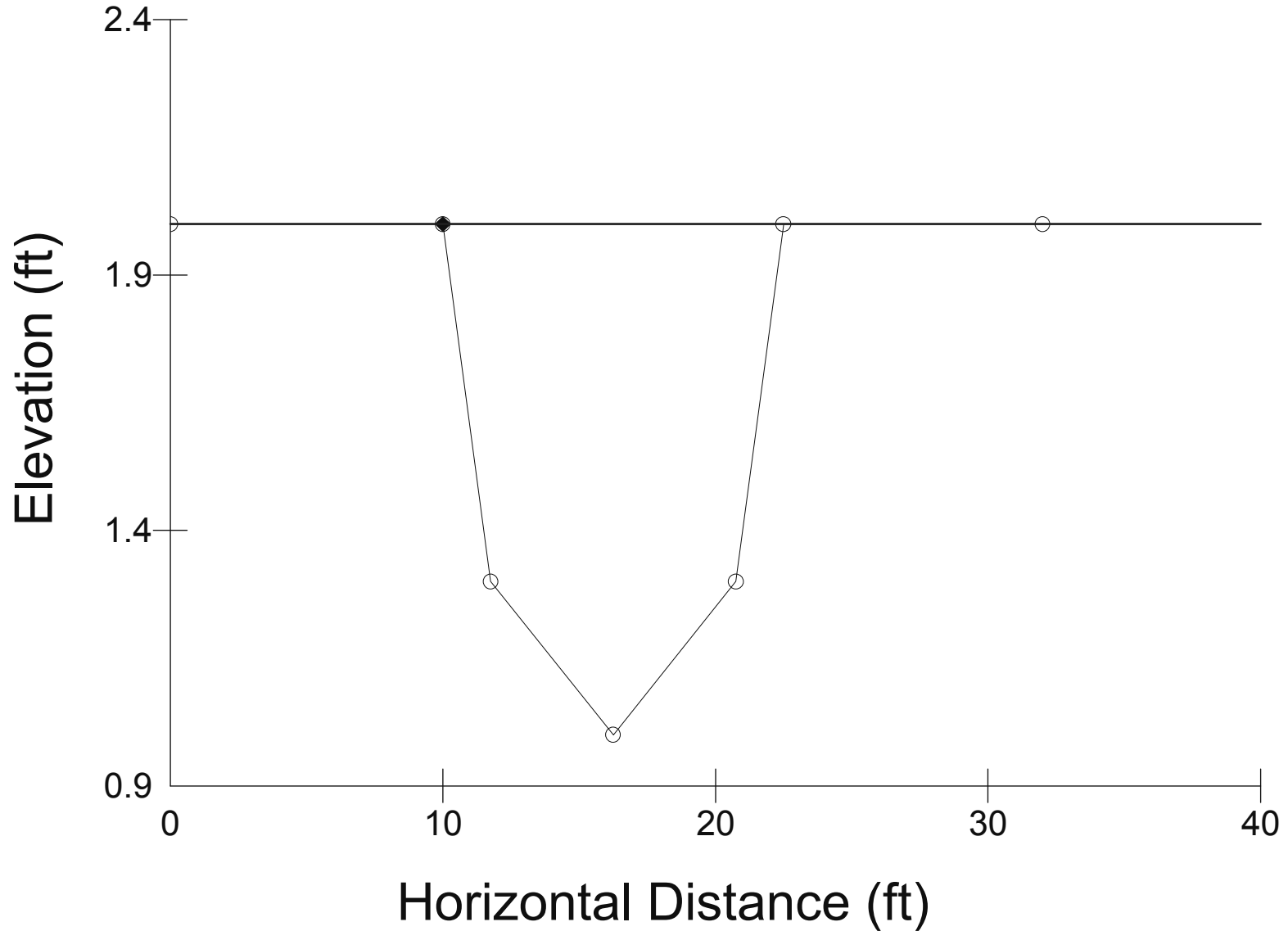
Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

# Typical Section - B stream

○ Ground Points    ◆ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 12.5      Dbkf = .71      Abkf = 8.88



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River Name: Proposed Design  
 Reach Name: Main Stem  
 Cross Section Name: Typical Section - C stream  
 Survey Date: 09/13/2018

-----

Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
11.5	0	1.4	
14.5	0	1.2	
17.5	0	1.4	
19	0	2	
29	0	2	

-----

Cross Sectional Geometry

-----

	Channel	Left	Right
Floodprone Elevation (ft)	2.8	2.8	2.8
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	29	-----	-----
Bankfull width (ft)	9	4.5	4.5
Entrenchment Ratio	3.22	-----	-----
Mean Depth (ft)	0.57	0.57	0.57
Maximum Depth (ft)	0.8	0.8	0.8
width/Depth Ratio	15.79	7.94	7.89
Bankfull Area (sq ft)	5.1	2.55	2.55
wetted Perimeter (ft)	9.24	5.42	5.42
Hydraulic Radius (ft)	0.55	0.47	0.47
Begin BKF Station	10	10	14.5
End BKF Station	19	14.5	19

-----

Entrainment Calculations

-----

Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

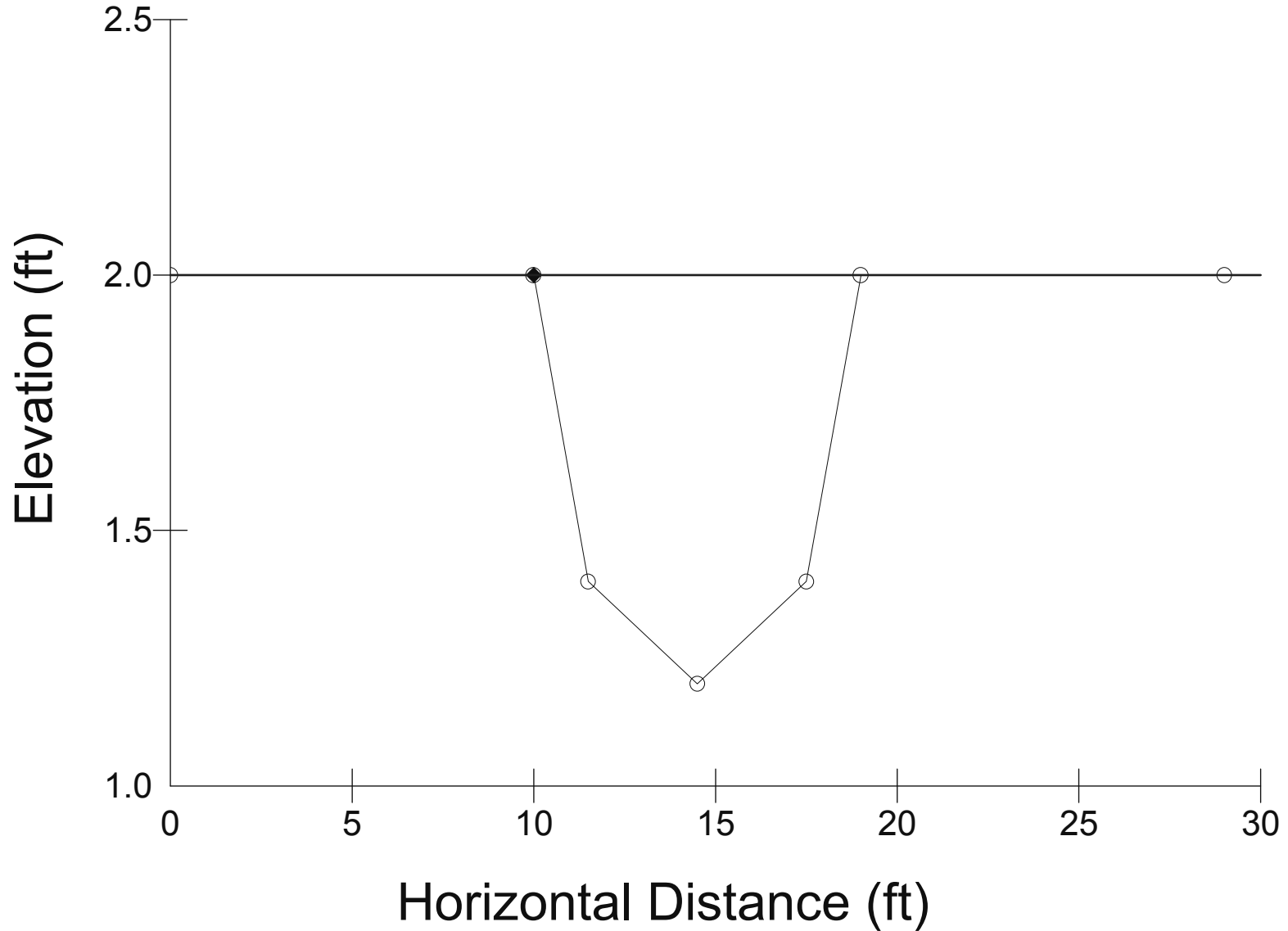
# Typ Section - 9 ft

○ Ground Points    ◆ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 9

Dbkf = .57

Abkf = 5.1





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River Name: Proposed Design  
 Reach Name: South Tributary  
 Cross Section Name: Typical Section - B stream  
 Survey Date: 09/18/2018

-----

Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
10.75	0	1.7	
12	0	1.7	
13	0	1.3	
14	0	1.3	
15	0	1.7	
16.25	0	1.7	
17	0	2	
26	0	2	

-----

Cross Sectional Geometry

-----

	Channel	Left	Right
Floodprone Elevation (ft)	2.7	2.7	2.7
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	26	-----	-----
Bankfull width (ft)	7	3	4
Entrenchment Ratio	3.71	-----	-----
Mean Depth (ft)	0.38	0.33	0.42
Maximum Depth (ft)	0.7	0.7	0.7
width/Depth Ratio	18.42	9.11	9.52
Bankfull Area (sq ft)	2.67	0.99	1.69
wetted Perimeter (ft)	7.27	3.83	4.83
Hydraulic Radius (ft)	0.37	0.26	0.35
Begin BKF Station	10	10	13
End BKF Station	17	13	17

-----

Entrainment Calculations

-----

Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

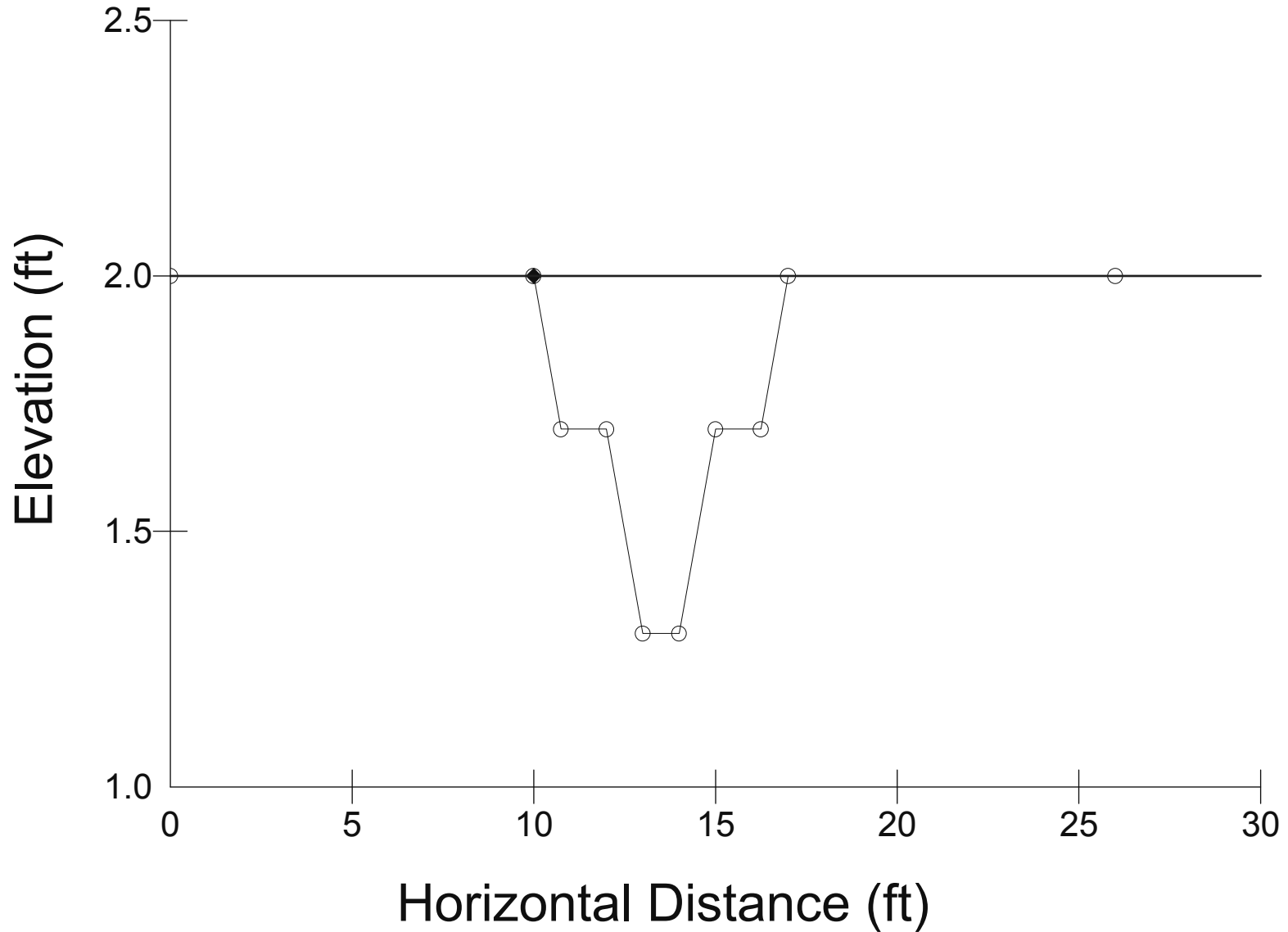
# Typical Section - B stream

○ Ground Points    ♦ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 7

Dbkf = .38

Abkf = 2.68



-----

River Name: Proposed Design  
 Reach Name: South Tributary  
 Cross Section Name: Typical Section - C Stream  
 Survey Date: 09/19/2018

-----

Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
11.05	0	1.58	
12.25	0	1.5	
13.45	0	1.58	
14.5	0	2	
24.5	0	2	

-----

Cross Sectional Geometry

-----

	Channel	Left	Right
Floodprone Elevation (ft)	2.5	2.5	2.5
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	24.5	-----	-----
Bankfull width (ft)	4.5	2.25	2.25
Entrenchment Ratio	5.44	-----	-----
Mean Depth (ft)	0.34	0.34	0.34
Maximum Depth (ft)	0.5	0.5	0.5
width/Depth Ratio	13.24	6.55	6.62
Bankfull Area (sq ft)	1.54	0.77	0.77
wetted Perimeter (ft)	4.67	2.83	2.83
Hydraulic Radius (ft)	0.33	0.27	0.27
Begin BKF Station	10	10	12.25
End BKF Station	14.5	12.25	14.5

-----

Entrainment Calculations

-----

Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

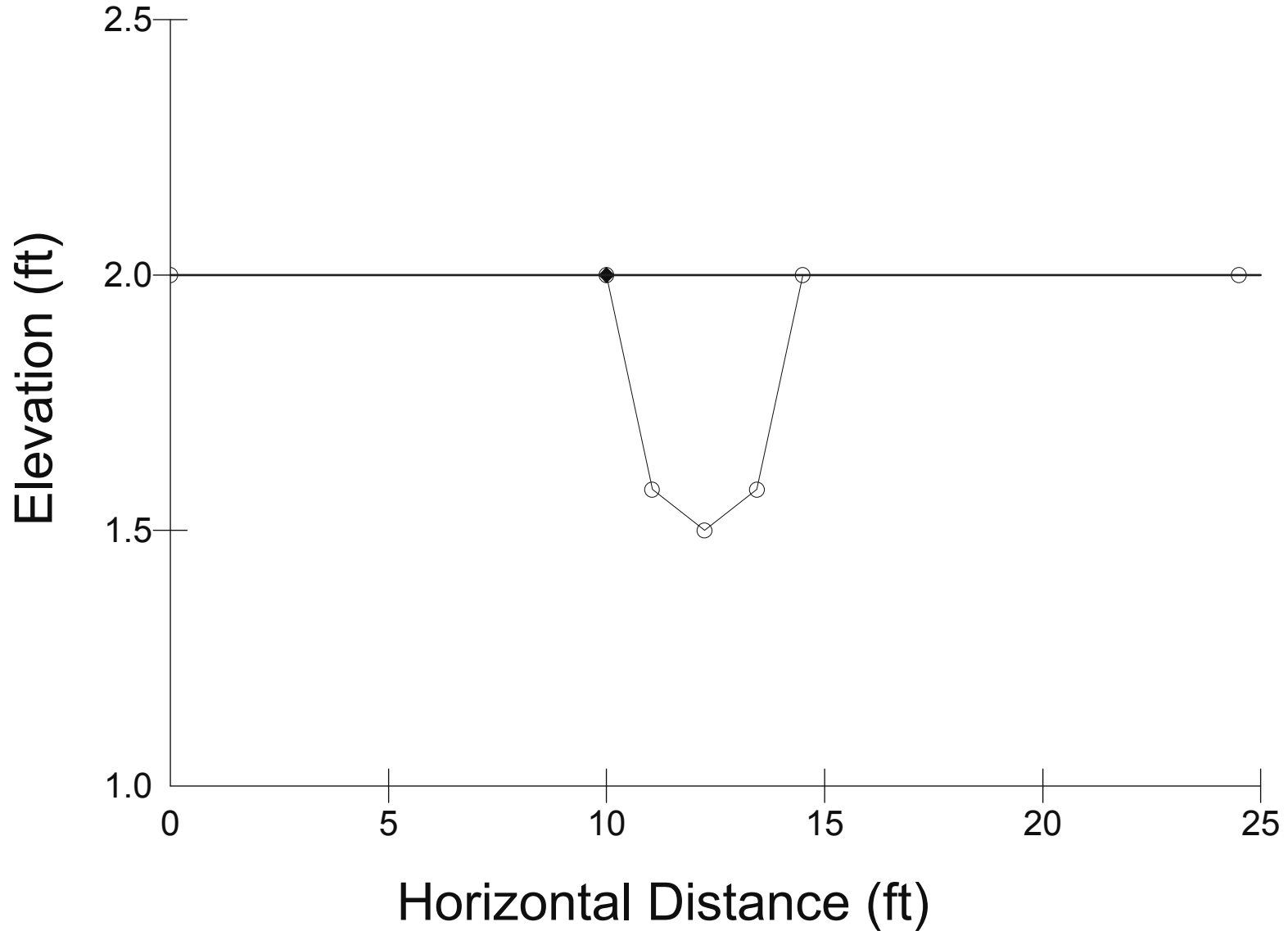
# Typical Section - C Stream

○ Ground Points    ◆ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 4.5

Dbkf = .34

Abkf = 1.55



-----

River Name: Proposed Design  
 Reach Name: North Tributary  
 Cross Section Name: Typical Section - B Stream 10ft  
 Survey Date: 09/18/2018

-----

Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
11.25	0	1.5	
12.75	0	1.5	
14	0	1.2	
16	0	1.2	
17.25	0	1.5	
18.75	0	1.5	
20	0	2	
30	0	2	

-----

Cross Sectional Geometry

-----

	Channel	Left	Right
Floodprone Elevation (ft)	2.8	2.8	2.8
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	30	-----	-----
Bankfull width (ft)	10	5	5
Entrenchment Ratio	3	-----	-----
Mean Depth (ft)	0.53	0.53	0.54
Maximum Depth (ft)	0.8	0.8	0.8
width/Depth Ratio	18.87	9.35	9.26
Bankfull Area (sq ft)	5.35	2.67	2.68
wetted Perimeter (ft)	10.26	5.93	5.93
Hydraulic Radius (ft)	0.52	0.45	0.45
Begin BKF Station	10	10	15
End BKF Station	20	15	20

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Entrainment Calculations

-----

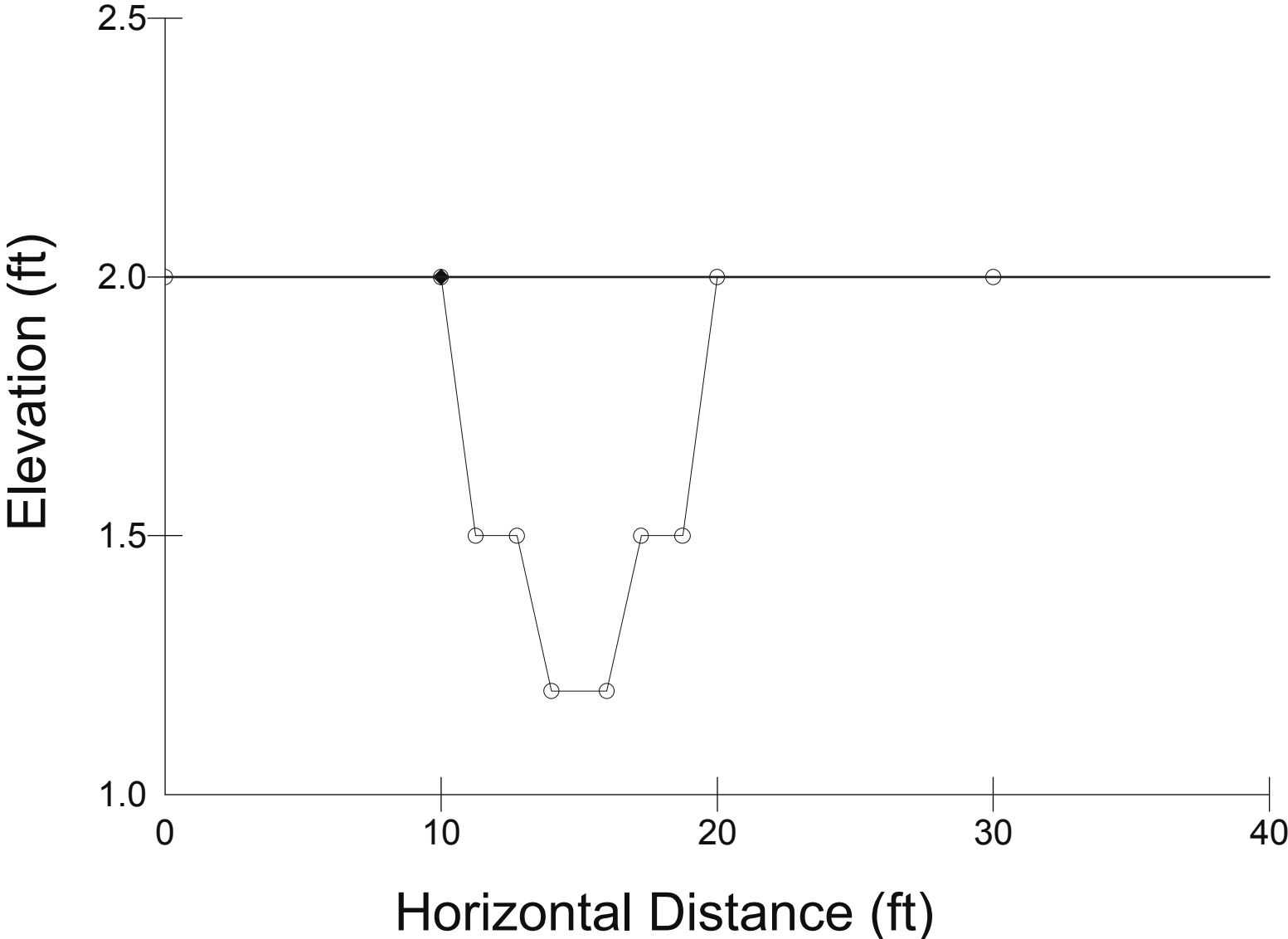
Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

# Typical Section 10ft

○ Ground Points    ◆ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 10      Dbkf = .54      Abkf = 5.35



-----

River Name: Proposed Design  
 Reach Name: North Tributary  
 Cross Section Name: Typical Section - C Stream  
 Survey Date: 09/18/2018

-----

Cross Section Data Entry

BM Elevation: 0 ft  
 Backsight Rod Reading: 0 ft

TAPE	FS	ELEV	NOTE
0	0	2	
10	0	2	BKF
11.15	0	1.54	
13.25	0	1.4	
15.35	0	1.54	
16.5	0	2	
28	0	2	

-----

Cross Sectional Geometry

-----

	Channel	Left	Right
Floodprone Elevation (ft)	2.6	2.6	2.6
Bankfull Elevation (ft)	2	2	2
Floodprone width (ft)	28	-----	-----
Bankfull width (ft)	6.5	4	2.5
Entrenchment Ratio	4.31	-----	-----
Mean Depth (ft)	0.42	0.45	0.38
Maximum Depth (ft)	0.6	0.6	0.55
width/Depth Ratio	15.48	8.85	6.58
Bankfull Area (sq ft)	2.75	1.81	0.95
wetted Perimeter (ft)	6.69	4.64	3.14
Hydraulic Radius (ft)	0.41	0.39	0.3
Begin BKF Station	10	10	14
End BKF Station	16.5	14	16.5

-----

Entrainment Calculations

-----

Entrainment Formula: Rosgen Modified Shields Curve

	Channel	Left Side	Right Side
Slope	0	0	0
Shear Stress (lb/sq ft)			
Movable Particle (mm)			

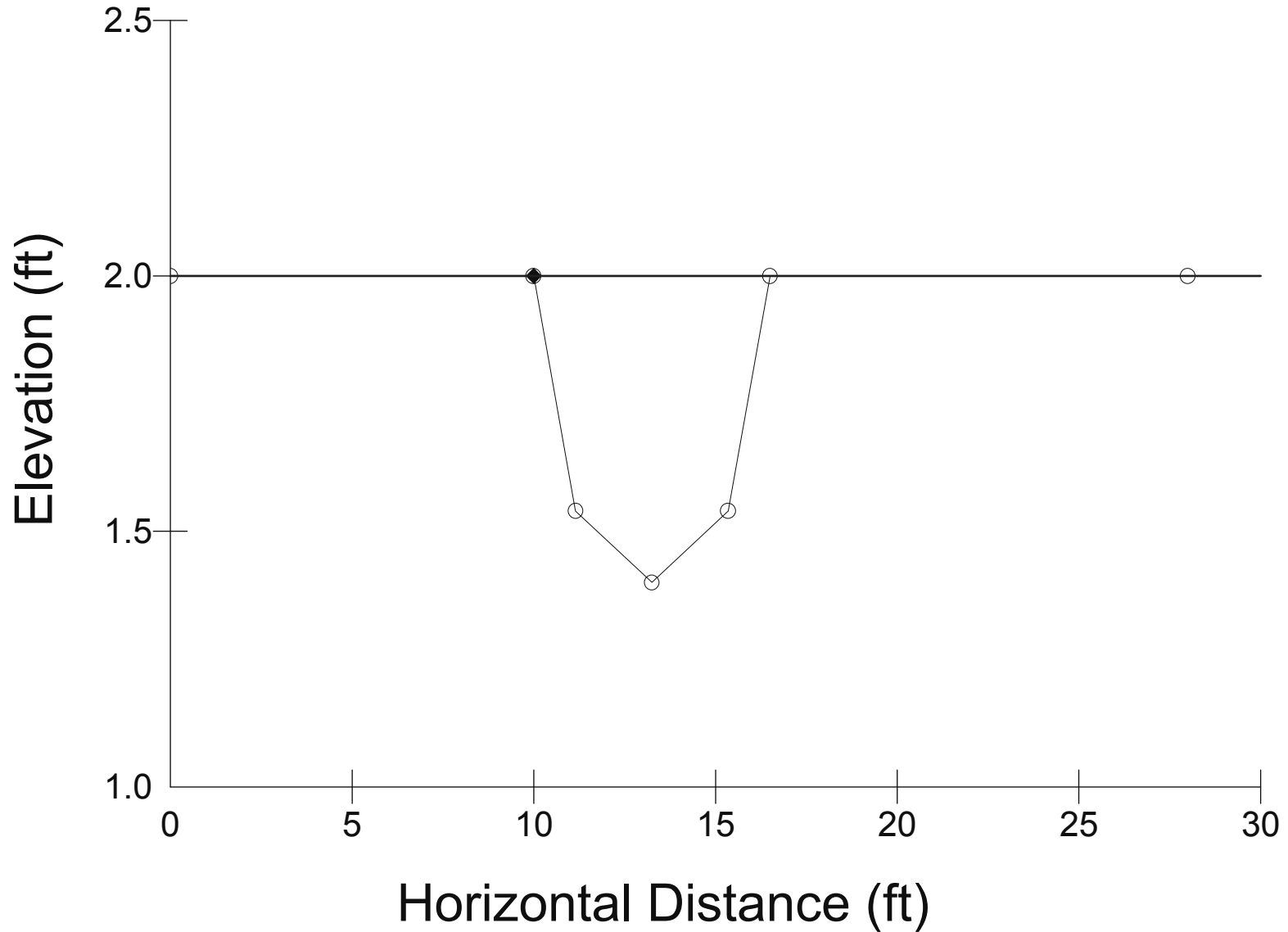
# Typical C Stream Section

○ Ground Points    ◆ Bankfull Indicators    ▼ Water Surface Points

Wbkf = 6.5

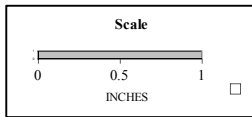
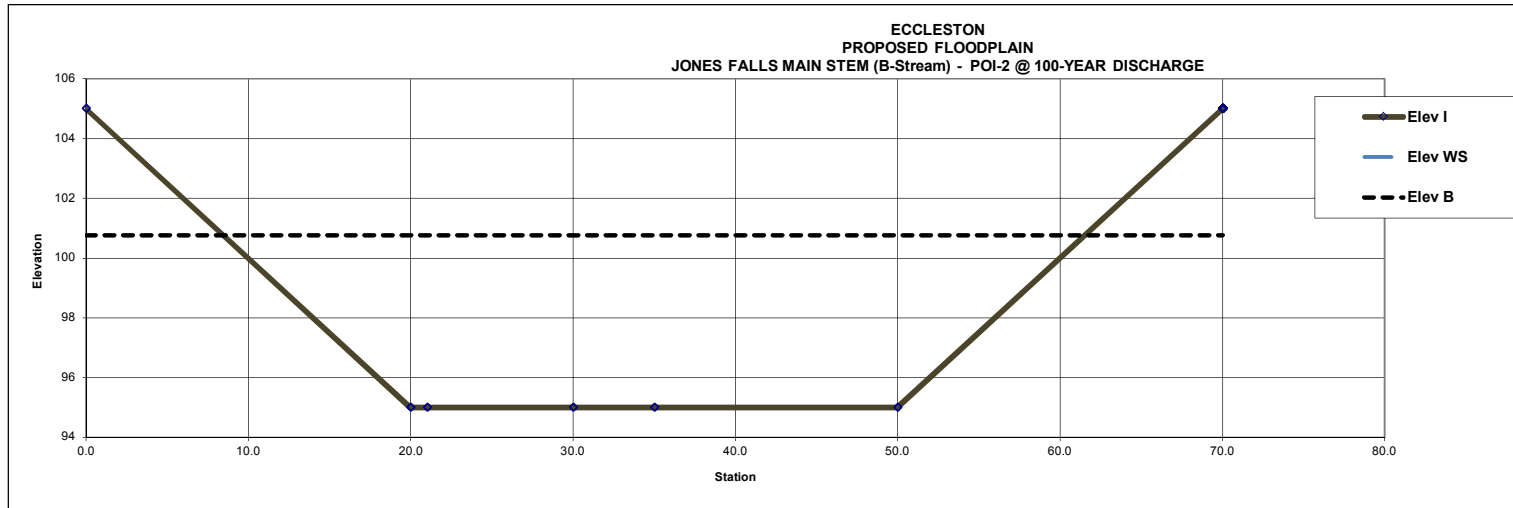
Dbkf = .42

Abkf = 2.76





Proposed Floodplain - Jones Falls Sta. 100+00 to 101+43.02



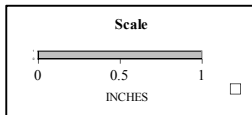
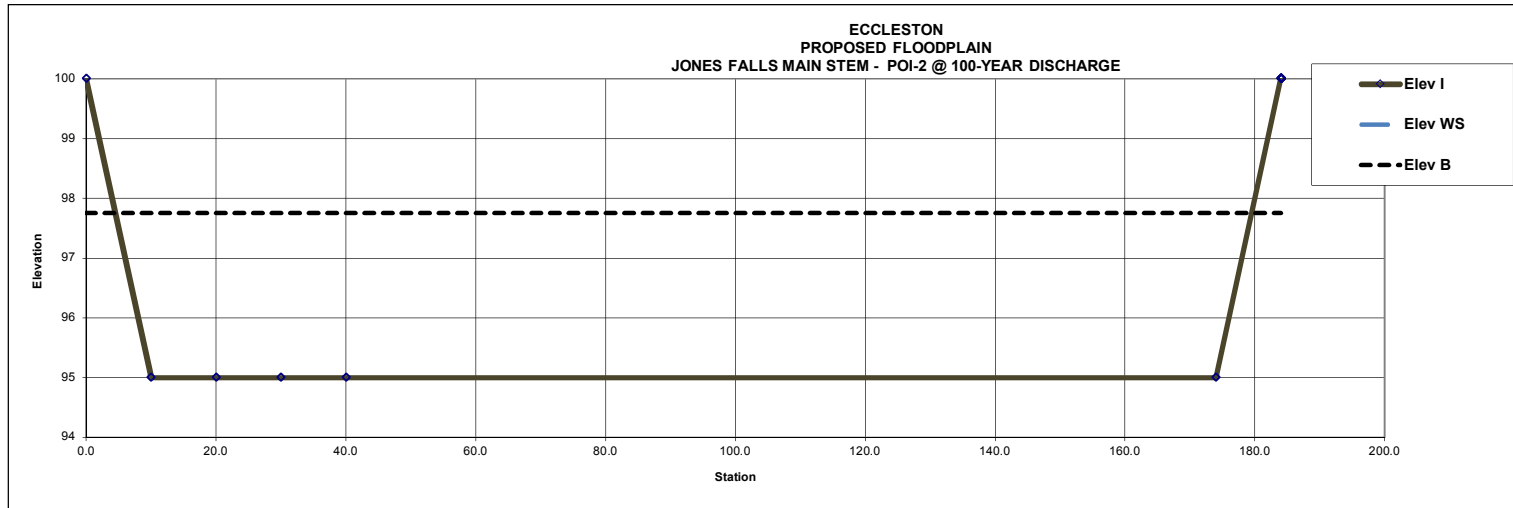
Note: Scale set for print and view magnification at 65%

Plot Scale		
1 inch =		feet horiz.
1 inch =		feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	100.77
Bank Full Width	53.06
Bank Full Depth	4.51
Maximum Depth	5.77
Bank Full Area	239.4
Hydraulic Radius	3.96
Wetted Perimeter	60.5
Width/Depth Ratio	11.8
Flood Prone Width	70.0
Entrenchment	
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0205
Manning's n	0.08
Velocity (ft./s)	6.66
Froud #	0.59
Discharge (cu ft/s)	1593.4
Shear Stress (lb/sq ft)	5.06
CX Power (ft lb/s)	2038.2
Critical Size (mm)	497.1
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - Jones Falls Sta. 101+43.02 to 107+52.45



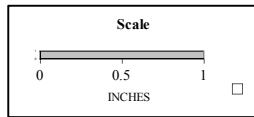
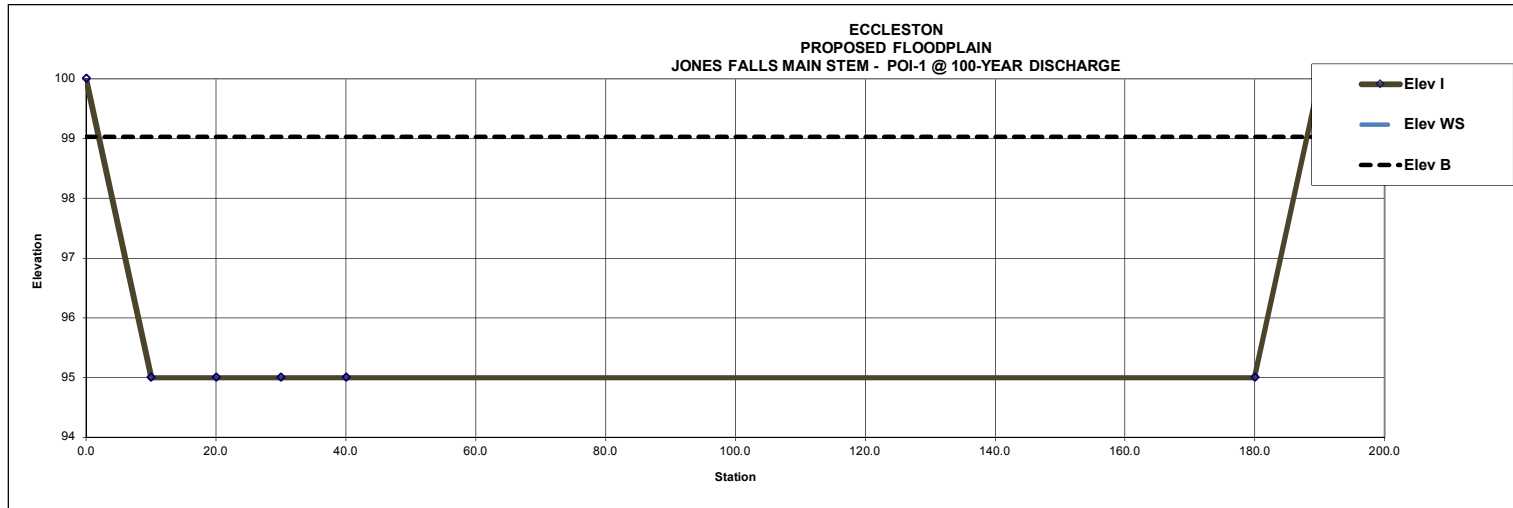
Note: Scale set for print and view magnification at 65%

Plot Scale	
1 inch =	feet horiz.
1 inch =	feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	97.75
Bank Full Width	175.01
Bank Full Depth	2.67
Maximum Depth	2.75
Bank Full Area	466.7
Hydraulic Radius	2.61
Wetted Perimeter	178.9
Width/Depth Ratio	65.6
Flood Prone Width	184.0
Entrenchment	
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0094
Manning's n	0.08
Velocity (ft./s)	3.41
Froud #	0.37
Discharge (cu ft/s)	1593.1
Shear Stress (lb/sq ft)	1.53
CX Power (ft lb/s)	934.5
Critical Size (mm)	136.7
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - Jones Falls Sta. 107+52.45 to 126+76.91  
 \*Minimum width of the proposed floodplain to calculate the maximum shear stress.



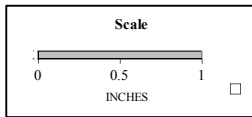
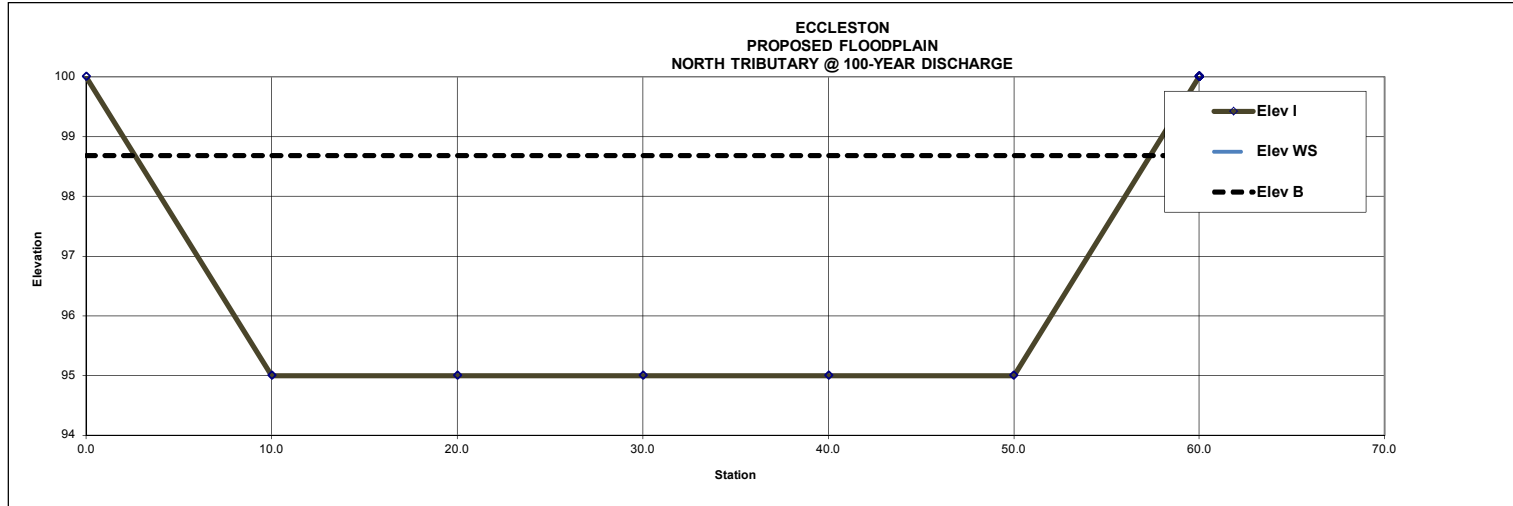
Note: Scale set for print and view magnification at 65%

Plot Scale	
1 inch =	feet horiz.
1 inch =	feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	99.03
Bank Full Width	186.11
Bank Full Depth	3.85
Maximum Depth	4.03
Bank Full Area	717.2
Hydraulic Radius	3.80
Wetted Perimeter	189.0
Width/Depth Ratio	48.3
Flood Prone Width	190.0
Entrenchment	
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0094
Manning's n	0.08
Velocity (ft./s)	4.38
Froud #	0.40
Discharge (cu ft/s)	3144.2
Shear Stress (lb/sq ft)	2.23
CX Power (ft lb/s)	1844.3
Critical Size (mm)	204.9
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - North Tributary 600+00 to 601+35.58



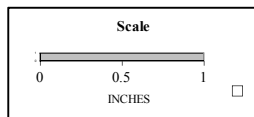
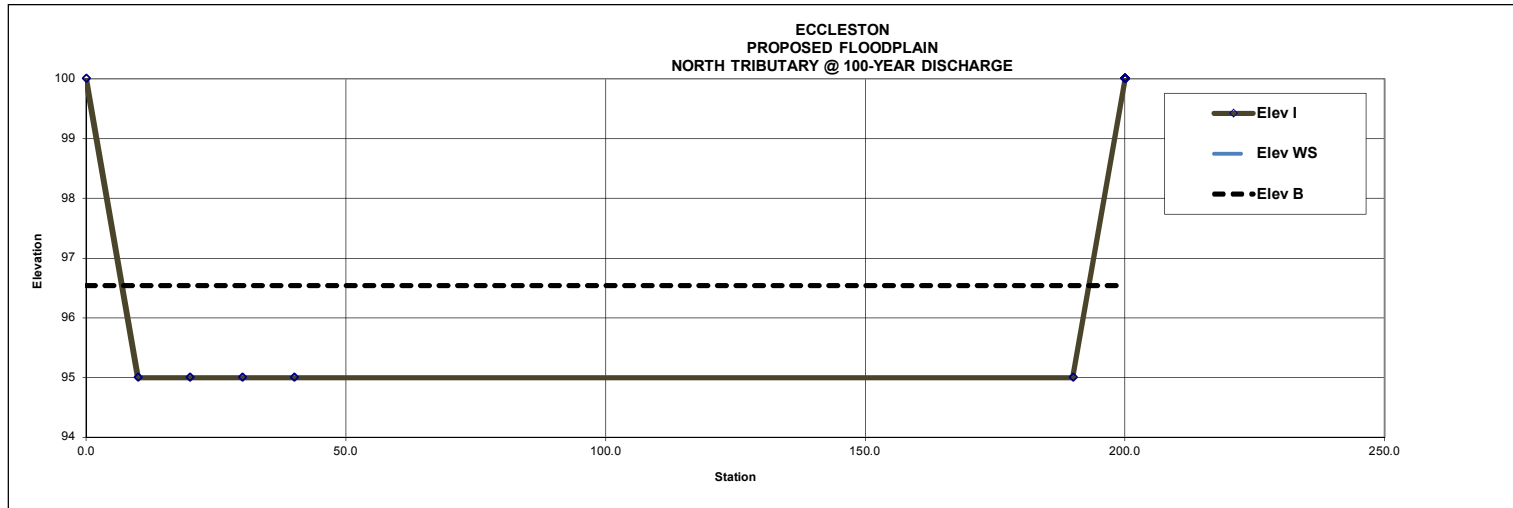
Note: Scale set for print and view magnification at 65%

Plot Scale		
1 inch =		feet horiz.
1 inch =		feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	98.68
Bank Full Width	54.72
Bank Full Depth	3.19
Maximum Depth	3.68
Bank Full Area	174.3
Hydraulic Radius	3.02
Wetted Perimeter	57.8
Width/Depth Ratio	17.2
Flood Prone Width	60.0
Entrenchment	
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0230
Manning's n	0.08
Velocity (ft./s)	5.88
Froud #	0.60
Discharge (cu ft/s)	1025.7
Shear Stress (lb/sq ft)	4.33
CX Power (ft lb/s)	1472.0
Critical Size (mm)	420.0
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - North Tributary 601+35.58 to 607+66.51



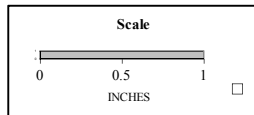
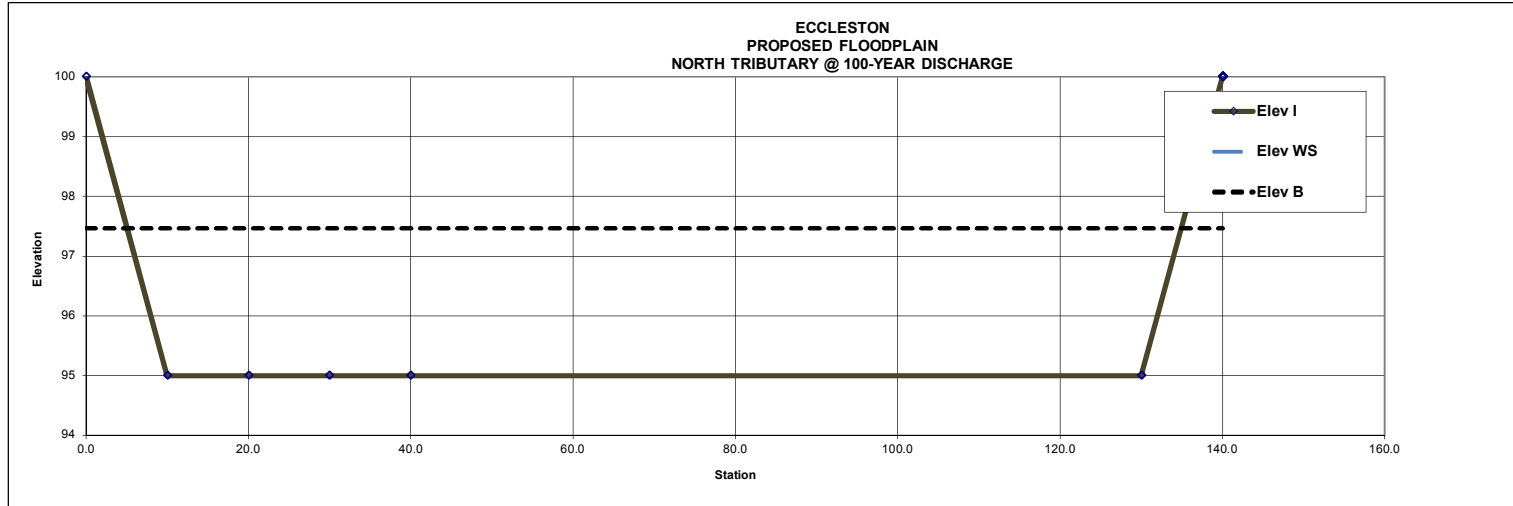
Note: Scale set for print and view magnification at 65%

Plot Scale		
1 inch =		feet horiz.
1 inch =		feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	96.54
Bank Full Width	186.16
Bank Full Depth	1.51
Maximum Depth	1.54
Bank Full Area	281.9
Hydraulic Radius	1.47
Wetted Perimeter	191.7
Width/Depth Ratio	122.9
Flood Prone Width	192.3
Entrenchment	1.03
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0230
Manning's n	0.08
Velocity (ft./s)	3.64
Froud #	0.53
Discharge (cu ft/s)	1027.2
Shear Stress (lb/sq ft)	2.11
CX Power (ft lb/s)	1474.2
Critical Size (mm)	193.4
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - North Tributary 607+66.51 to 614+42.09



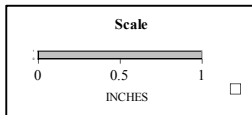
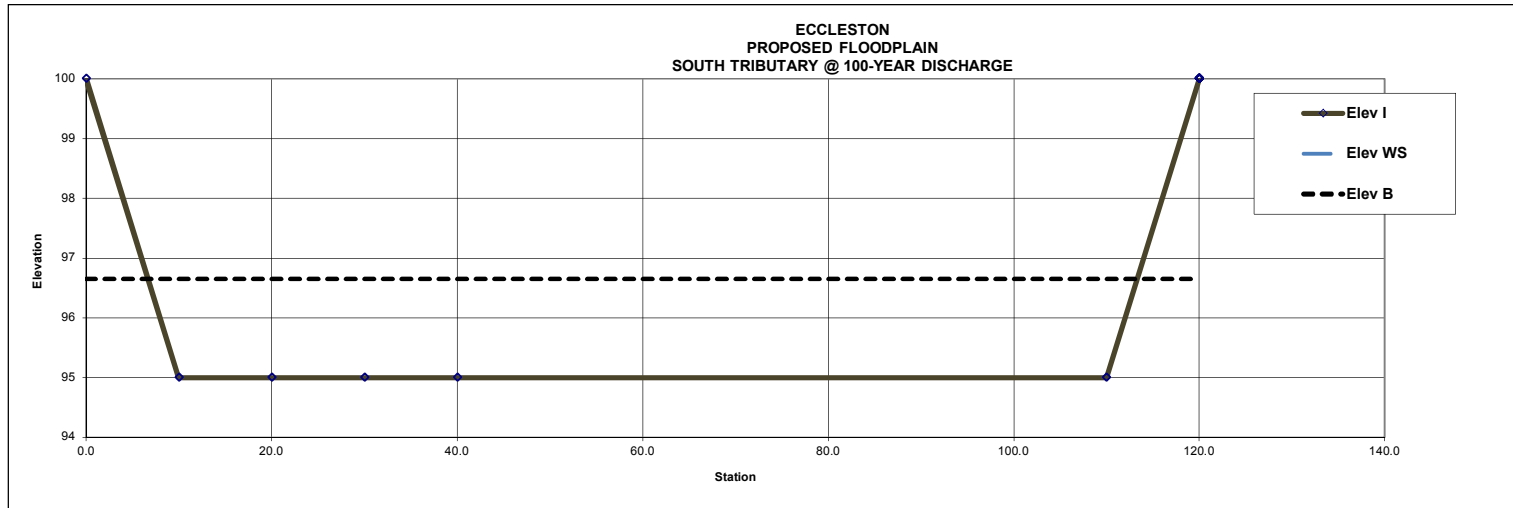
Note: Scale set for print and view magnification at 65%

Plot Scale	
1 inch =	feet horiz.
1 inch =	feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	97.47
Bank Full Width	129.86
Bank Full Depth	2.37
Maximum Depth	2.47
Bank Full Area	308.0
Hydraulic Radius	2.30
Wetted Perimeter	134.0
Width/Depth Ratio	54.8
Flood Prone Width	139.7
Entrenchment	1.08
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0106
Manning's n	0.08
Velocity (ft./s)	3.33
Froud #	0.39
Discharge (cu ft/s)	1025.7
Shear Stress (lb/sq ft)	1.52
CX Power (ft lb/s)	678.4
Critical Size (mm)	135.7
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - South Tributary 200+00 to 207+62.80 and 300+00 to 304+16.88



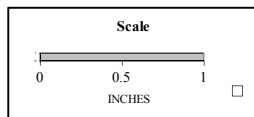
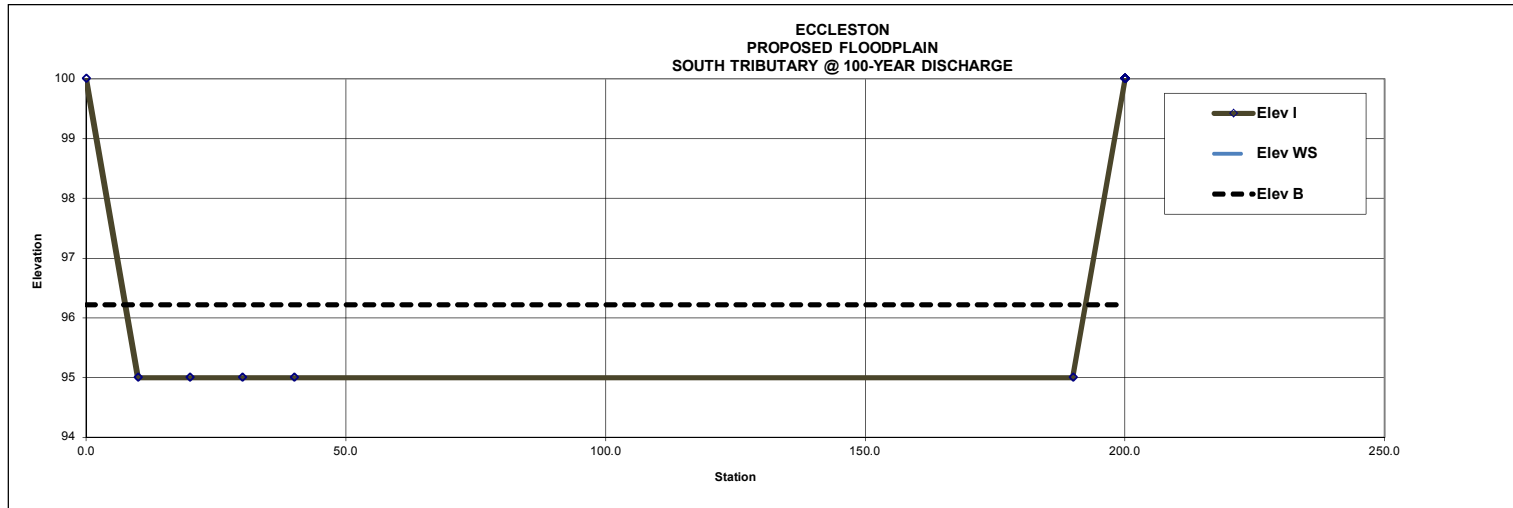
Note: Scale set for print and view magnification at 65%

Plot Scale		
1 inch =		feet horiz.
1 inch =		feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	96.65
Bank Full Width	106.60
Bank Full Depth	1.60
Maximum Depth	1.65
Bank Full Area	170.6
Hydraulic Radius	1.52
Wetted Perimeter	112.0
Width/Depth Ratio	66.6
Flood Prone Width	113.2
Entrenchment	1.06
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0220
Manning's n	0.08
Velocity (ft./s)	3.65
Froud #	0.52
Discharge (cu ft/s)	622.1
Shear Stress (lb/sq ft)	2.09
CX Power (ft lb/s)	854.0
Critical Size (mm)	191.5
Flow Type	Subcritical Flow
Rec. Interval	

Proposed Floodplain - South Tributary 207+62.80 to 211+52.03 and 304+16.88 to 309+72.59



Note: Scale set for print and view magnification at 65%

Plot Scale		
1 inch =		feet horiz.
1 inch =		feet vert.

English Units	
GEOMETRY	
Bank Full Elevation	96.22
Bank Full Width	184.88
Bank Full Depth	1.20
Maximum Depth	1.22
Bank Full Area	222.6
Hydraulic Radius	1.16
Wetted Perimeter	191.1
Width/Depth Ratio	153.6
Flood Prone Width	189.8
Entrenchment	1.03
Stream Type	C4/E4
Rc	

English Units	
HYDRAULICS	
Slope (ft/ft)	0.0186
Manning's n	0.08
Velocity (ft./s)	2.80
Froud #	0.46
Discharge (cu ft/s)	624.2
Shear Stress (lb/sq ft)	1.35
CX Power (ft lb/s)	724.4
Critical Size (mm)	119.6
Flow Type	Subcritical Flow
Rec. Interval	



Intersection Tributary Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft <sup>1/6</sup> )	R/D84	VELOCITY (fps)	U/U*	U <sup>2</sup> /2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.1	0.1	0.03	0.54	0.5	0.05	0.05	0.0218	0.055	0	0.54	2.89	0	0.02	0.07	0.02	0.04	0.43	0
1.2	0.2	0.1	1.08	1	0.09	0.1	0.0218	0.055	0	0.8	3.19	0.01	0.08	0.12	0.11	0.11	0.45	0
1.3	0.3	0.23	1.62	1.5	0.14	0.15	0.0218	0.055	0	1.08	3.43	0.02	0.25	0.19	0.34	0.22	0.49	0
1.4	0.4	0.4	2.15	2	0.19	0.2	0.0218	0.055	0	1.32	3.61	0.03	0.53	0.26	0.72	0.36	0.52	0
1.5	0.5	0.63	2.69	2.5	0.23	0.25	0.0218	0.055	0	1.5	3.73	0.03	0.94	0.31	1.28	0.51	0.53	0.01
1.6	0.6	0.9	3.23	3	0.28	0.3	0.0218	0.055	0	1.71	3.85	0.05	1.54	0.38	2.09	0.7	0.55	0
1.7	0.7	1.22	3.77	3.5	0.32	0.35	0.0218	0.055	0	1.87	3.94	0.05	2.28	0.44	3.1	0.89	0.56	0
1.8	0.8	1.6	4.31	4	0.37	0.4	0.0218	0.055	0	2.06	4.04	0.07	3.29	0.5	4.48	1.12	0.57	0.83

↑  
MAX SHEAR STRESS

**Intersection Trib Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** Intersection Tributary  
**D(50) Riffle:** 152 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.0218 ft/ft

Use Class '0' RipRap with D50 of 6 inches (152 mm) - Shear stress is meets the stress results from the hydraulic analysis (1.01 lb/sf).

\*Slope of the steepest riffle to evaluate the max shear conditions

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	1.11986	1.89 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		1.39 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

Main Stem B Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV	DEPTH	AREA	WET PER	WIDTH	HYD RAD	MEAN D	SLOPE	ROUGH	R/D84	VELOCITY	U/U*	U <sup>2</sup> /2g	DISCHARGE	SHEAR	POWER	POWER/W	FROUDE	TRANSPORT
(ft)	(ft)	(sq ft)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	[n] (ft <sup>1/6</sup> )		(fps)		(ft)	(cfs)	(psf)	(lb/s)	(lb/ft/s)		(lb/s)
1.1	0.1	0.15	3.01	3	0.05	0.05	0.02	0.055	0	0.52	2.89	0	0.08	0.06	0.1	0.03	0.41	0
1.2	0.2	0.6	6.01	6	0.1	0.1	0.02	0.055	0	0.82	3.24	0.01	0.49	0.12	0.62	0.1	0.46	0
1.3	0.3	1.35	9.02	9	0.15	0.15	0.02	0.055	0	1.08	3.47	0.02	1.46	0.19	1.82	0.2	0.49	0
1.4	0.4	2.27	9.56	9.5	0.24	0.24	0.02	0.055	0	1.48	3.75	0.03	3.35	0.3	4.18	0.44	0.53	0
1.5	0.5	3.25	10.1	10	0.32	0.32	0.02	0.055	0	1.79	3.94	0.05	5.81	0.4	7.25	0.73	0.56	0
1.6	0.6	4.28	10.64	10.5	0.4	0.41	0.02	0.055	0	2.08	4.09	0.07	8.88	0.5	11.08	1.06	0.57	2.01
1.7	0.7	5.35	11.17	11	0.48	0.49	0.02	0.055	0	2.34	4.21	0.09	12.54	0.6	15.65	1.42	0.59	5.73
1.8	0.8	6.48	11.71	11.5	0.55	0.56	0.02	0.055	0	2.57	4.31	0.1	16.63	0.69	20.75	1.8	0.6	11.05
1.9	0.9	7.65	12.25	12	0.62	0.64	0.02	0.055	0	2.78	4.4	0.12	21.26	0.77	26.53	2.21	0.61	18.47

MAX SHEAR STRESS

**Main Stem B Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** Mainstem B Stream  
**D(50) Riffle:** 152 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.02 ft/ft

Use Class '0' RipRap with D50 of 6 inches (152 mm) - Shear stress is meets the stress results from the hydraulic analysis (0.77 lb/sf).

\*Slope of the steepest riffle to evaluate the max shear conditions

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	1.11986	1.89 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		1.52 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

Main Stem C Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft <sup>1/6</sup> )	R/D84	VELOCITY (fps)	U/U*	U <sup>2</sup> /2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.3	0.1	0.15	3.01	3	0.05	0.05	0.007	0.055	0	0.32	2.89	0	0.05	0.02	0.02	0.01	0.25	0
1.4	0.2	0.6	6.01	6	0.1	0.1	0.007	0.055	0	0.5	3.24	0	0.3	0.05	0.14	0.02	0.28	0
1.5	0.3	1.23	6.55	6.5	0.19	0.19	0.007	0.055	0	0.77	3.61	0.01	0.95	0.09	0.44	0.07	0.31	0
1.6	0.4	1.9	7.09	7	0.27	0.27	0.007	0.055	0	0.97	3.83	0.01	1.85	0.12	0.85	0.12	0.33	0
1.7	0.5	2.62	7.63	7.5	0.34	0.35	0.007	0.055	0	1.13	3.98	0.02	2.97	0.16	1.37	0.18	0.34	0
1.8	0.6	3.4	8.17	8	0.42	0.43	0.007	0.055	0	1.3	4.12	0.03	4.43	0.19	2.05	0.26	0.35	0
1.9	0.7	4.23	8.71	8.5	0.49	0.5	0.007	0.055	0	1.45	4.23	0.03	6.11	0.23	2.82	0.33	0.36	0

MAX SHEAR STRESS

**Main Stem C Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** Mainstem C Stream  
**D(50) Riffle:** 26.84 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.0074 ft/ft

\*Slope of the steepest riffle to evaluate the max shear conditions

Use Basal Gravel on site with average D50 of 26.84 mm - Shear stress is within acceptable tolerance of the results from the hydraulic analysis (0.23 lb/sf), therefore imported material is not needed.

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	0.19981	0.34 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		0.73 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

North Tributary B Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft^(1/6))	R/D84	VELOCITY (fps)	U/U*	U^2/2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.3	0.1	0.24	2.86	2.83	0.08	0.09	0.0207	0.055	0	0.72	3.13	0.01	0.17	0.1	0.22	0.08	0.42	0
1.4	0.2	0.57	3.71	3.67	0.15	0.15	0.0207	0.055	0	1.1	3.47	0.02	0.63	0.19	0.81	0.22	0.5	0
1.5	0.3	0.97	4.57	4.5	0.21	0.22	0.0207	0.055	0	1.37	3.67	0.03	1.33	0.27	1.72	0.38	0.52	0
1.6	0.4	1.75	8.11	8	0.22	0.22	0.0207	0.055	0	1.42	3.7	0.03	2.48	0.28	3.2	0.4	0.53	0
1.7	0.5	2.57	8.65	8.5	0.3	0.3	0.0207	0.055	0	1.74	3.9	0.05	4.48	0.39	5.78	0.68	0.56	0
1.8	0.6	3.45	9.19	9	0.38	0.38	0.0207	0.055	0	2.04	4.05	0.06	7.04	0.49	9.09	1.01	0.58	1.55
1.9	0.7	4.37	9.73	9.5	0.45	0.46	0.0207	0.055	0	2.28	4.17	0.08	9.98	0.58	12.89	1.36	0.59	4.26

↑  
MAX SHEAR STRESS

**North Trib B Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** North Trib B Stream  
**D(50) Riffle:** 152 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.0207 ft/ft

Use Class '0' RipRap with D50 of 6 inches (152 mm) - Shear stress is meets the stress results from the hydraulic analysis (0.58 lb/sf).

\*Slope of the steepest riffle to evaluate the max shear conditions

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	1.11986	1.89 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		1.46 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$



North Tributary C Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft <sup>1/6</sup> )	R/D84	VELOCITY (fps)	U/U*	U <sup>2</sup> /2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.5	0.1	0.15	3.01	3	0.05	0.05	0.0083	0.055	0	0.33	2.89	0	0.05	0.03	0.03	0.01	0.26	0
1.6	0.2	0.56	4.53	4.5	0.12	0.12	0.0083	0.055	0	0.6	3.35	0.01	0.34	0.06	0.17	0.04	0.3	0
1.7	0.3	1.03	5.07	5	0.2	0.21	0.0083	0.055	0	0.84	3.64	0.01	0.87	0.1	0.45	0.09	0.32	0
1.8	0.4	1.56	5.61	5.5	0.28	0.28	0.0083	0.055	0	1.05	3.85	0.02	1.64	0.14	0.85	0.15	0.35	0
1.9	0.5	2.13	6.15	6	0.35	0.35	0.0083	0.055	0	1.22	4	0.02	2.6	0.18	1.35	0.22	0.36	0



MAX SHEAR STRESS

**North Trib C Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** North Tributary C Stream  
**D(50) Riffle:** 26.84 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.008 ft/ft

\*Slope of the steepest riffle to evaluate the max shear conditions

Use Basal Gravel on site with average D50 of 26.84 mm - Shear stress is within acceptable tolerance of the results from the hydraulic analysis (0.18 lb/sf), therefore imported material is not needed.

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	0.19981	0.34 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		0.68 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

South Tributary B Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft <sup>1/6</sup> )	R/D84	VELOCITY (fps)	U/U*	U <sup>2</sup> /2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.4	0.1	0.12	1.54	1.5	0.08	0.08	0.0218	0.055	0	0.74	3.13	0.01	0.09	0.11	0.12	0.08	0.46	0
1.5	0.2	0.3	2.08	2	0.14	0.15	0.0218	0.055	0	1.08	3.43	0.02	0.32	0.19	0.44	0.22	0.49	0
1.6	0.3	0.53	2.62	2.5	0.2	0.21	0.0218	0.055	0	1.36	3.64	0.03	0.72	0.27	0.98	0.39	0.52	0
1.7	0.4	0.8	3.15	3	0.25	0.27	0.0218	0.055	0	1.58	3.78	0.04	1.27	0.34	1.72	0.57	0.54	0
1.8	0.5	1.38	6.19	6	0.22	0.23	0.0218	0.055	0	1.45	3.7	0.03	2.01	0.3	2.73	0.46	0.53	0
1.9	0.6	2	6.73	6.5	0.3	0.31	0.0218	0.055	0	1.79	3.9	0.05	3.58	0.41	4.87	0.75	0.57	0



**South Trib B Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** South Trib B Stream  
**D(50) Riffle:** 152 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.0218 ft/ft

Use Class '0' RipRap with D50 of 6 inches (152 mm) - Shear stress is meets the stress results from the hydraulic analysis (0.41 lb/sf).

\*Slope of the steepest riffle to evaluate the max shear conditions

<i>Andrews 1984 Methodology</i>		<i>Andrews 1995 Methodology</i>	
<u>Tc* (1984)</u>	<u>Tc (1984)</u>	<u>Tc* (1995)</u>	<u>Tc (1995)</u>
#VALUE!	#VALUE! lb/sf	1.11986	1.89 lb/sf
<u>Depth (1984)</u>		<u>Depth (1995)</u>	
#VALUE! ft.		1.39 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

South Tributary C Stream Typical Riffle Section Q-Qs output for determining max shear stress within channel

ELEV (ft)	DEPTH (ft)	AREA (sq ft)	WET PER (ft)	WIDTH (ft)	HYD RAD (ft)	MEAN D (ft)	SLOPE (ft/ft)	ROUGH [n] (ft^(1/6))	R/D84	VELOCITY (fps)	U/U*	U^2/2g (ft)	DISCHARGE (cfs)	SHEAR (psf)	POWER (lb/s)	POWER/W (lb/ft/s)	FROUDE	TRANSPORT (lb/s)
1.6	0.1	0.15	2.51	2.5	0.06	0.06	0.012	0.055	0	0.45	2.98	0	0.07	0.04	0.05	0.02	0.33	0
1.7	0.2	0.42	3.05	3	0.14	0.14	0.012	0.055	0	0.8	3.43	0.01	0.34	0.1	0.25	0.08	0.38	0
1.8	0.3	0.74	3.59	3.5	0.21	0.21	0.012	0.055	0	1.05	3.67	0.02	0.77	0.16	0.58	0.17	0.4	0
1.9	0.4	1.12	4.13	4	0.27	0.28	0.012	0.055	0	1.24	3.83	0.02	1.39	0.2	1.04	0.26	0.41	0

↑  
MAX SHEAR STRESS

**South Trib C Stream Section Bankfull Channel - Critical Shear**

**Sediment Transport Analysis  
Andrews Methodology**

**Project:** Eccleston Mitigation Site  
**Reach:** South Trib C Stream  
**D(50) Riffle:** 26.84 mm  
**D(50) Bar/Sub:** n/a mm  
**Mobile Size (Di):** 5 mm = 0.016 ft.  
**\*Slope:** 0.012 ft/ft

\*Slope of the steepest riffle to evaluate the max shear conditions

Use Basal Gravel on site with average D50 of 26.84 mm - Shear stress is within acceptable tolerance of the results from the hydraulic analysis (0.20 lb/sf), therefore imported material is not needed.

<b>Andrews 1984 Methodology</b>		<b>Andrews 1995 Methodology</b>	
<b><u>Tc* (1984)</u></b>	<b><u>Tc (1984)</u></b>	<b><u>Tc* (1995)</u></b>	<b><u>Tc (1995)</u></b>
#VALUE!	#VALUE! lb/sf	0.19981	0.34 lb/sf
<b><u>Depth (1984)</u></b>		<b><u>Depth (1995)</u></b>	
#VALUE! ft.		0.45 ft.	

**Andrews 1984 Methodology:**

$$Tc^* = 0.0834 \times [(D50(\text{riffle}) / D50(\text{bar}))^{-0.872}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

**Andrews 1995 Methodology:**

$$Tc^* = 0.0376 \times [(Di / D50(\text{riffle}))^{-0.994}]$$

$$Tc = Tc^* \times 1.65 \times 62.4 \times Di$$

$$\text{Depth} = (Tc^* \times 1.65 \times Di) / \text{Slope}$$

Outlet Protection of 60" RCP at Intersection Tributary

Type I - Preformed Scour Hole (Depressed 1/2 Culvert Rise)

Using Empirical Preformed Scour Hole Equations from MDSHA Highway Drainage Manual, I-3-E-2:

$D_{50} = (0.0125d^2/TW)(Q_{10}/d^{2.5})^{1.333}$	where:	D <sub>50</sub> = Median stone diameter (ft)		
		d = Pipe diameter (ft)	=	5
D <sub>50</sub> = 1.22 ft		***TW = Tailwater depth (ft)	=	2
		**Q = Design pipe flow (cfs)	=	261
C = Basin Length (ft)	=	30		
A = Basin Inlet Width (ft)	=	25		
B = Basin Outlet Width (ft)	=	25		
D = Basin Riprap Thickness (ft)	=	2.4373 *		
E = Culvert Span (ft)	=	5		
F = Basin Depression (ft)	=	2.5		
d = Culvert Diameter or Span (ft)	=	5		
D <sub>50</sub> = Median Stone Diameter (ft)	=	1.22		

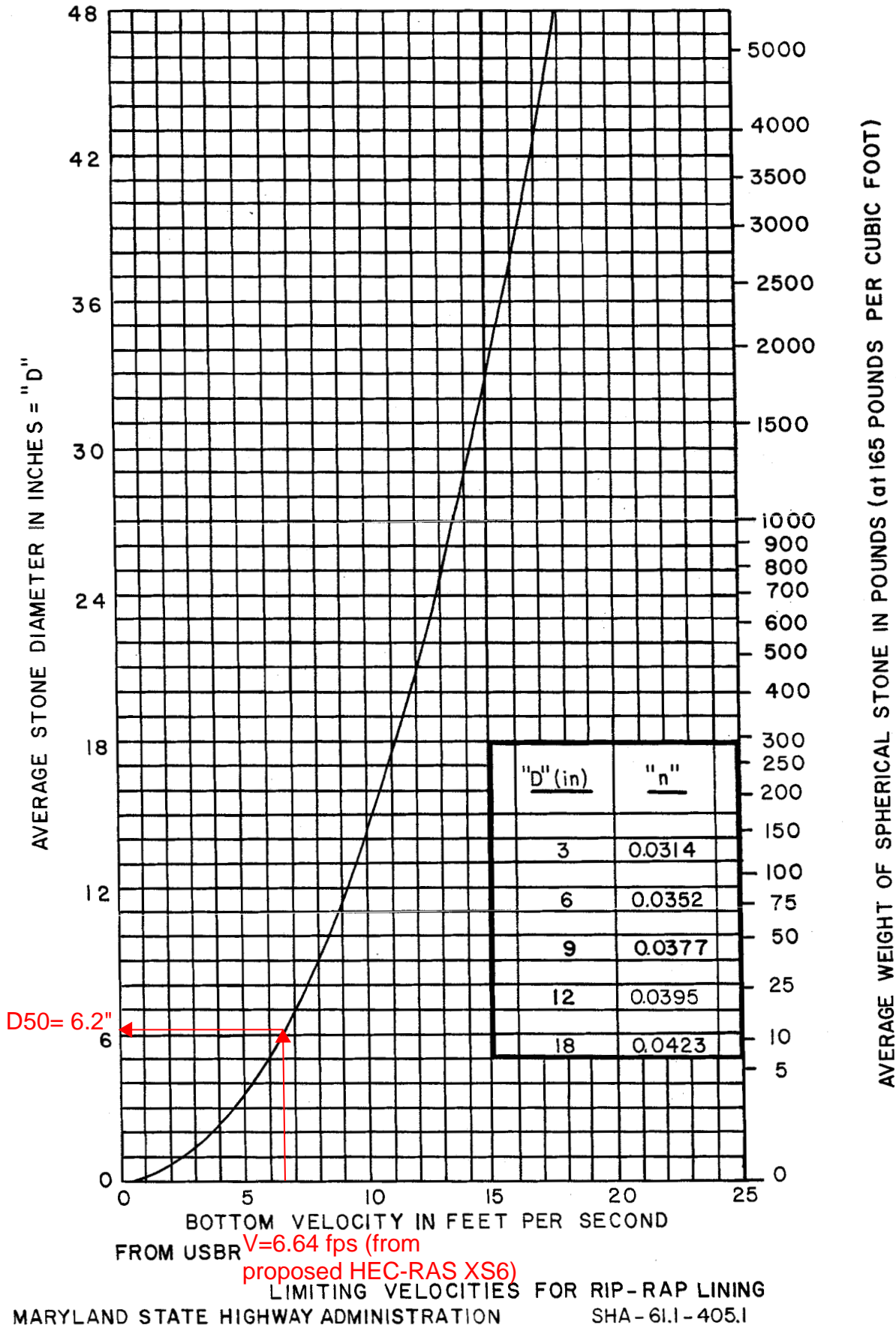
CLASS 'II' RIPRAP WITH D50 OF 16 INCHES IS PROPOSED

**NOTES**

\*\* Q100 Value

\*\*\*Assumed Tailwater. Value to be revised once 100-year elevation is computed.

Riprap Sizing for Scour Hole at South Tributary  
 This supports that the proposed Class 'I' riprap (d50=9.5") is within the acceptable tolerance for the channel velocities.





Riprap Channel Stabilization Computations								
Location	Drainage Area, A (ac)	Runoff Coefficient, C*	Rainfall Intensity, I (in/hr)**	Q10=CIA (cfs)	Velocity, v=Q/A (ft/s)***	d50 (in)****	Proposed Riprap Size	Riprap d50 (in)
RP-1	3.00	0.26	5.1	3.98	5.60	4.5"	Class 'I'	9.5
RP-2	3.19	0.28	5.1	4.56	6.01	5.0"	Class 'I'	9.5

\*Runoff Coefficient from Table SHA 61.1-401.0. (Residential 1 acre, Type B soil, 6%+ and Residential 1 acre, Type C soil, 6%

\*\*Rainfall Intensity from MDE Stormwater Sizing Criteria Table 2.2

\*\*\*Velocity from Rivermorph Q-Qs Output over channel cell

\*\*\*\*d50 determined from Isbash curve

## RIPRAP CHANNEL STABILIZATION COMPUTATIONS FOR RP-1 AND RP-2

### Sizing techniques

There are many techniques for sizing stone, and each method has advantages and disadvantages. Many techniques were derived under specific conditions and developed for particular applications. While this list is not complete and the description is not exhaustive, several commonly used methods are presented. The designer should review the applicability of a technique before choosing it to size stone for a particular project. Following is a brief description of several rock sizing techniques.

#### Isbash method

The Isbash formula (Isbash 1936) was developed for the construction of dams by depositing rocks into moving water. The Isbash curve should only be used for quick estimates or for comparisons. A coefficient is provided to target high- and low-turbulence flow conditions, so this method can be a high- or low-energy application. The equation is:

$$V_c = C \times \left( 2 \times g \times \frac{\gamma_s - \gamma_w}{\gamma_w} \right)^{0.50} \times (D_{50})^{0.50} \quad (\text{eq. TS14C-1})$$

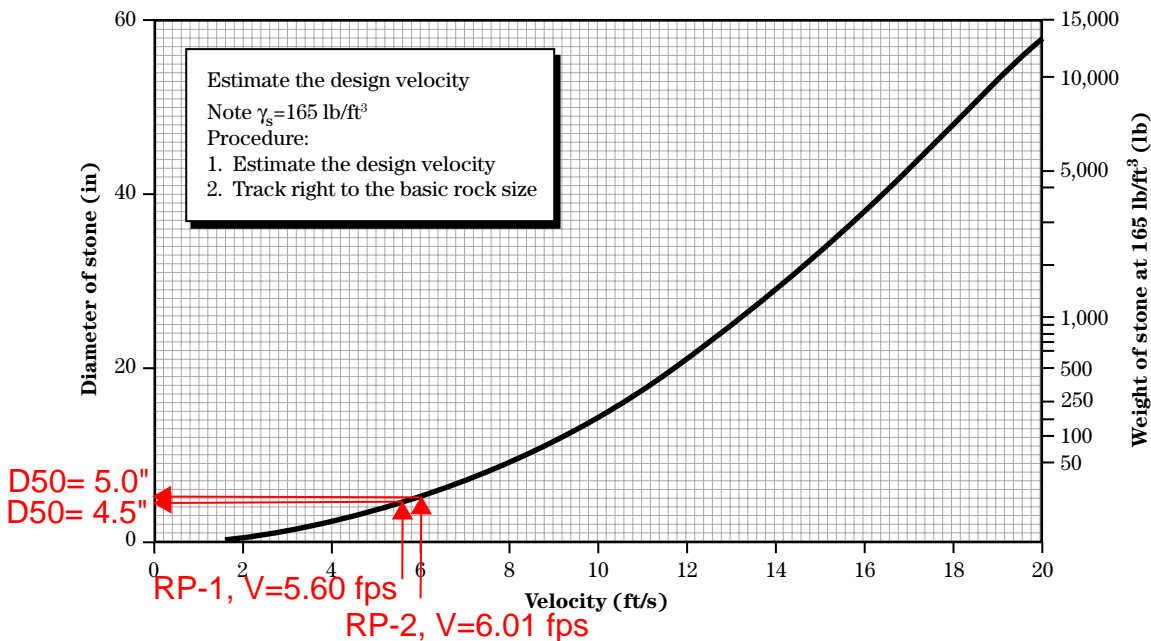
where:

- $V_c$  = critical velocity (ft/s)
- $C$  = 0.86 for high turbulence
- $C$  = 1.20 for low turbulence
- $g$  = 32.2 ft/s<sup>2</sup>
- $\gamma_s$  = stone density (lb/ft<sup>3</sup>)
- $\gamma_w$  = water density (lb/ft<sup>3</sup>)
- $D_{50}$  = median stone diameter (ft)

A graphical solution is provided in figure TS14C-5 (ch. 16 of the Engineering Field Manual) This graph should be used only for quick estimates at a conceptual design level.

The U.S. Army Corps of Engineers (USACE) provides additional guidance for the use of the Isbash technique in EM 1110-2-1601. The required inputs are channel velocity, specific gravity of the stone, and a turbulence coefficient. The turbulence coefficient has two values that represent either high turbulence or low turbulence. The graphical solution for this is shown in figure TS14C-6(a) and (b).

**Figure TS14C-5** Rock size based on Isbash curve





## APPENDIX H

### Hydraulics Analysis Data

**Stream and Location:** Eccleston

**Reach:** South Tributary

**Event for which n is assigned:** Bankfull flow condition

**1. Is roughness uniform throughout the reach being considered?** Yes

**If not, should n be assigned for the average condition of the reach?** N/A

**2. Is roughness uniformly distributed along the cross section?** Yes

**Is a division between channel and floodplain necessary?** Yes

**(Channel Roughness uses steps 3-13, Floodplain roughness uses steps 14-23.)**

**Is roughness uniformly distributed across the channel?** Yes

**If not, what basis should n for the individual segments be weighted?** N/A

**3. Describe the channel. Are present conditions representative of those during the flood?**

A silt/sand bed stream with coarse gravel exhibiting a riffle / pool system.

**If not, describe the probable conditions during the flood.** Assume that channel conditions are representative of channel flowing full (to elevation of the floodplain/bankfull condition)

**4. How will the roughness-producing effects of the following on the channel be accounted for?**

**Bank roughness:** \_\_\_\_\_

**Bedrock outcrops:** No bedrock present.

**Isolated boulders:** No boulders present.

**Vegetation:** Mostly herbaceous near channel. Mix of woody and herbaceous vegetation in floodplain.

**Obstructions:** Woody debris jams.

**Meander:** Little to No sinuosity

**5-10. Computation n for the channel**

Segment Number and Material	Approximate Dimensions		Wetted perimeter	Area (sf)	Grain Size (mm)	Base n for segment	Adjustments	Adjusted n	Weight Factors	times weight factors
	width (ft)	depth (ft)								
1					37.42	0.032	None	N/A	N/A	0.032
<b>SUM</b>										0.032
<b>Weighted n =</b>										<b>0.032</b>

**11-13. Computation n for the channel  
Adjustment factors for the channel**

Factor	Describe conditions briefly	Adjustment
<b>Irregularity, n<sub>1</sub></b>	Minor - Compares to carefully degraded channels in good condition but having slightly eroded or scoured side slopes.	0.005
<b>Alignment, n<sub>2</sub></b>	Alternating occasionally- Large and small cross sections alternate occasionally, or the main flow occasionally shifts from side to side owing to changes in cross-sectional shape.	0.003
<b>Obstructions, n<sub>3</sub></b>	Minor- Obstructions less than 15 percent of the cross-sectional area, and the spacing between obstructions is such that the sphere of influence around one obstruction does not extend to the sphere of influence around another obstruction.	0.007
<b>Vegetation, n<sub>4</sub></b>	Medium- Brushy, moderately dense vegetation, similar to 1-to-2-year-old willow trees in the dormant season, growing along the banks, and no significant vegetation is evident along the channel bottoms	0.010
<b>Meander, m</b>	Minor- Ratio of the channel length to valley length is 1.0 to 1.2	1.00
<b>Weighted n plus n<sub>x</sub> adjustments</b>		0.057
<b>Computed n times m factor =</b>		<b>0.057</b>

**Stream and Location:** Eccleston

**Reach:** North Tributary

**Event for which n is assigned:** Bankfull flow condition

**1. Is roughness uniform throughout the reach being considered?** Yes

**If not, should n be assigned for the average condition of the reach?** N/A

**2. Is roughness uniformly distributed along the cross section?** Yes

**Is a division between channel and floodplain necessary?** Yes

**(Channel Roughness uses steps 3-13, Floodplain roughness uses steps 14-23.)**

**Is roughness uniformly distributed across the channel?** Yes

**If not, what basis should n for the individual segments be weighted?** N/A

**3. Describe the channel. Are present conditions representative of those during the flood?**

A silt/sand bed stream with coarse gravel exhibiting a riffle / pool system

**If not, describe the probable conditions during the flood.** Assume that channel conditions are representative of channel flowing full (to elevation of the floodplain/bankfull condition)

**4. How will the roughness-producing effects of the following on the channel be accounted for?**

**Bank roughness:** \_\_\_\_\_

**Bedrock outcrops:** No bedrock present.

**Isolated boulders:** No boulders present.

**Vegetation:** Mostly herbaceous near channel. Mix of woody and herbaceous vegetation in floodplain.

**Obstructions:** Woody debris jams.

**Meander:** Little to No sinuosity

**5-10. Computation n for the channel**

Segment Number and Material	Approximate Dimensions		Wetted perimeter	Area (sf)	Grain Size (mm)	Base n for segment	Adjustments	Adjusted n	Weight Factors	times weight factors
	width (ft)	depth (ft)								
1					26.67	0.031	None	N/A	N/A	0.031
									<b>SUM</b>	0.031
									<b>Weighted n =</b>	<b>0.031</b>

**11-13. Computation n for the channel  
Adjustment factors for the channel**

Factor	Describe conditions briefly	Adjustment
<b>Irregularity, n<sub>1</sub></b>	Minor - Compares to carefully degraded channels in good condition but having slightly eroded or scoured side slopes.	0.005
<b>Alignment, n<sub>2</sub></b>	Alternating occasionally- Large and small cross sections alternate occasionally, or the main flow occasionally shifts from side to side owing to changes in cross-sectional shape.	0.003
<b>Obstructions, n<sub>3</sub></b>	Minor- Obstructions less than 15 percent of the cross-sectional area, and the spacing between obstructions is such that the sphere of influence around one obstruction does not extend to the sphere of influence around another obstruction.	0.007
<b>Vegetation, n<sub>4</sub></b>	Medium- Brushy, moderately dense vegetation, similar to 1-to-2-year-old willow trees in the dormant season, growing along the banks, and no significant vegetation is evident along the channel bottoms	0.015
<b>Meander, m</b>	Minor- Ratio of the channel length to valley length is 1.0 to 1.2	1.00
		<b>Weighted n plus n<sub>x</sub> adjustments</b>
		0.061
		<b>Computed n times m factor =</b>
		<b>0.061</b>

**Stream and Location:** Eccleston

**Reach:** Main Stem

**Event for which n is assigned:** Bankfull flow condition

**1. Is roughness uniform throughout the reach being considered?** Yes

**If not, should n be assigned for the average condition of the reach?** N/A

**2. Is roughness uniformly distributed along the cross section?** Yes

**Is a division between channel and floodplain necessary?** Yes

**(Channel Roughness uses steps 3-13, Floodplain roughness uses steps 14-23.)**

**Is roughness uniformly distributed across the channel?** Yes

**If not, what basis should n for the individual segments be weighted?** N/A

**3. Describe the channel. Are present conditions representative of those during the flood?**

A silt/sand bed stream with coarse gravel exhibiting a riffle / pool system.

**If not, describe the probable conditions during the flood.** Assume that channel conditions are representative of channel flowing full (to elevation of the floodplain/bankfull condition)

**4. How will the roughness-producing effects of the following on the channel be accounted for?**

**Bank roughness:** \_\_\_\_\_

**Bedrock outcrops:** No bedrock present.

**Isolated boulders:** No boulders present.

**Vegetation:** Mostly herbaceous near channel. Mix of woody and herbaceous vegetation in floodplain.

**Obstructions:** Woody debris jams.

**Meander:** Little to No sinuosity



**5-10. Computation n for the channel**

Segment Number and Material	Approximate Dimensions		Wetted perimeter	Area (sf)	Grain Size (mm)	Base n for segment	Adjustments	Adjusted n	Weight Factors	times weight factors
	width (ft)	depth (ft)								
1					36.892	0.032	None	N/A	N/A	0.032
<b>SUM</b>										0.032
<b>Weighted n =</b>										<b>0.032</b>

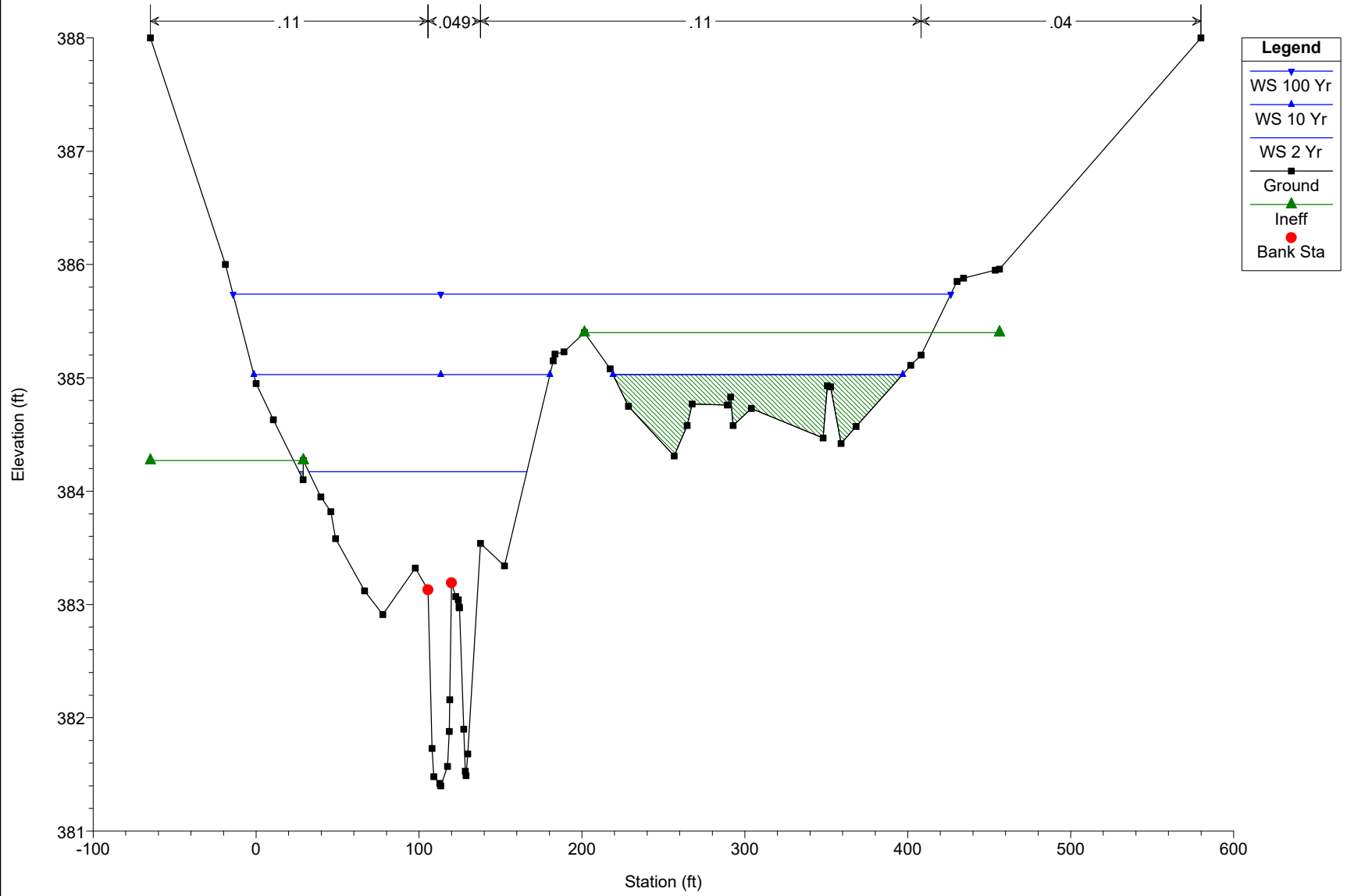
**11-13. Computation n for the channel  
Adjustment factors for the channel**

Factor	Describe conditions briefly	Adjustment
<b>Irregularity, n<sub>1</sub></b>	Minor - Compares to carefully degraded channels in good condition but having slightly eroded or scoured side slopes.	0.005
<b>Alignment, n<sub>2</sub></b>	Alternating occasionally- Large and small cross sections alternate occasionally, or the main flow occasionally shifts from side to side owing to changes in cross-sectional shape.	0.003
<b>Obstructions, n<sub>3</sub></b>	Minor- Obstructions less than 15 percent of the cross-sectional area, and the spacing between obstructions is such that the sphere of influence around one obstruction does not extend to the sphere of influence around another obstruction.	0.004
<b>Vegetation, n<sub>4</sub></b>	Small- the average depth of flow is atleast two times the height of the vegetation	0.005
<b>Meander, m</b>	Minor- Ratio of the channel length to valley length is 1.0 to 1.2	1.00
<b>Weighted n plus n<sub>x</sub> adjustments</b>		0.049
<b>Computed n times m factor =</b>		<b>0.049</b>

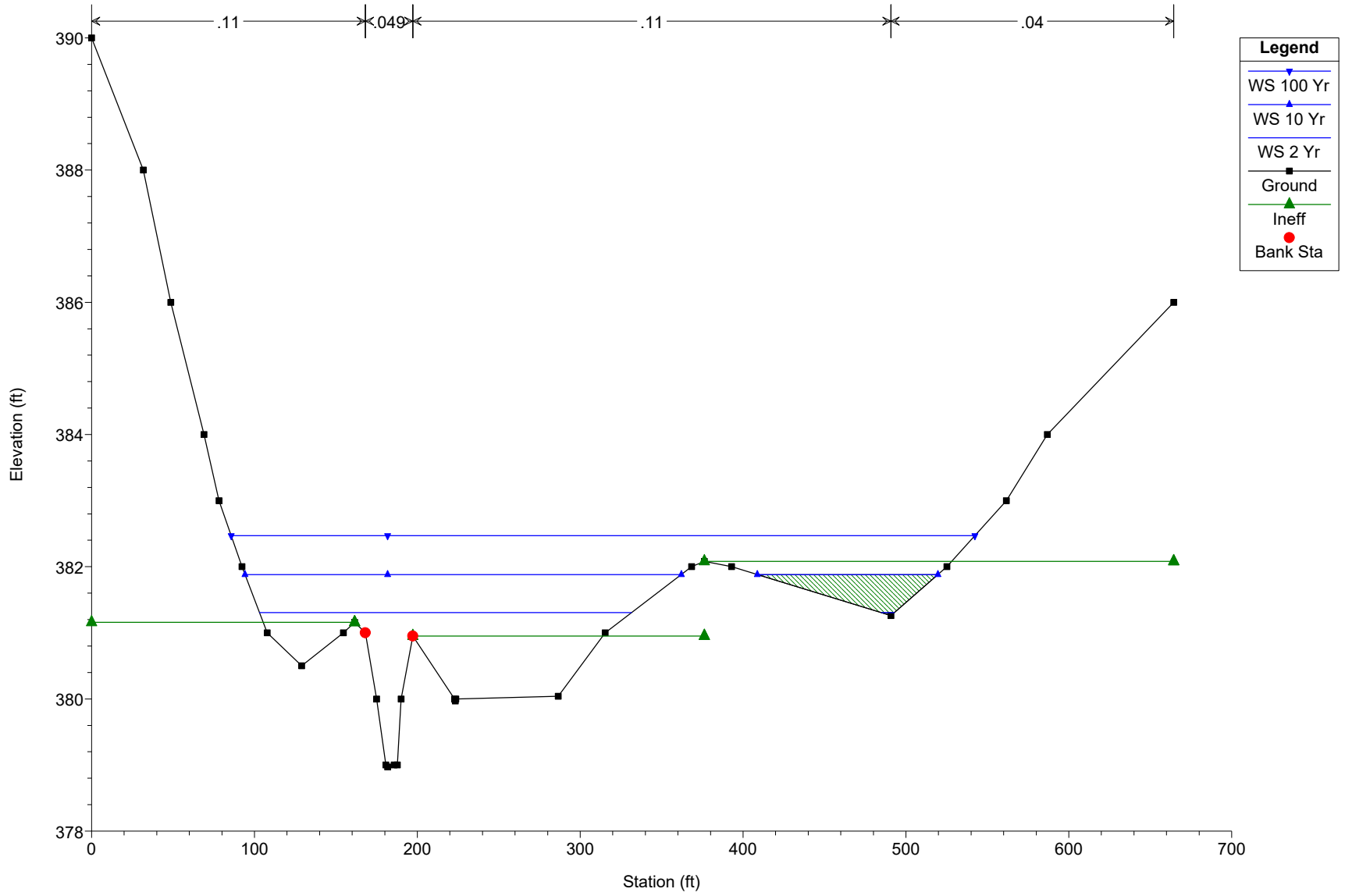
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = JonesFalls RS = 15



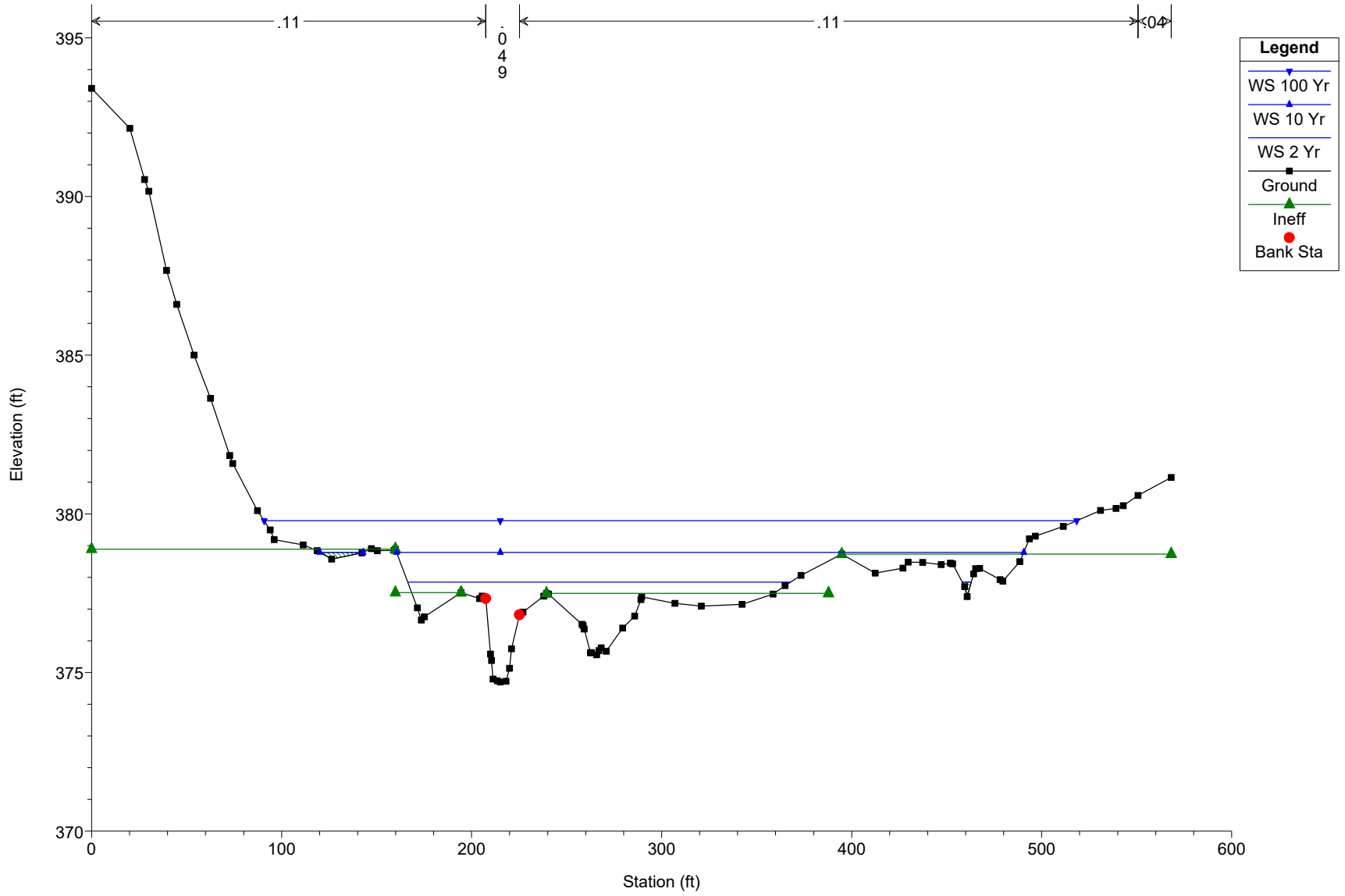
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EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

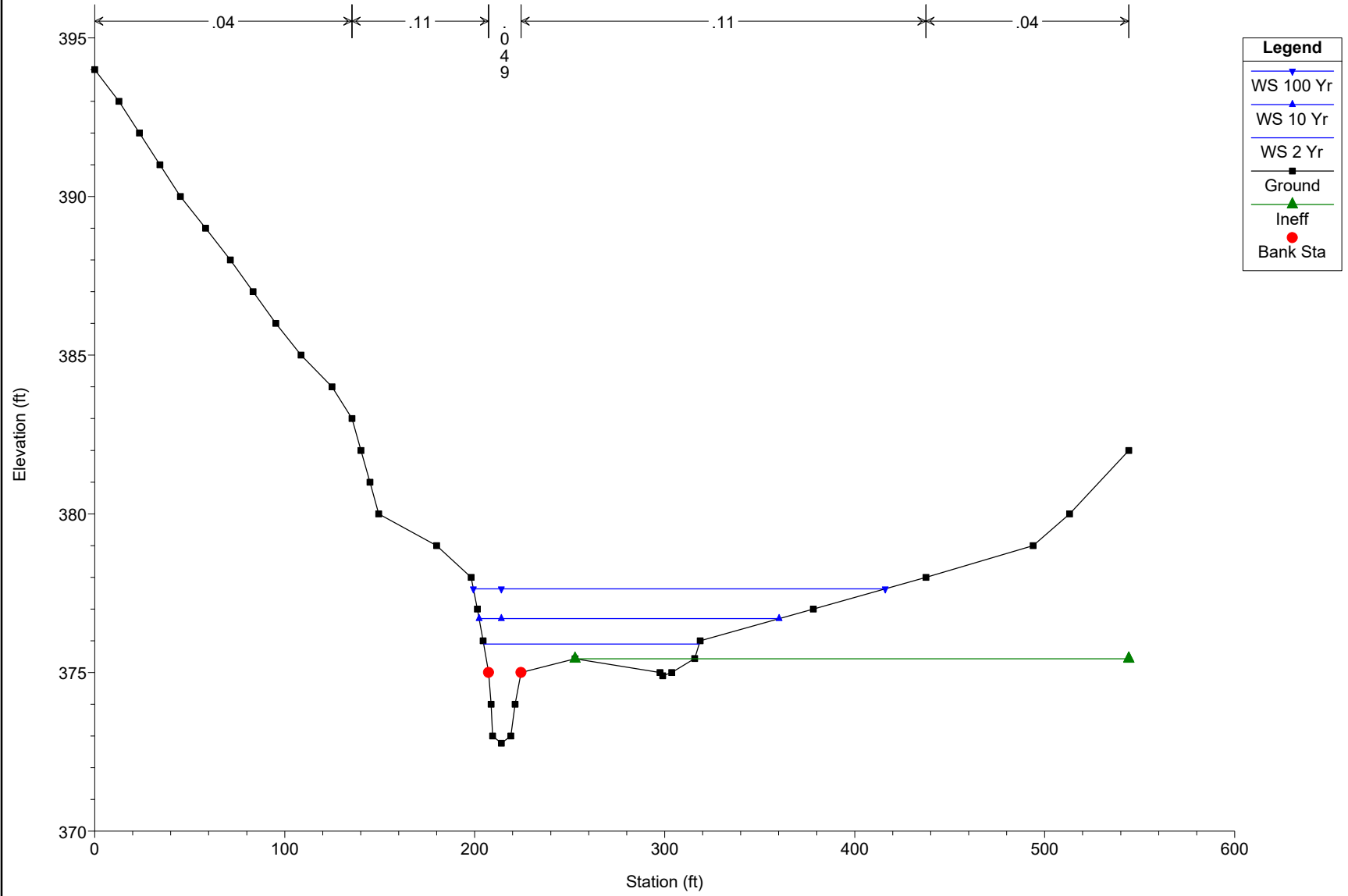
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EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

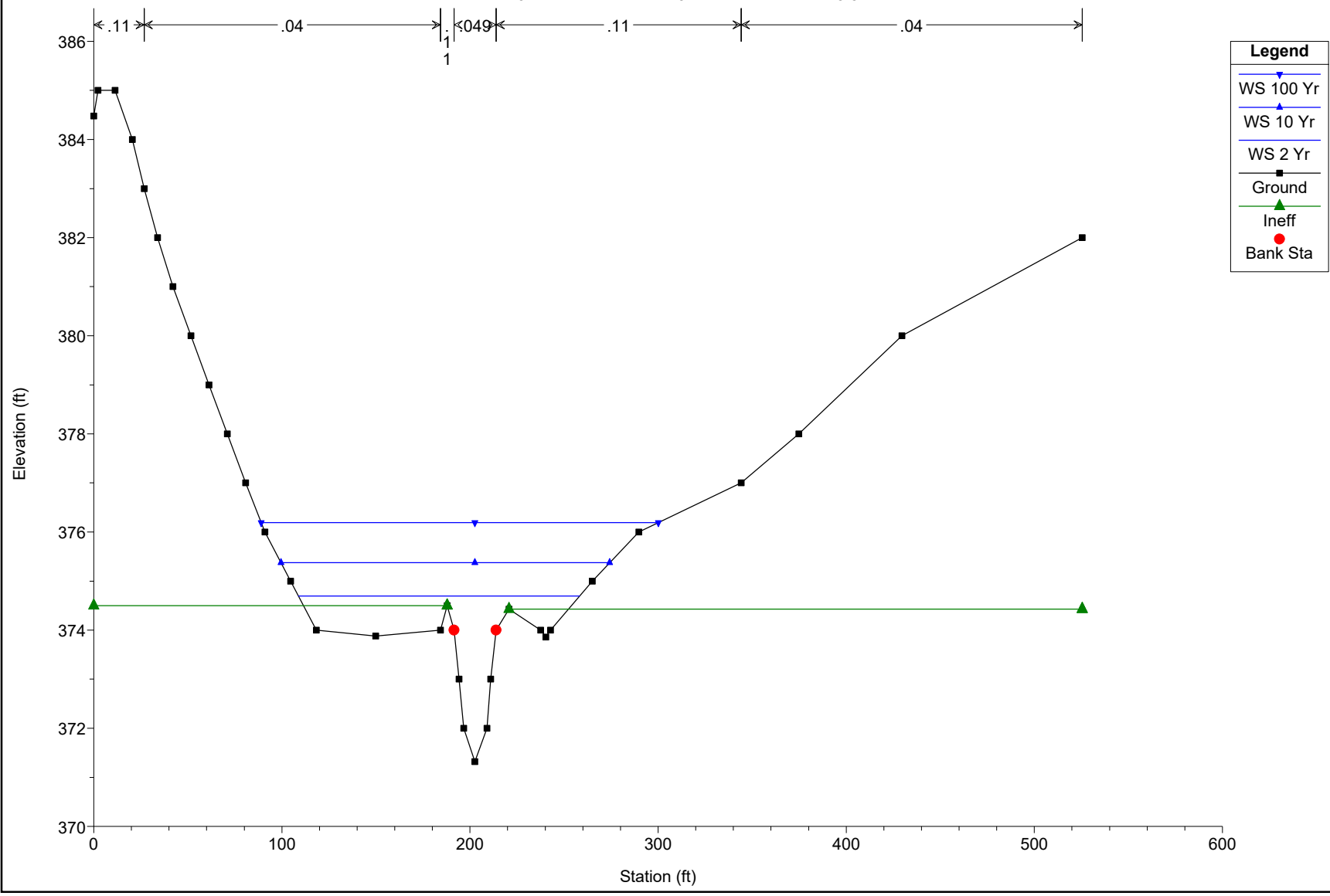
River = JonesFalls Reach = JonesFalls RS = 13.6



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

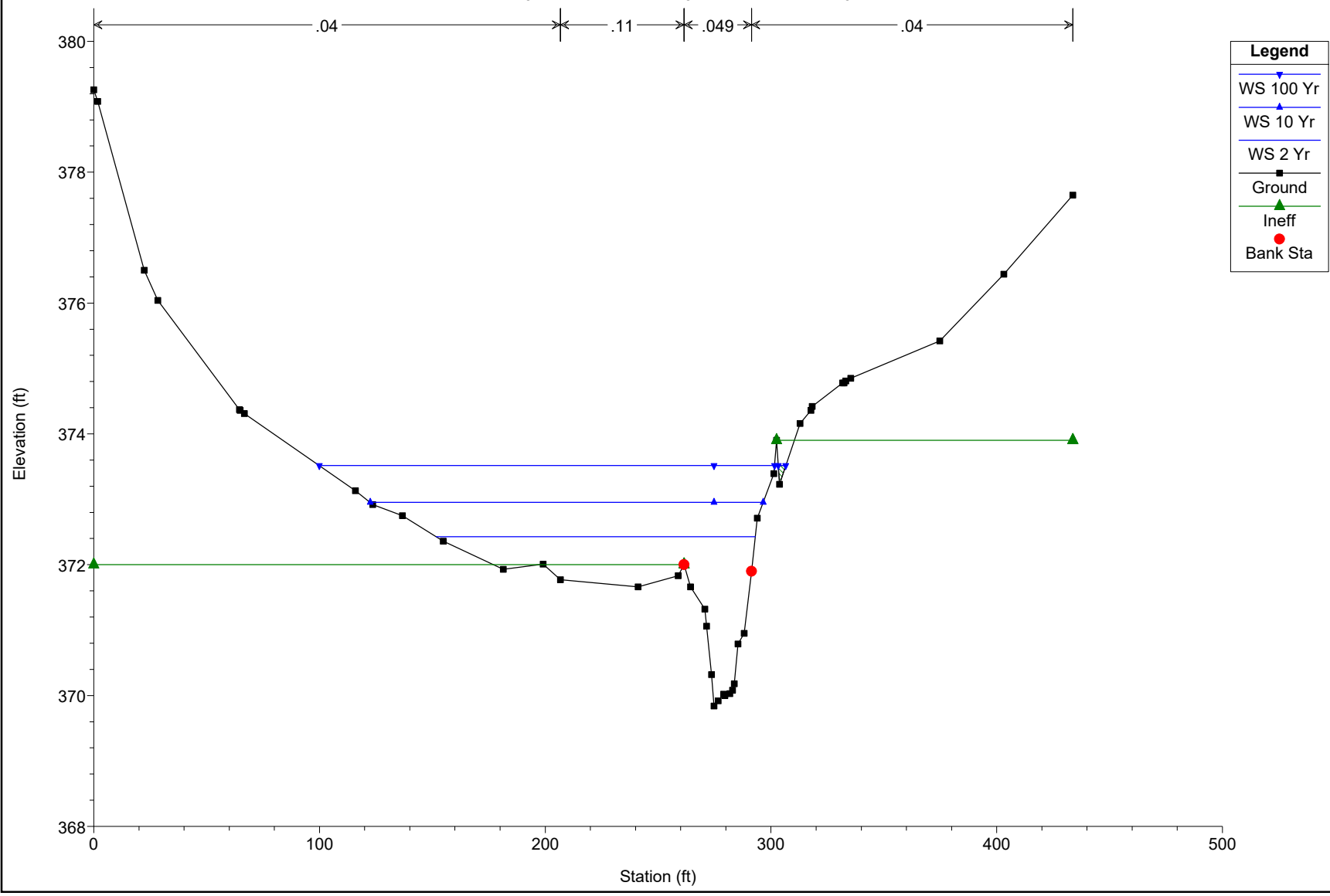
River = JonesFalls Reach = JonesFalls RS = 13.5



**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Ineff
- Bank Sta

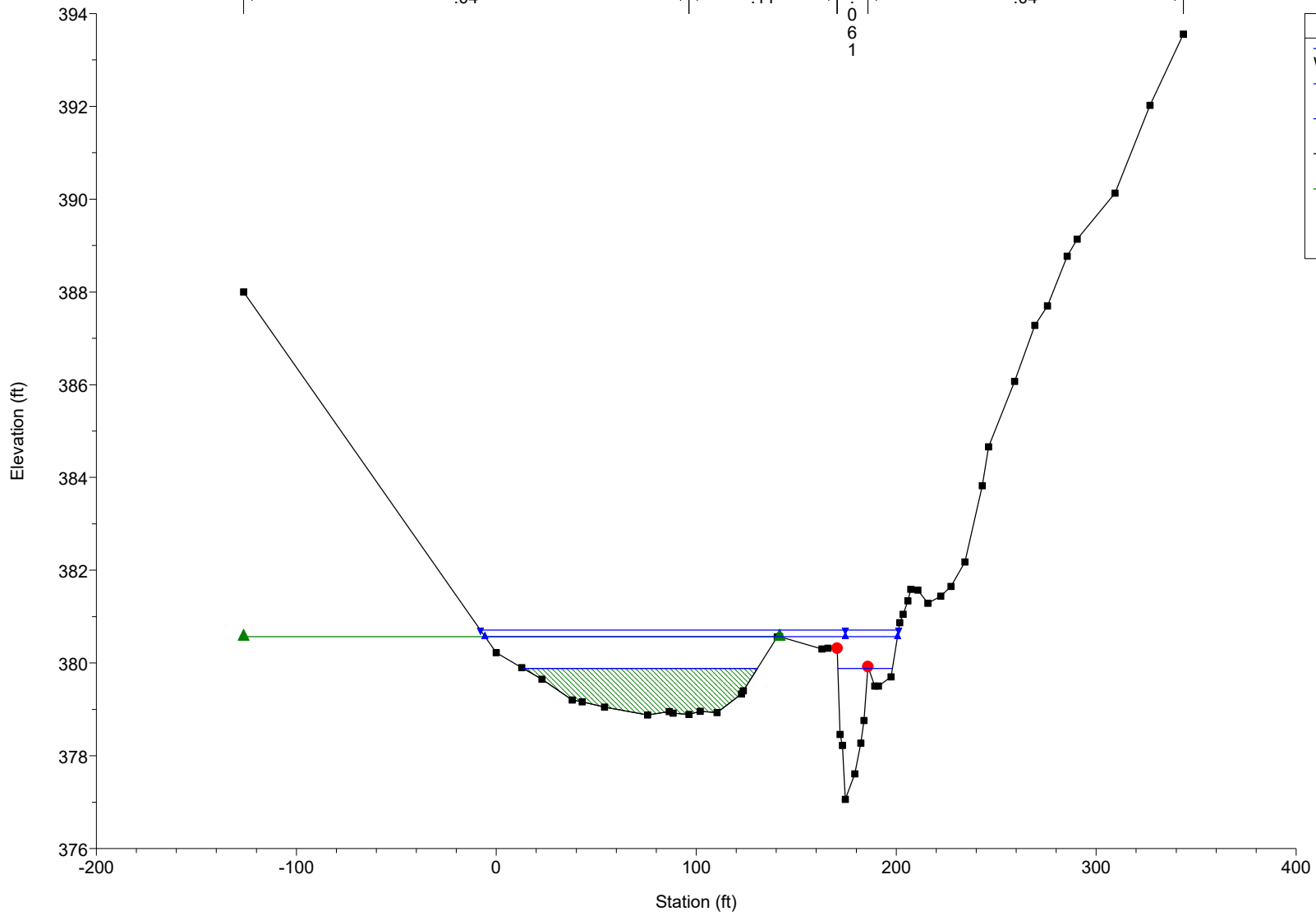
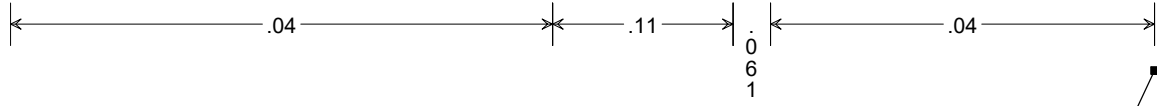
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 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls RS = 13



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = NorthTrib RS = 2



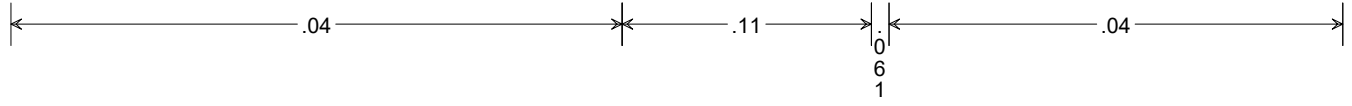
Legend	
WS 100 Yr	Blue line with downward triangle
WS 10 Yr	Blue line with upward triangle
WS 2 Yr	Blue line with square
Ground	Black line with square
Ineff	Green line with upward triangle
Bank Sta	Red circle



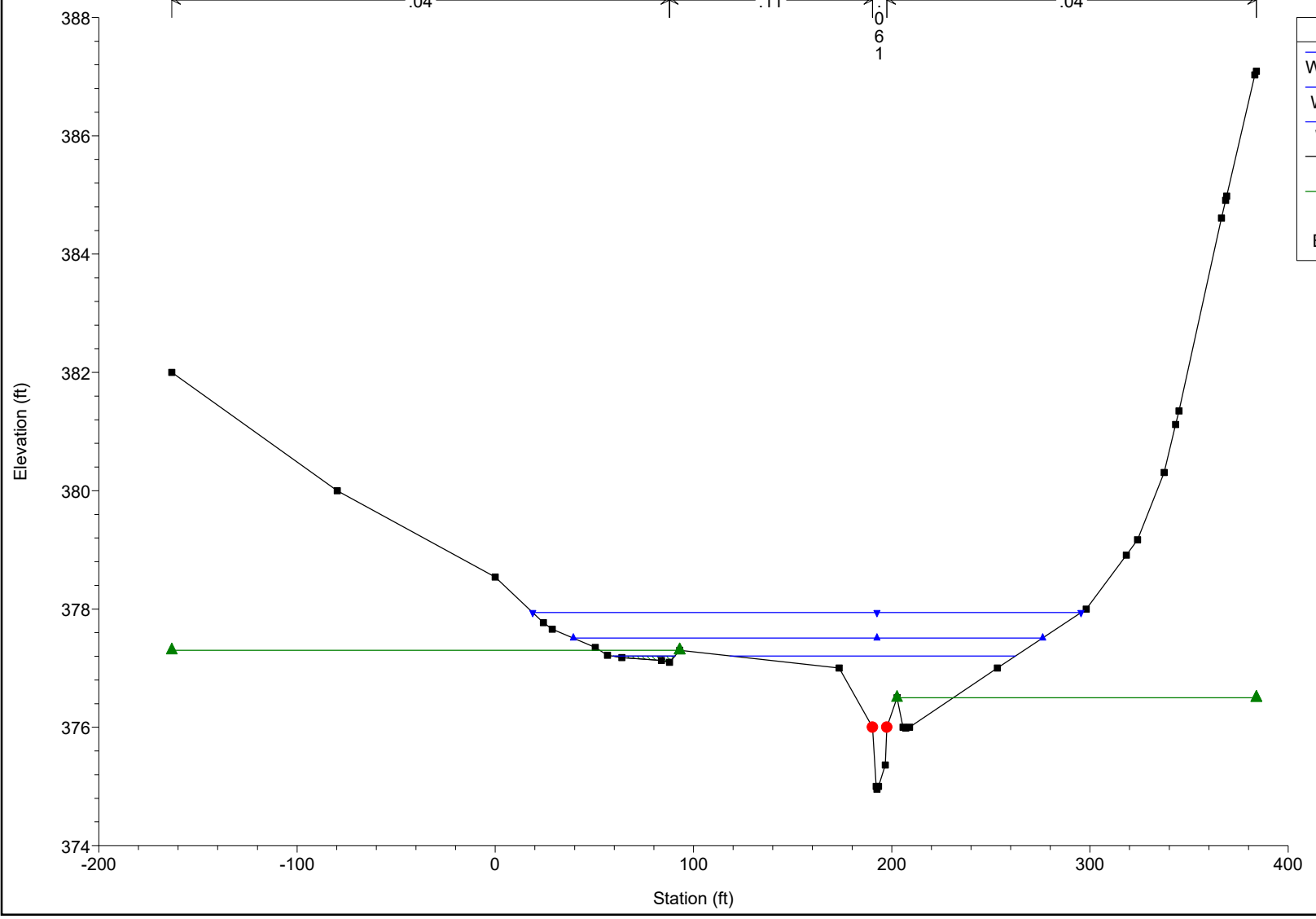
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Geom: ExistGeoECC

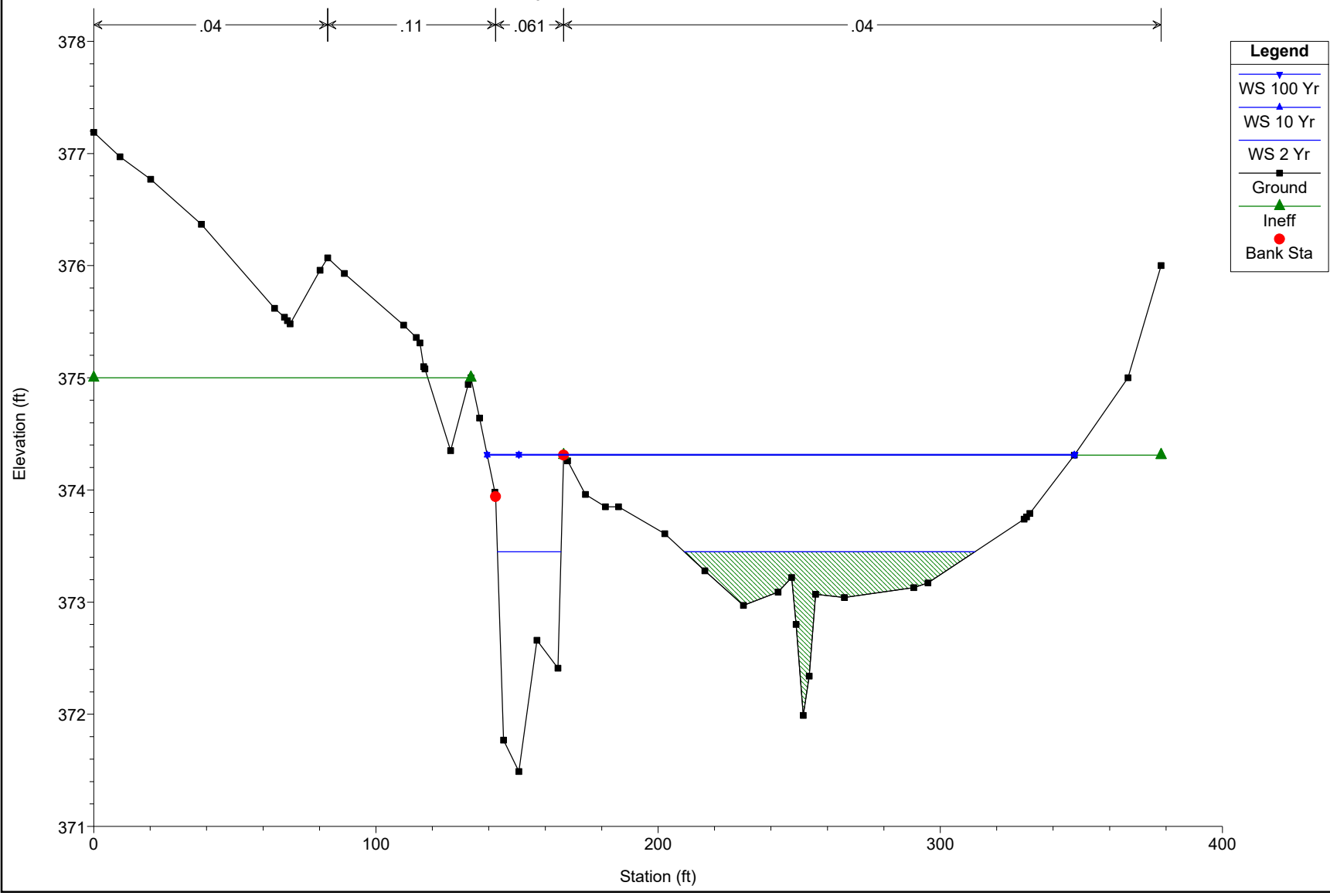
River = JonesFalls Reach = NorthTrib RS = 1.5



Legend	
WS 100 Yr	Blue line with downward triangle
WS 10 Yr	Blue line with upward triangle
WS 2 Yr	Blue line with square
Ground	Black line with square
Ineff	Green line with upward triangle
Bank Sta	Red line with circle



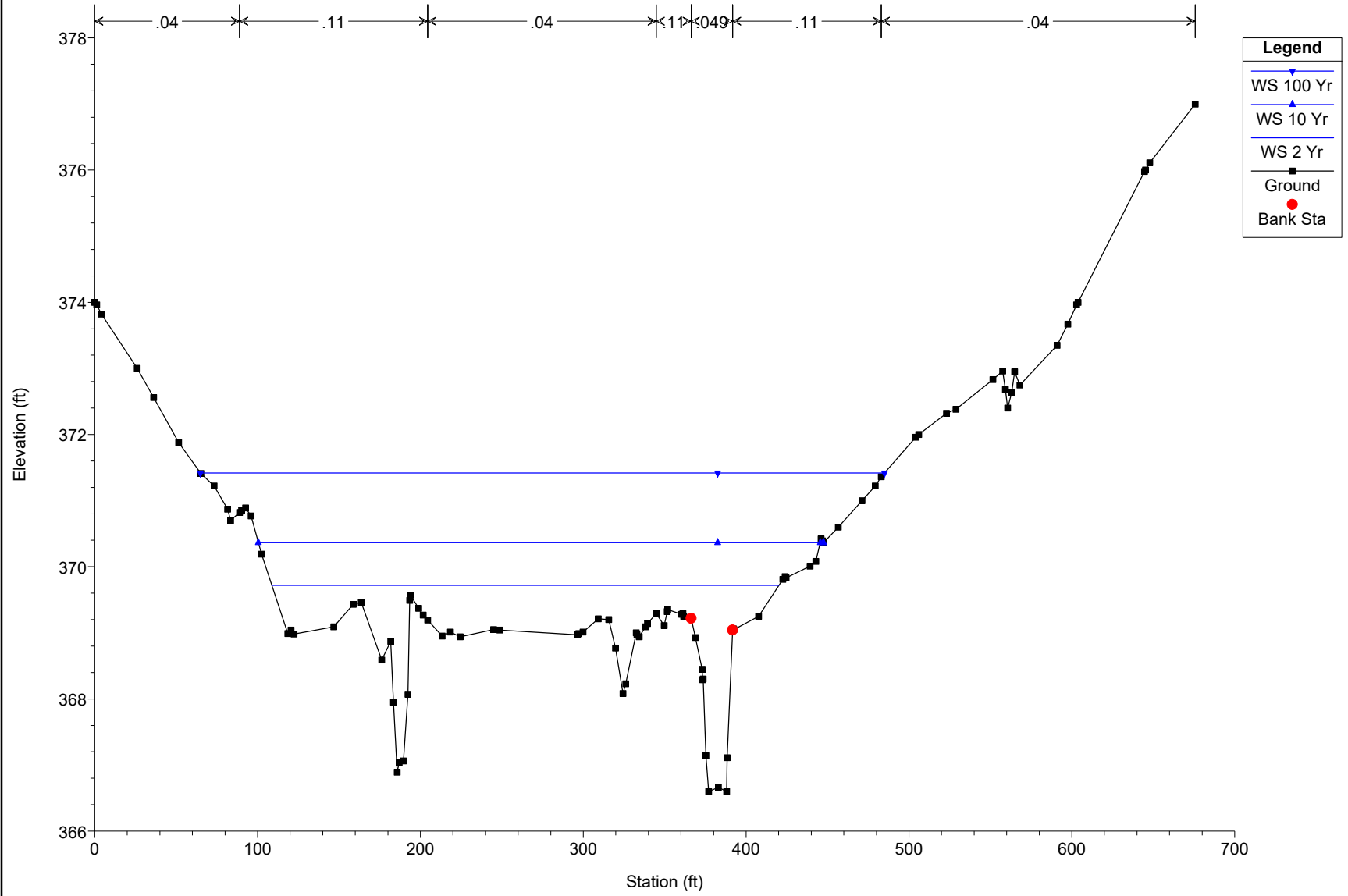
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 Geom: ExistGeoECC  
 River = JonesFalls Reach = NorthTrib RS = 1



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

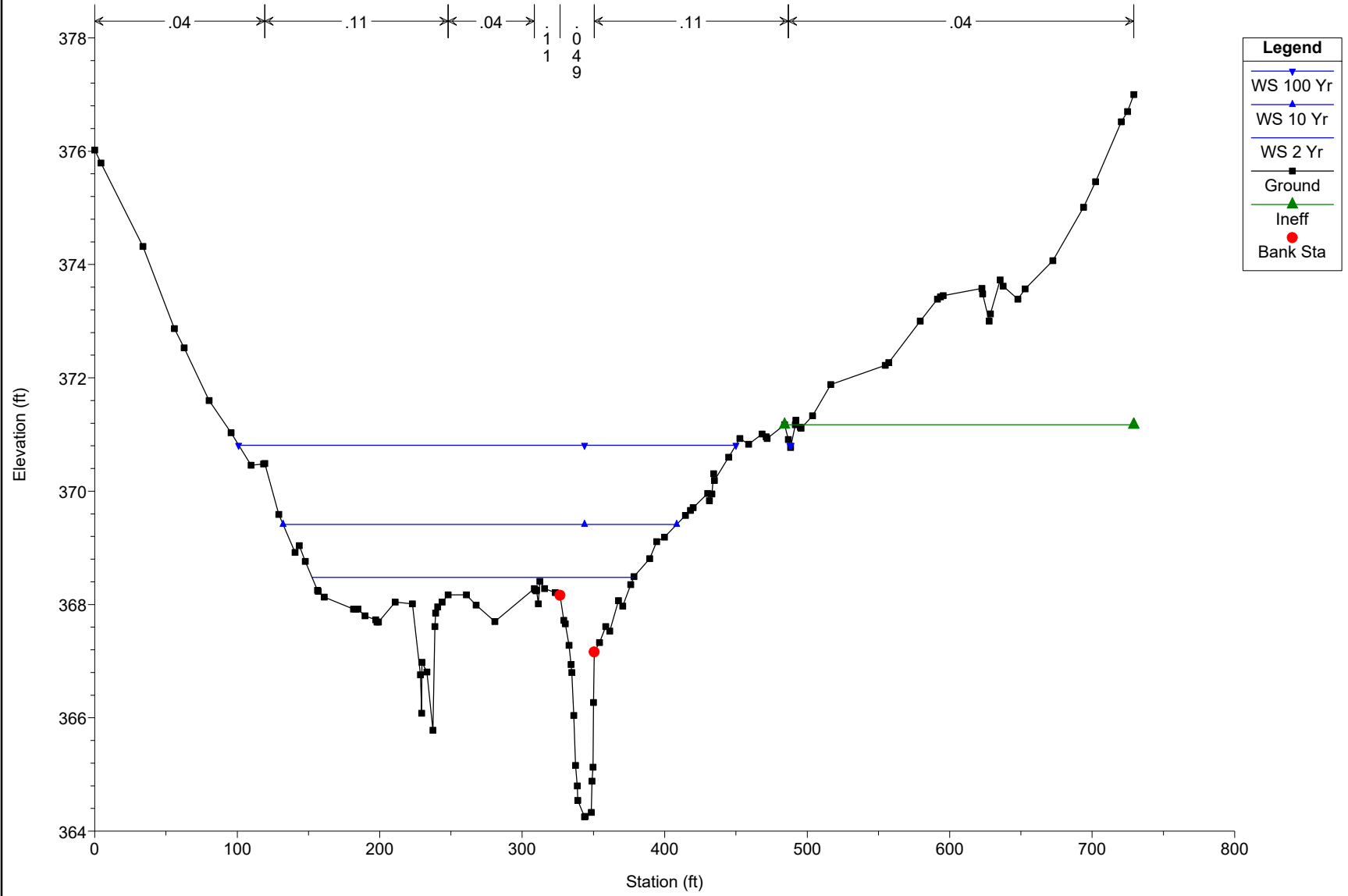
River = JonesFalls Reach = JonesFalls2 RS = 12



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

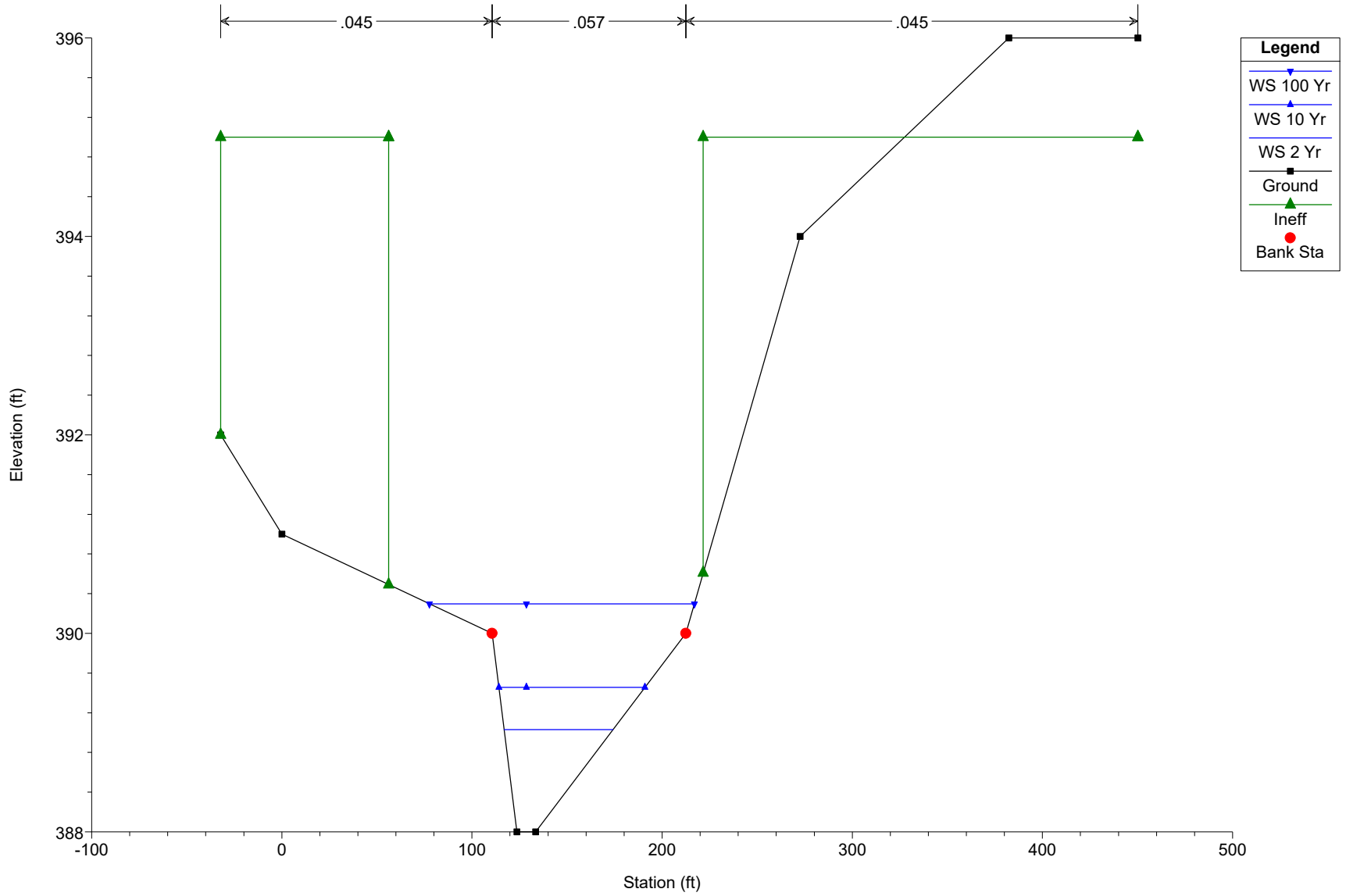
River = JonesFalls Reach = JonesFalls2 RS = 11



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

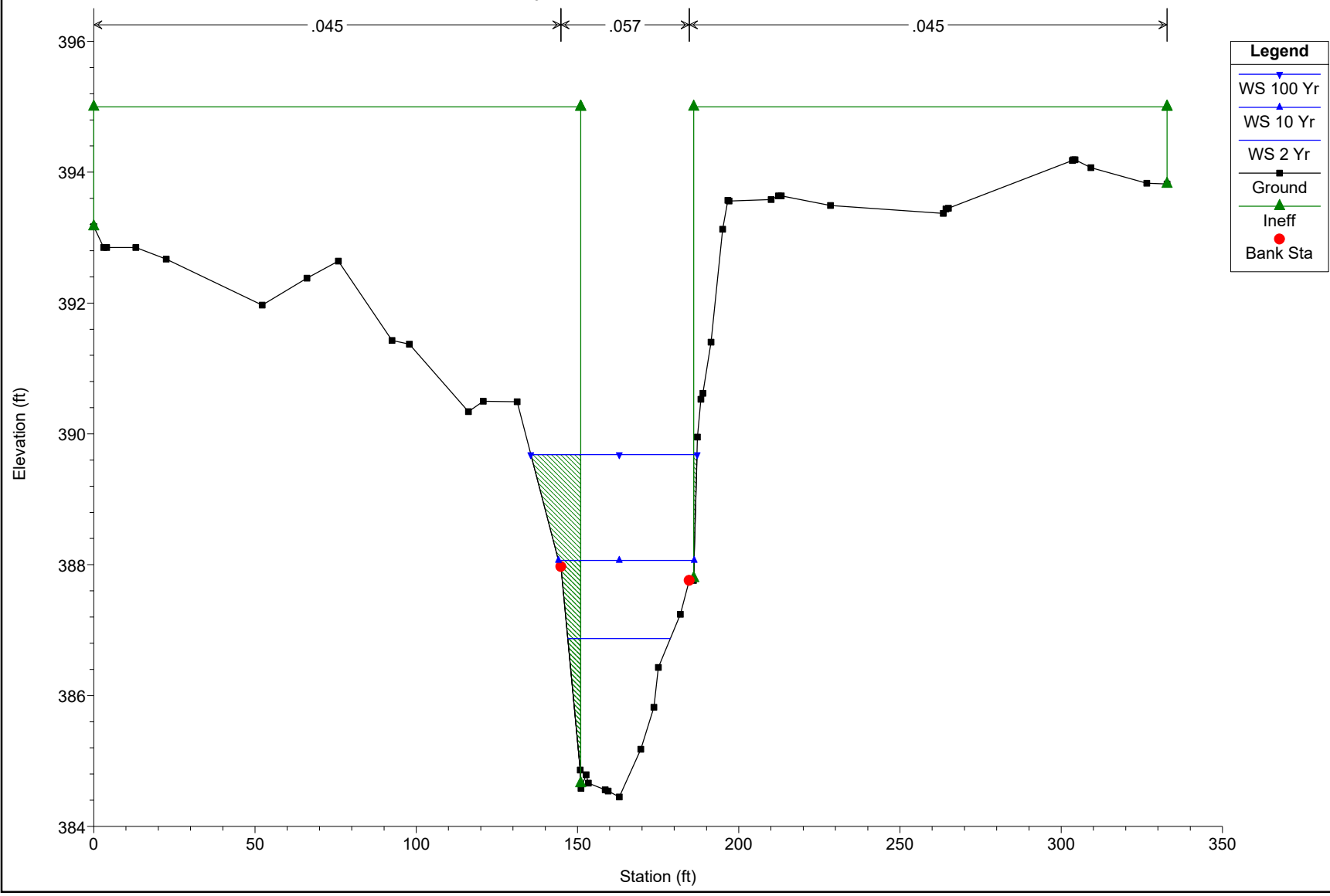
River = JonesFalls Reach = SouthTrib RS = 8



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

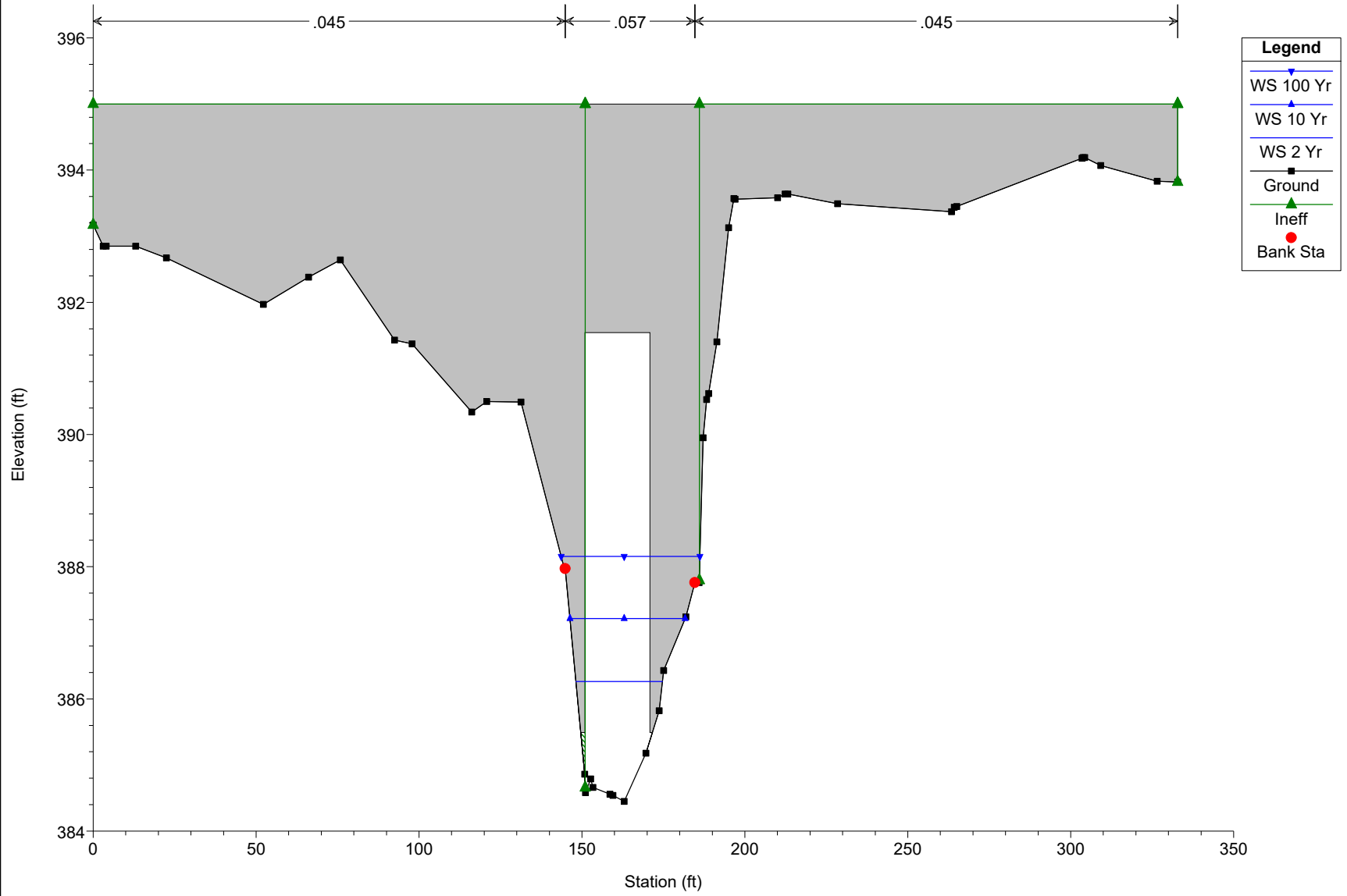
River = JonesFalls Reach = SouthTrib RS = 7



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

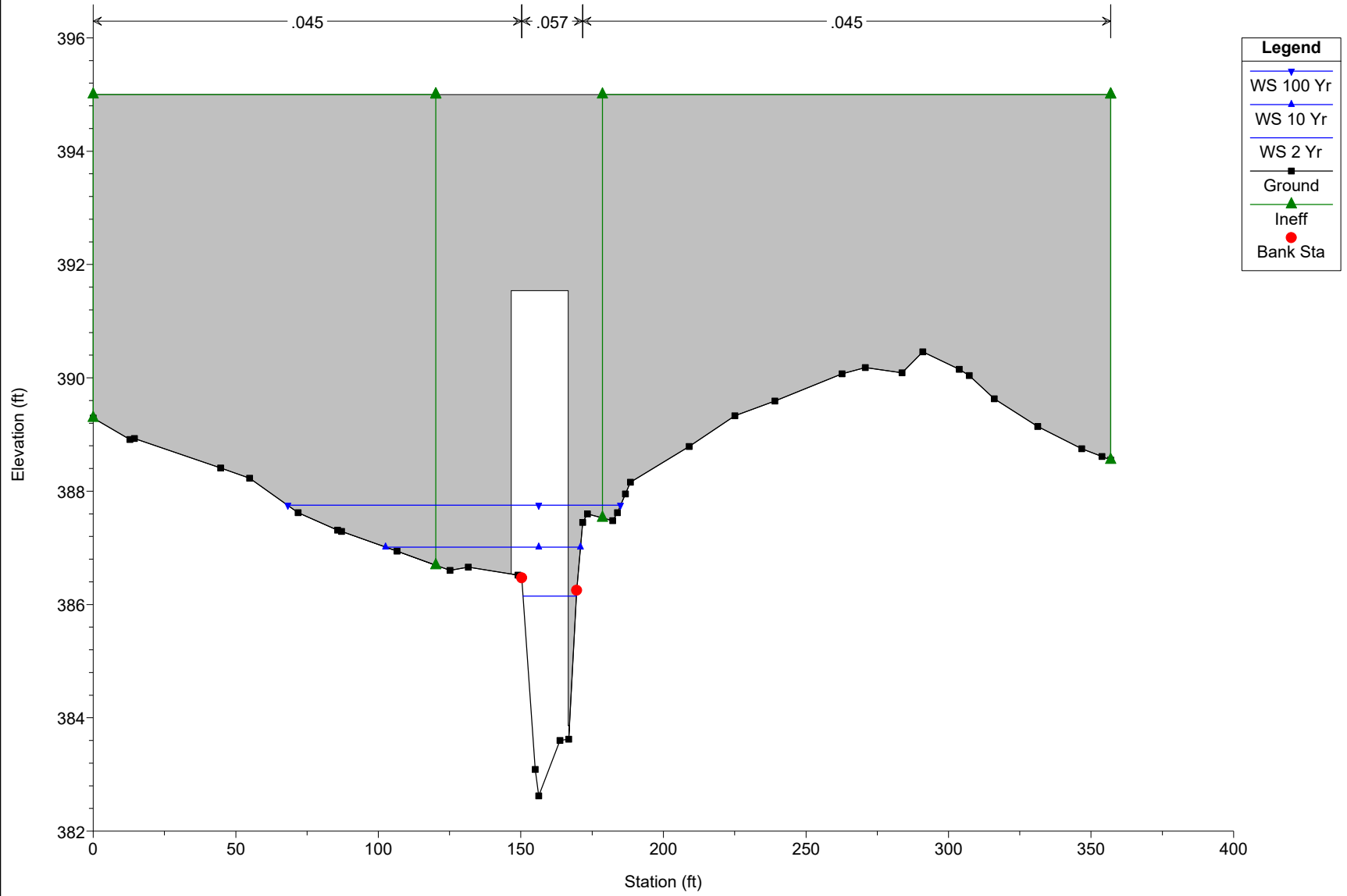
River = JonesFalls Reach = SouthTrib RS = 6.5 BR Cliffholme Rd Bridge



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = SouthTrib RS = 6.5 BR Cliffholme Rd Bridge

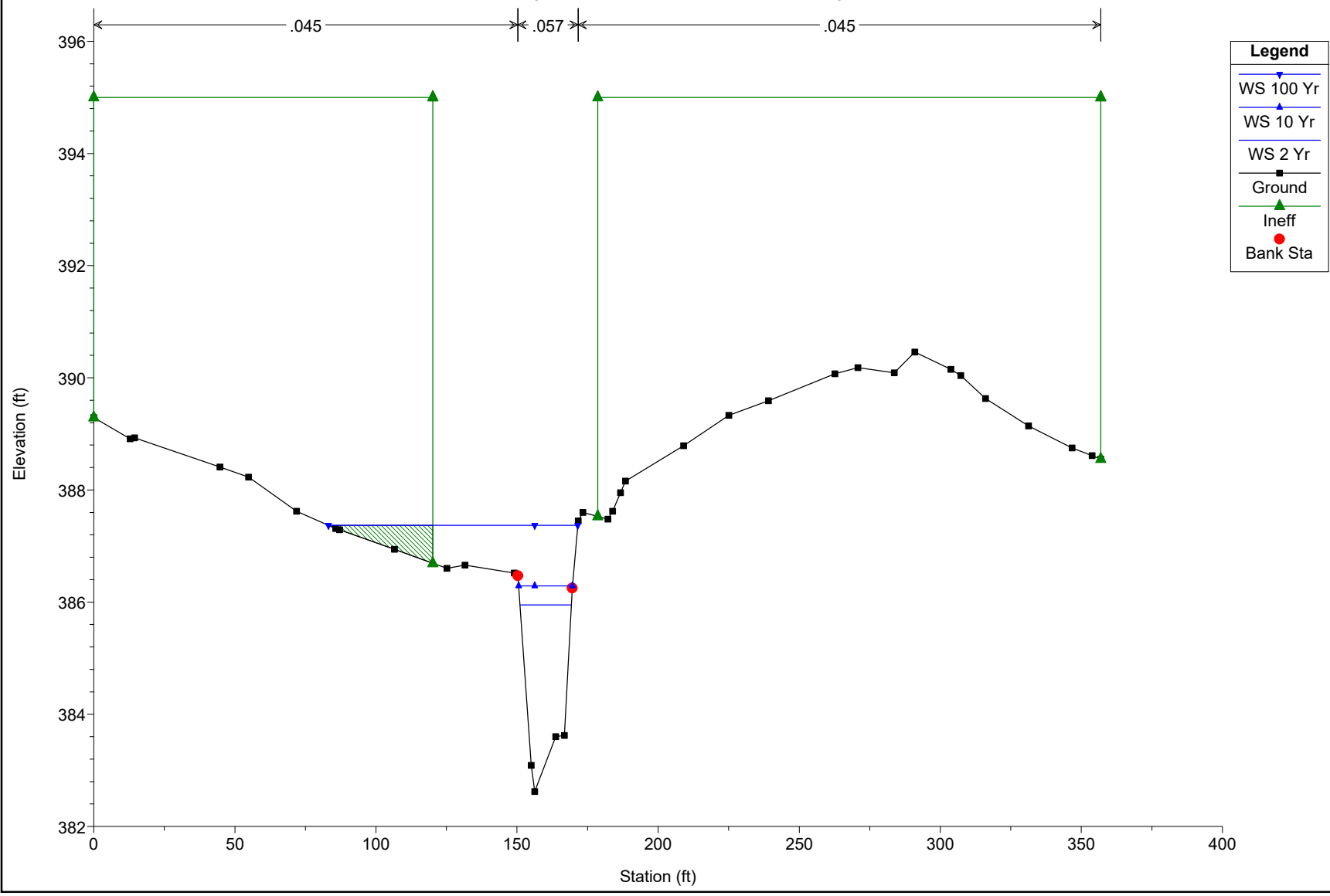




EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

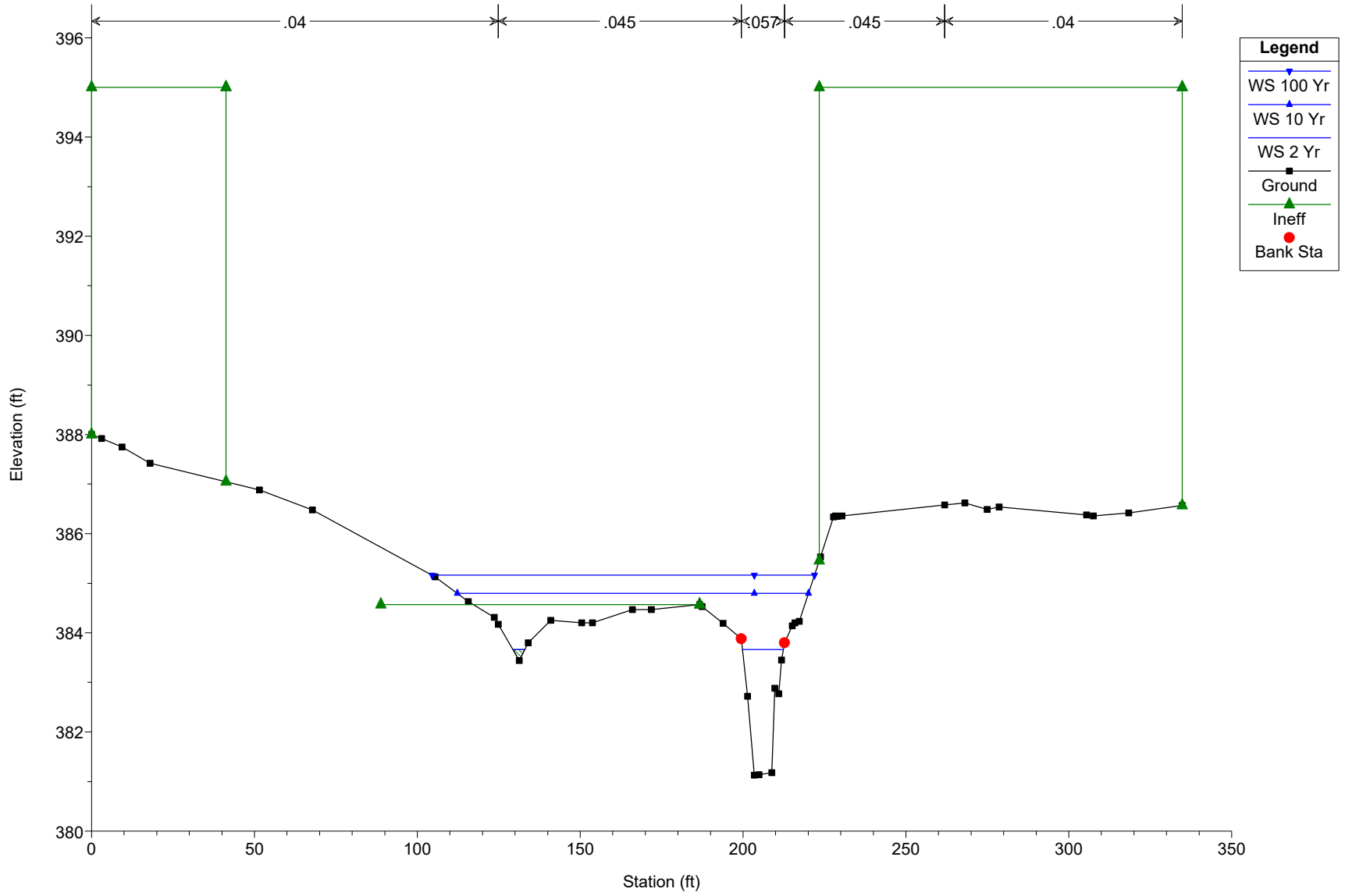
River = JonesFalls Reach = SouthTrib RS = 6



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

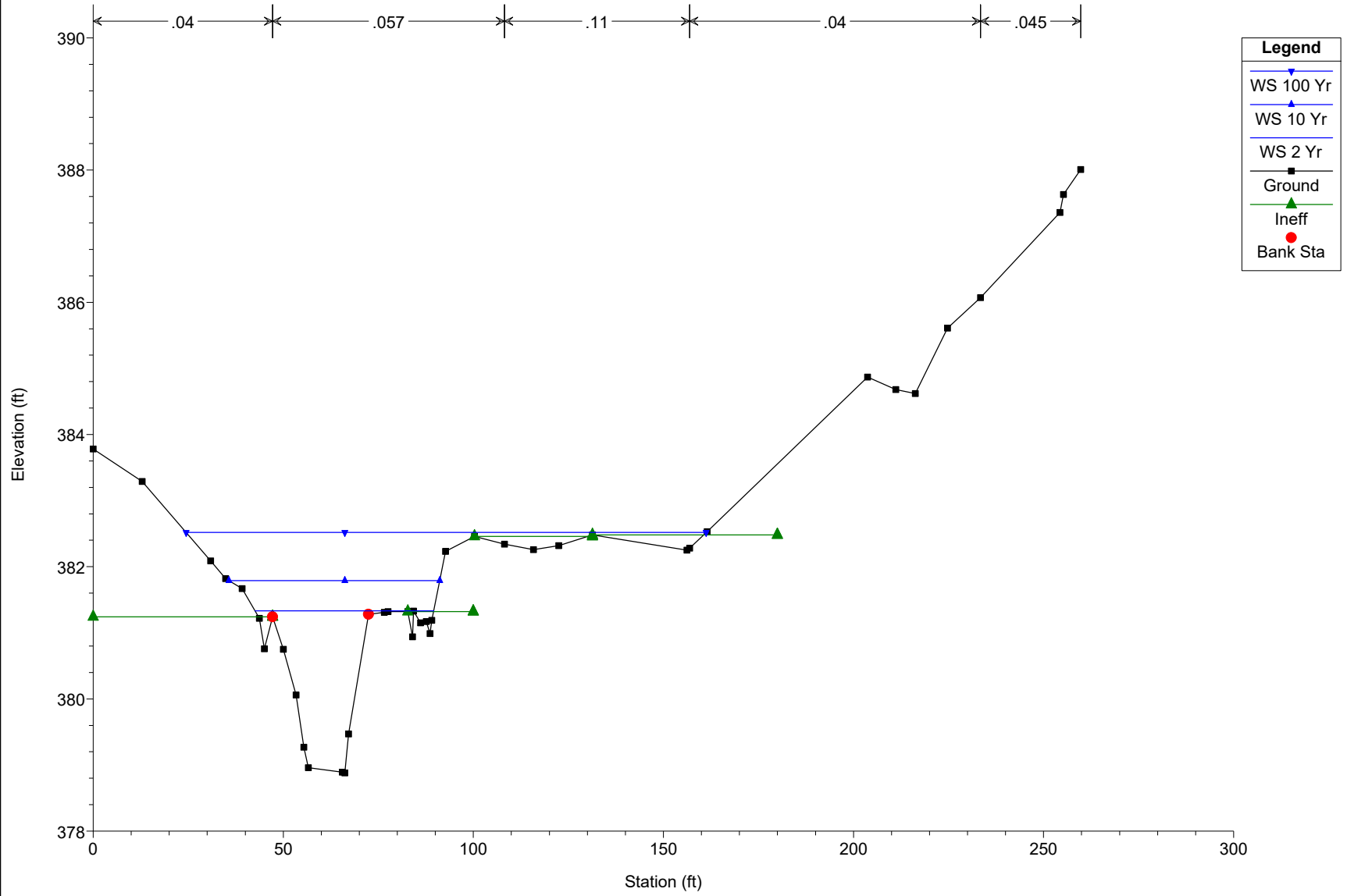
River = JonesFalls Reach = SouthTrib RS = 5



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

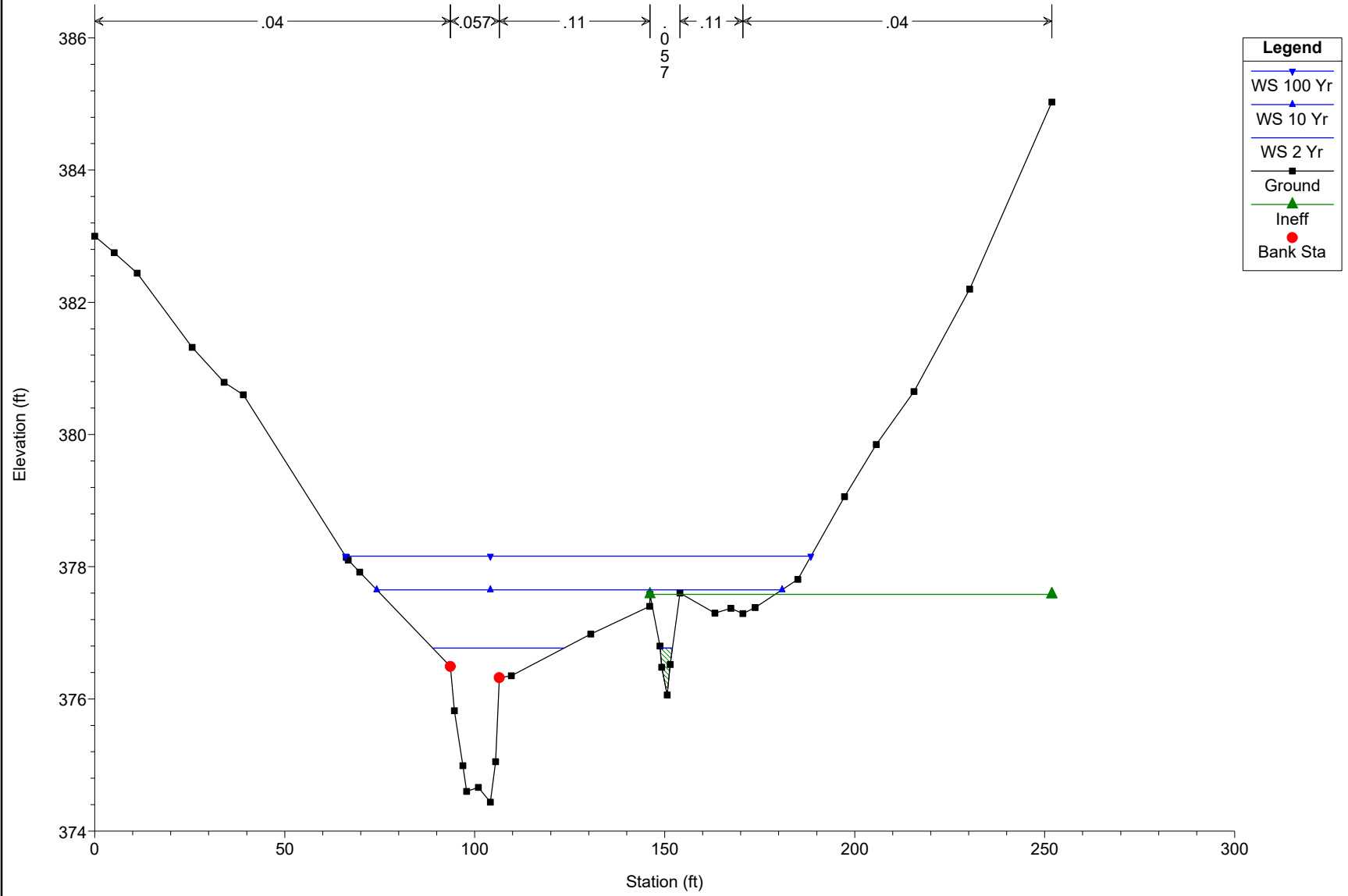
River = JonesFalls Reach = SouthTrib RS = 4



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

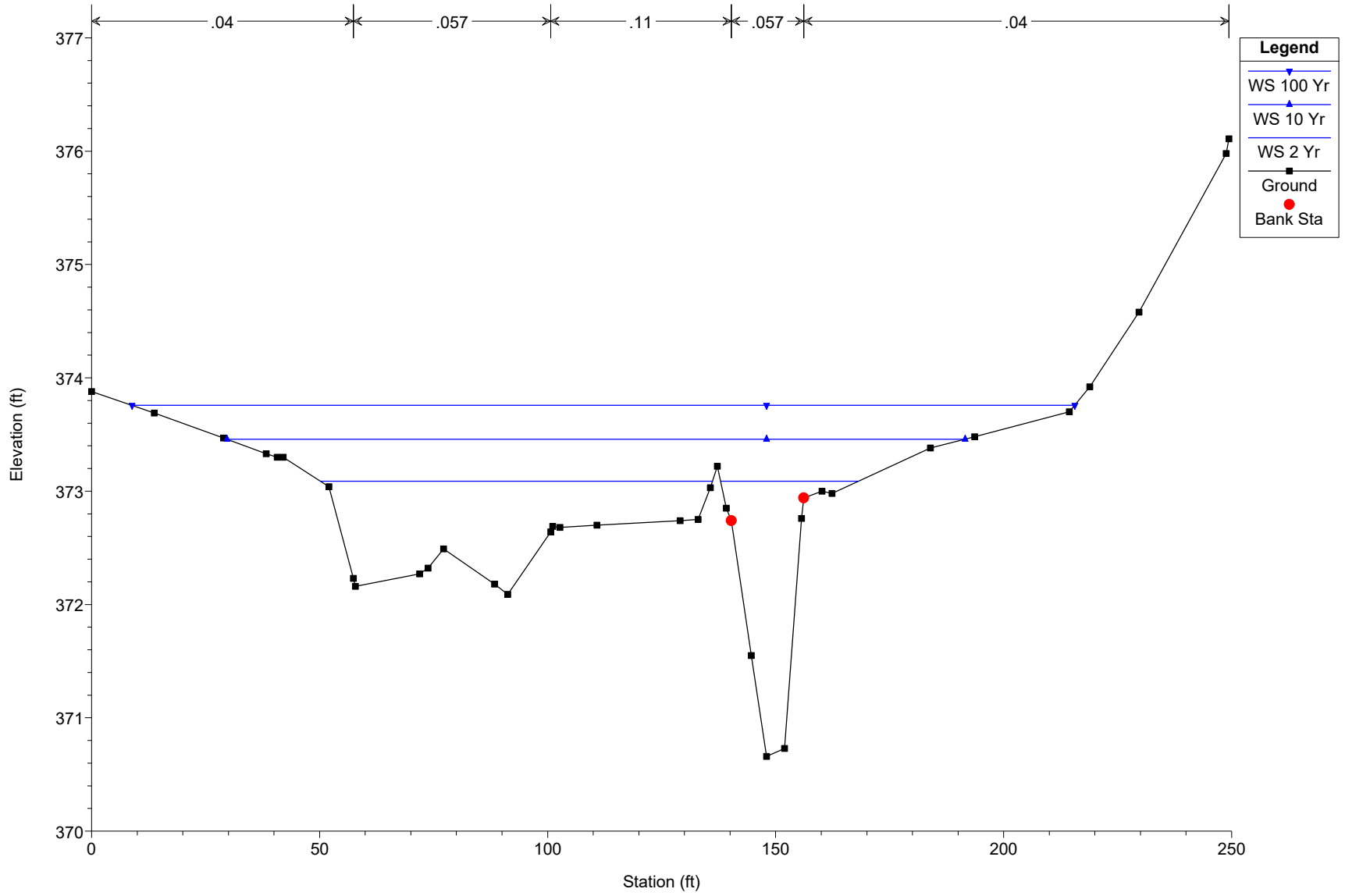
River = JonesFalls Reach = SouthTrib RS = 3



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

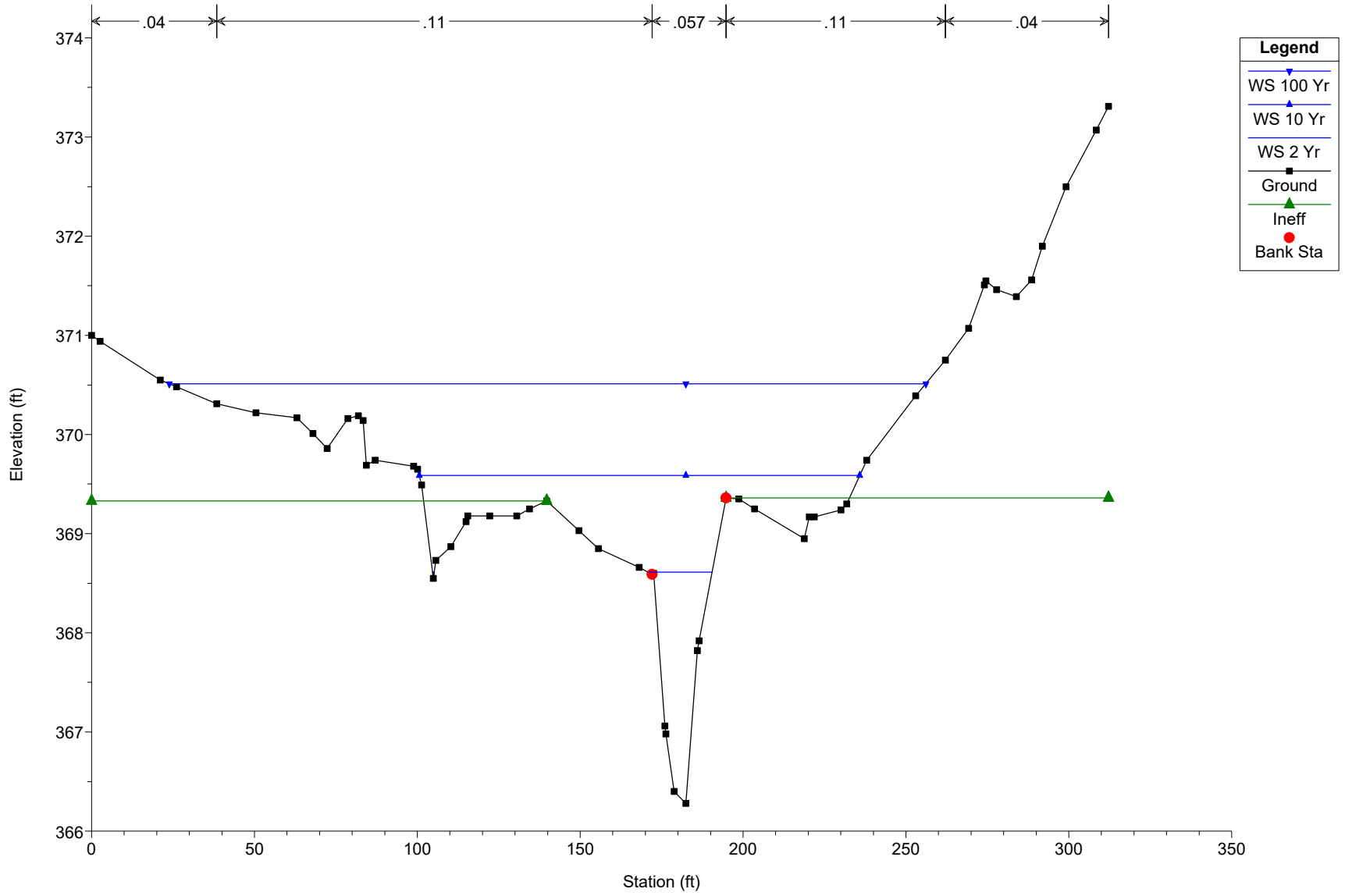
River = JonesFalls Reach = SouthTrib RS = 2



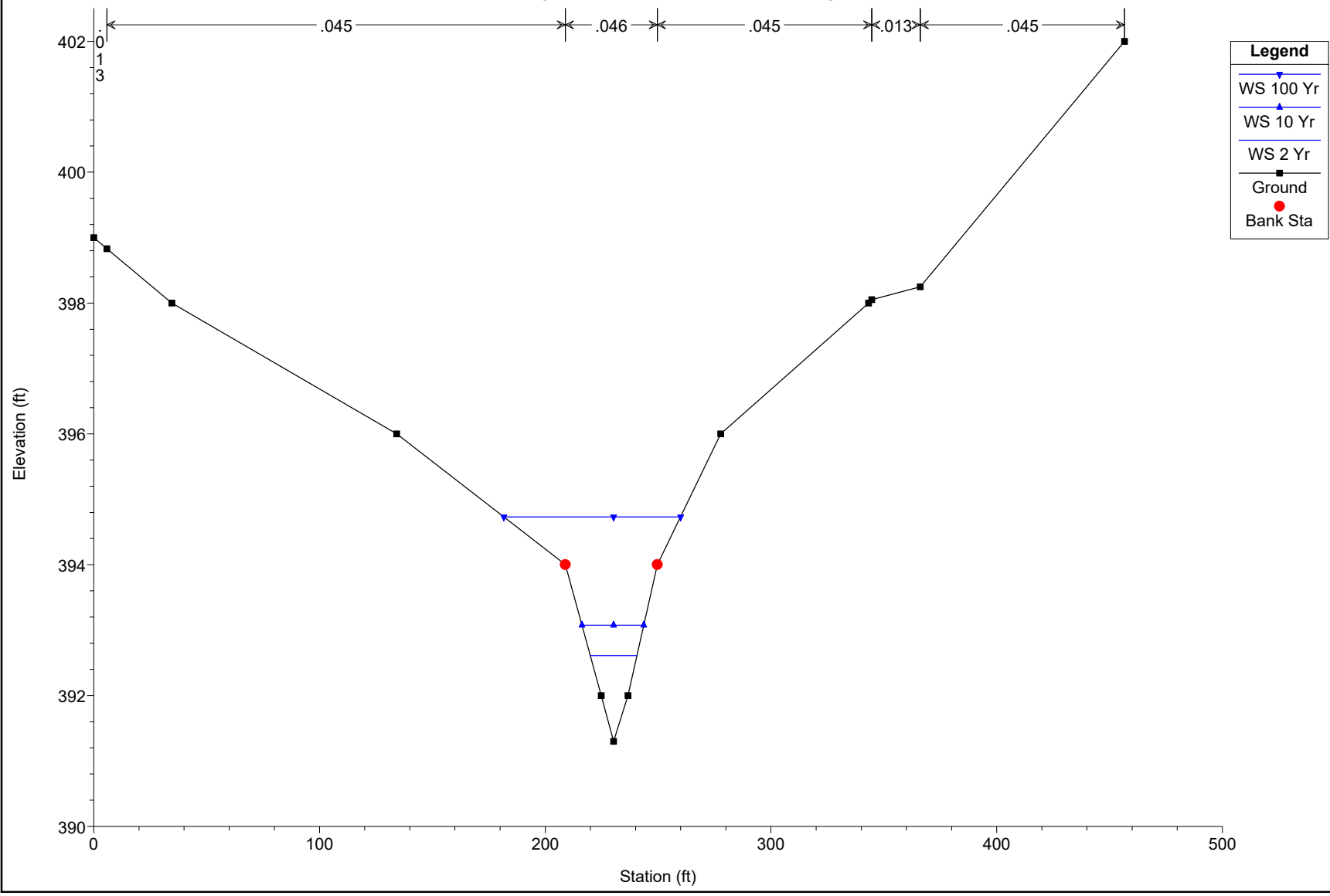
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Geom: ExistGeoECC

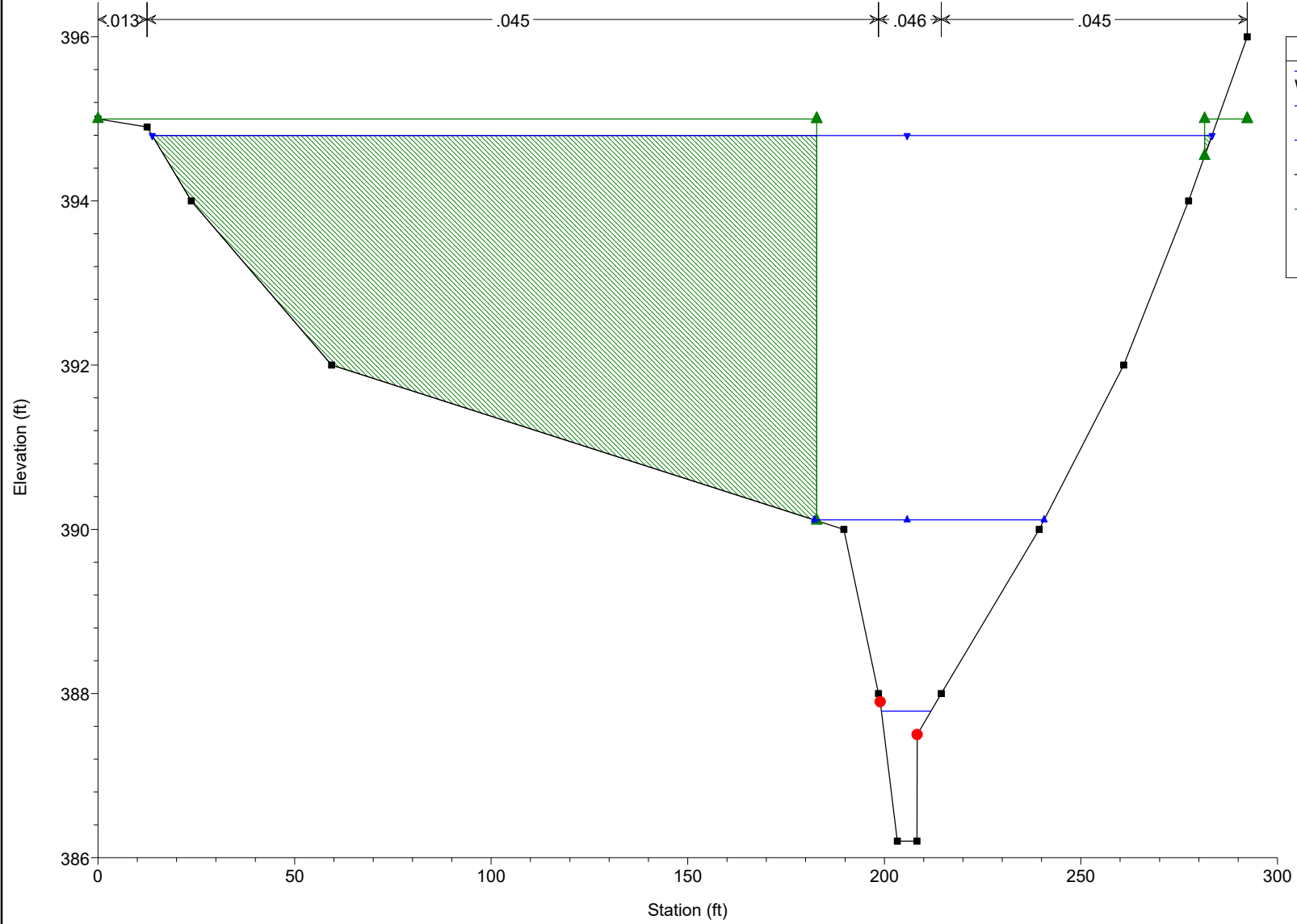
River = JonesFalls Reach = SouthTrib RS = 1



EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = IntsTrib RS = 8



EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = IntsTrib RS = 7



**Legend**

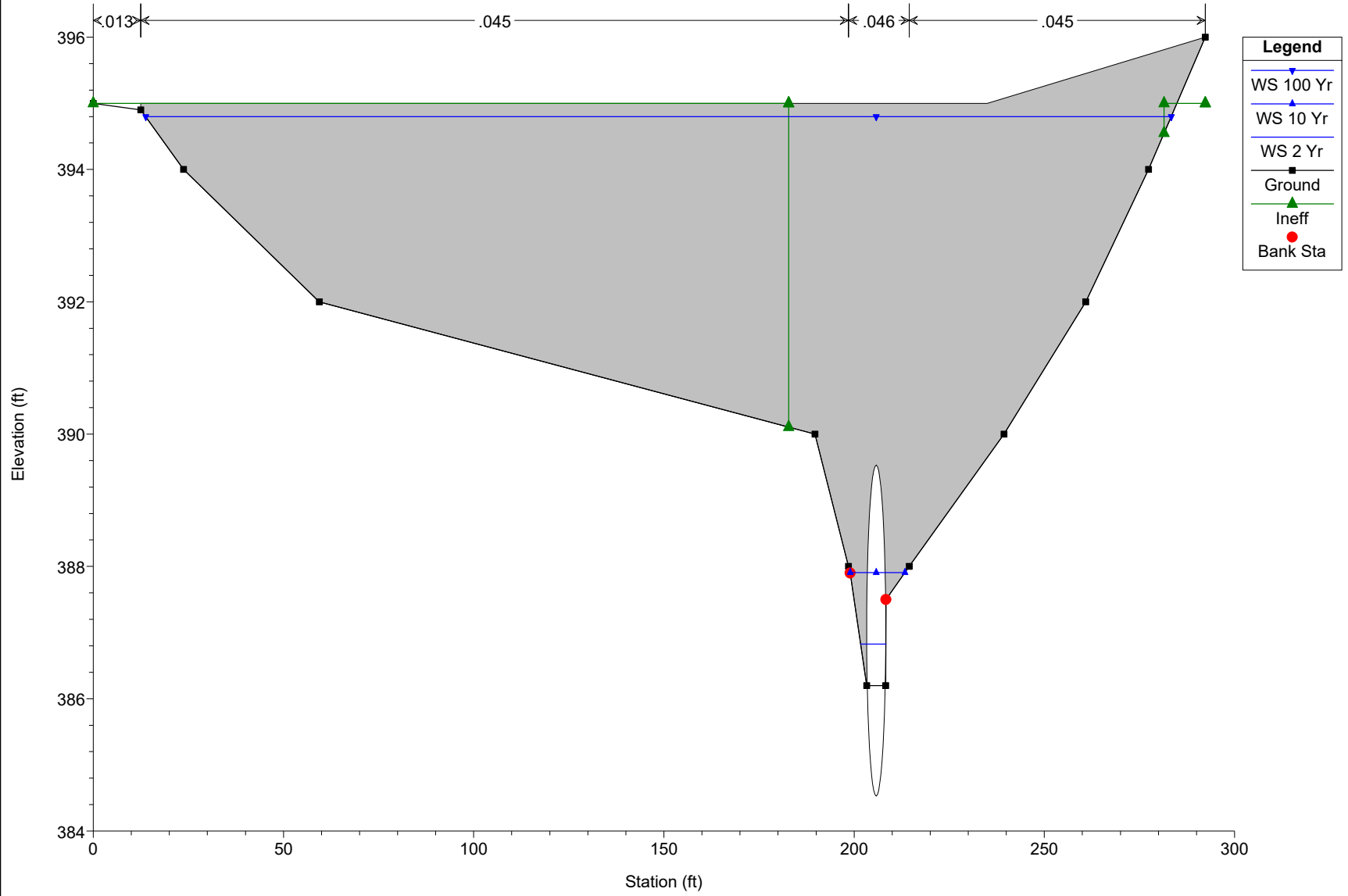
- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Ineff
- Bank Sta



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

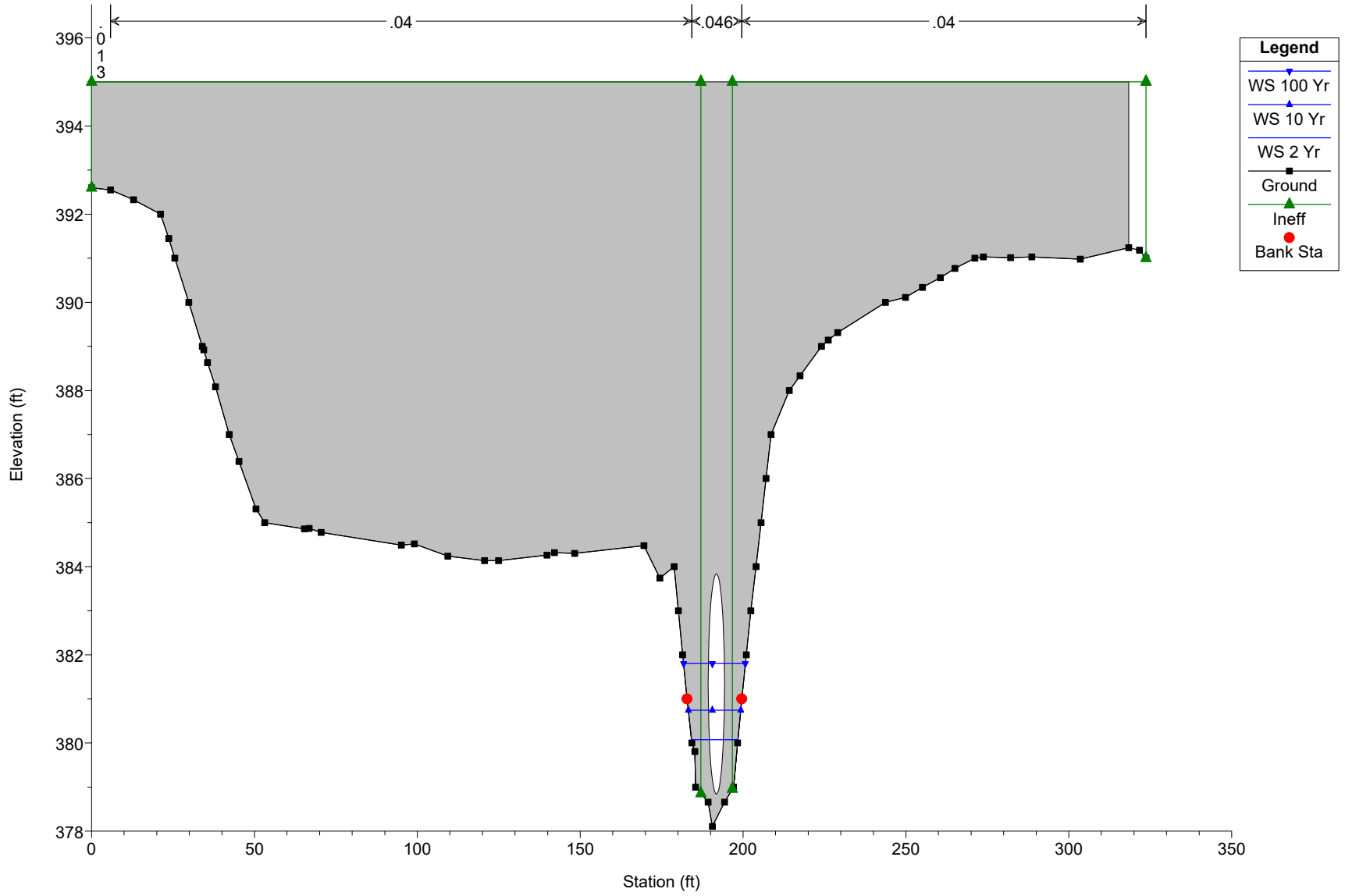
River = JonesFalls Reach = IntsTrib RS = 6.5 Culv Culvert under Greenspring Valley Rd.



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

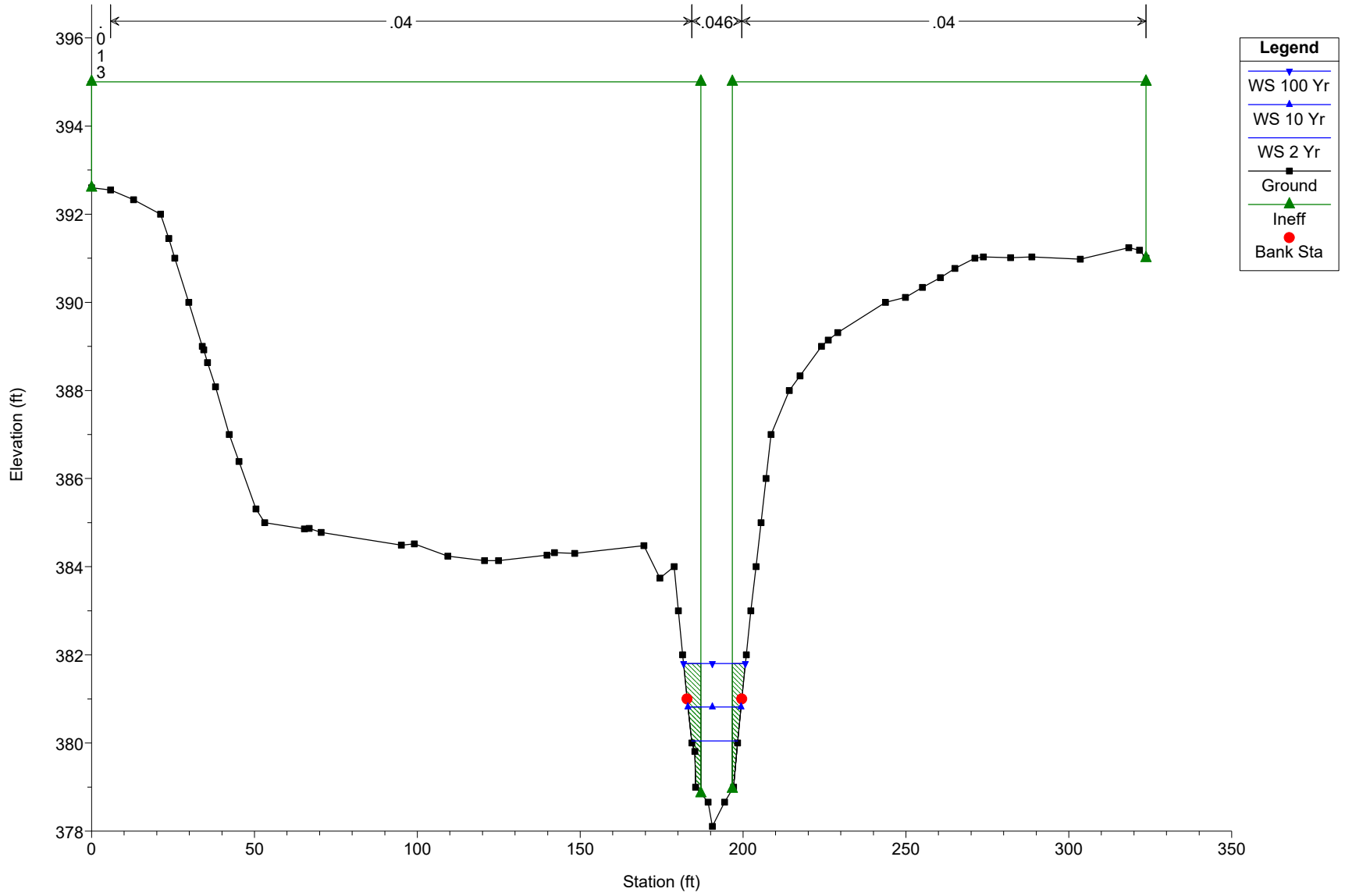
River = JonesFalls Reach = IntsTrib RS = 6.5 Culv Culvert under Greenspring Valley Rd.



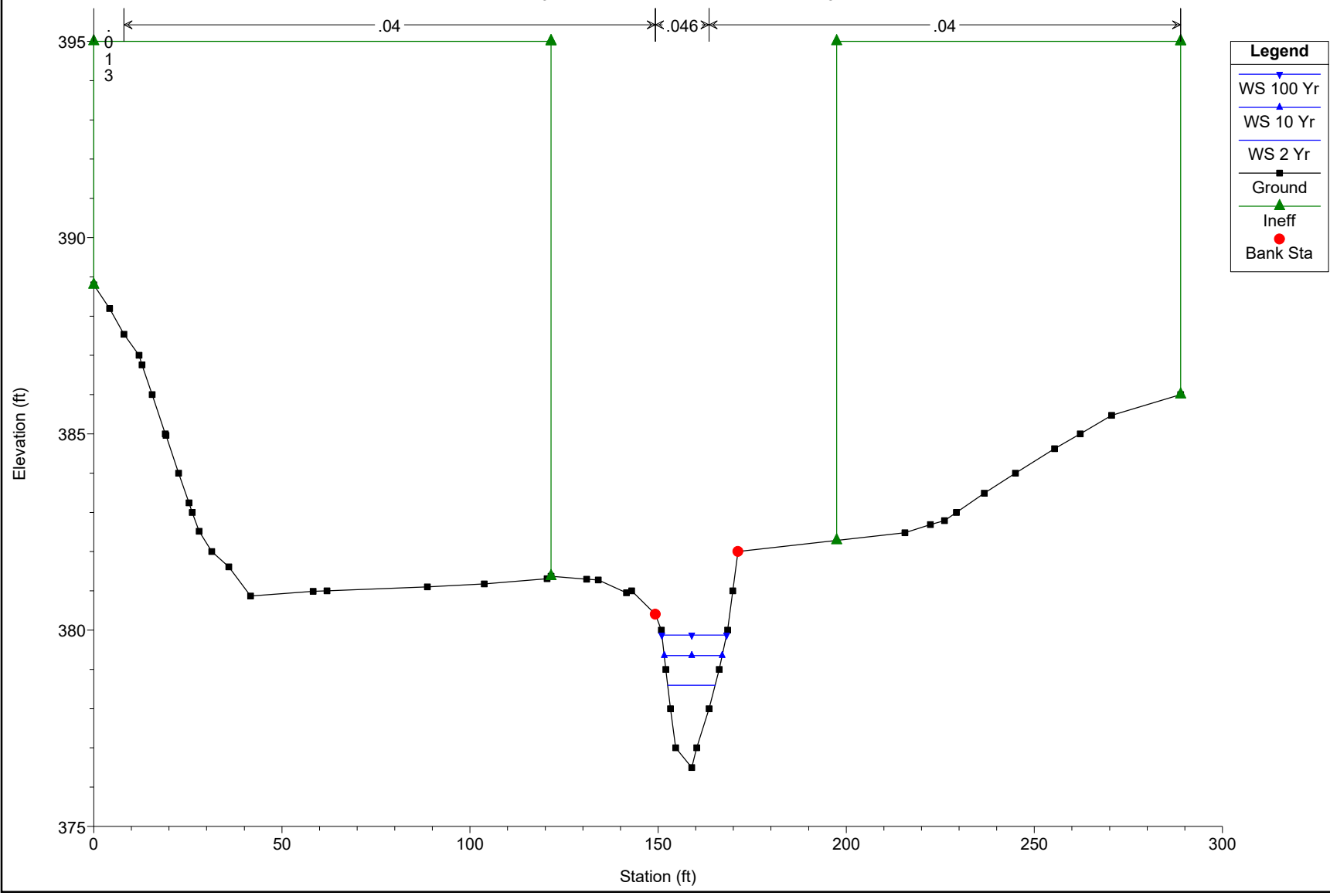
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = IntsTrib RS = 6



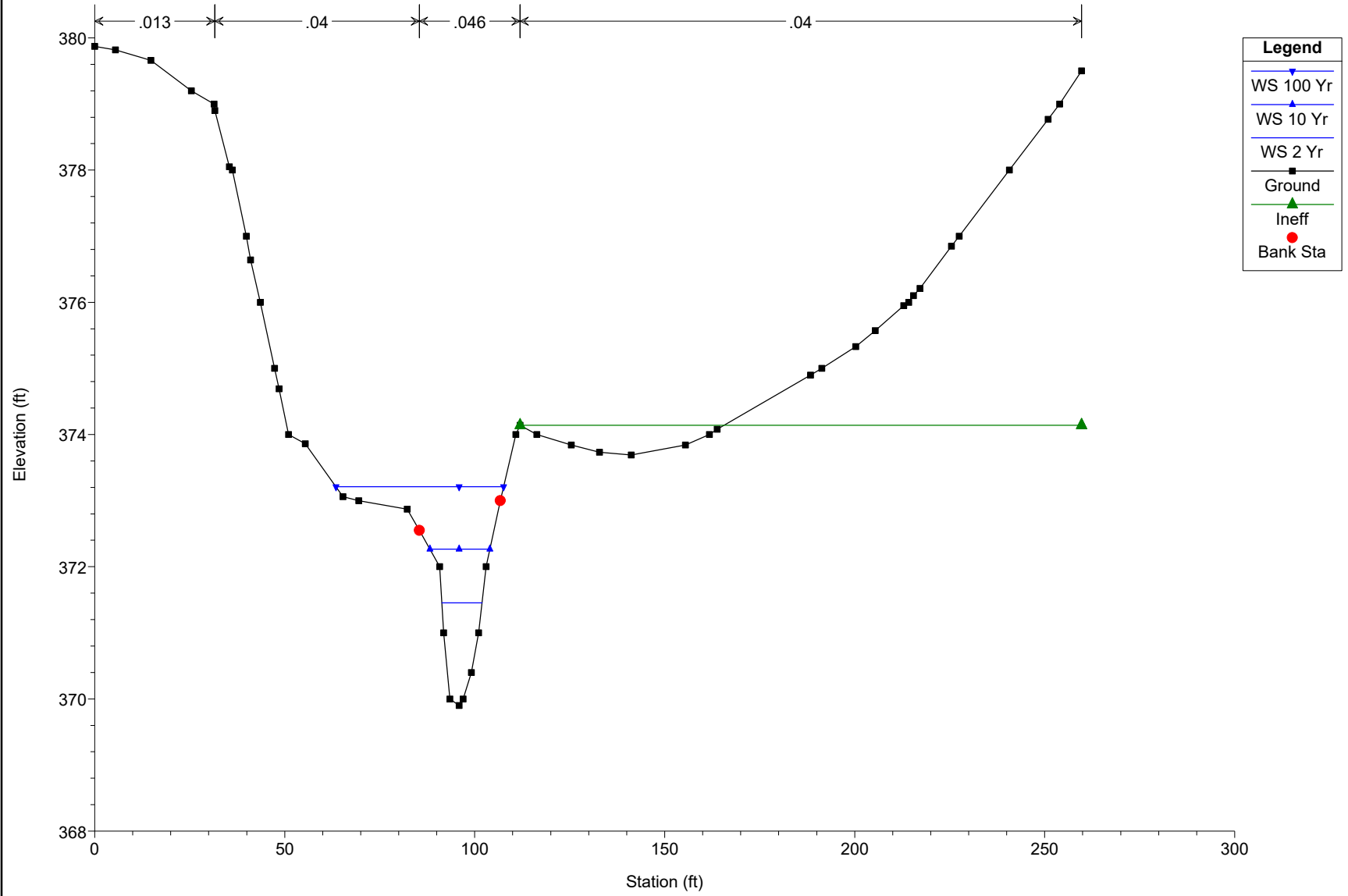
EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = IntsTrib RS = 5



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = IntsTrib RS = 4



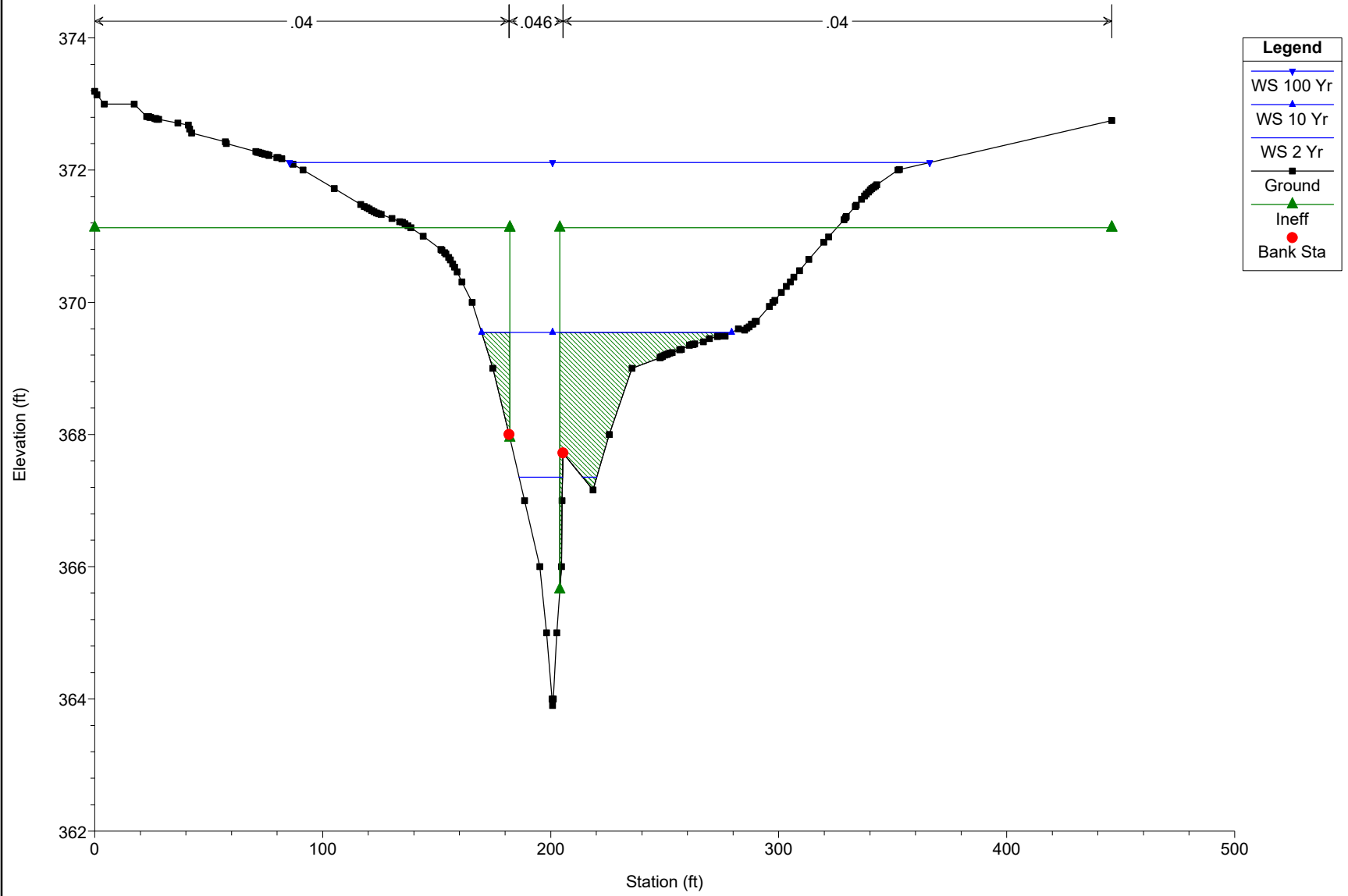
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Ineff
- Bank Sta

EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

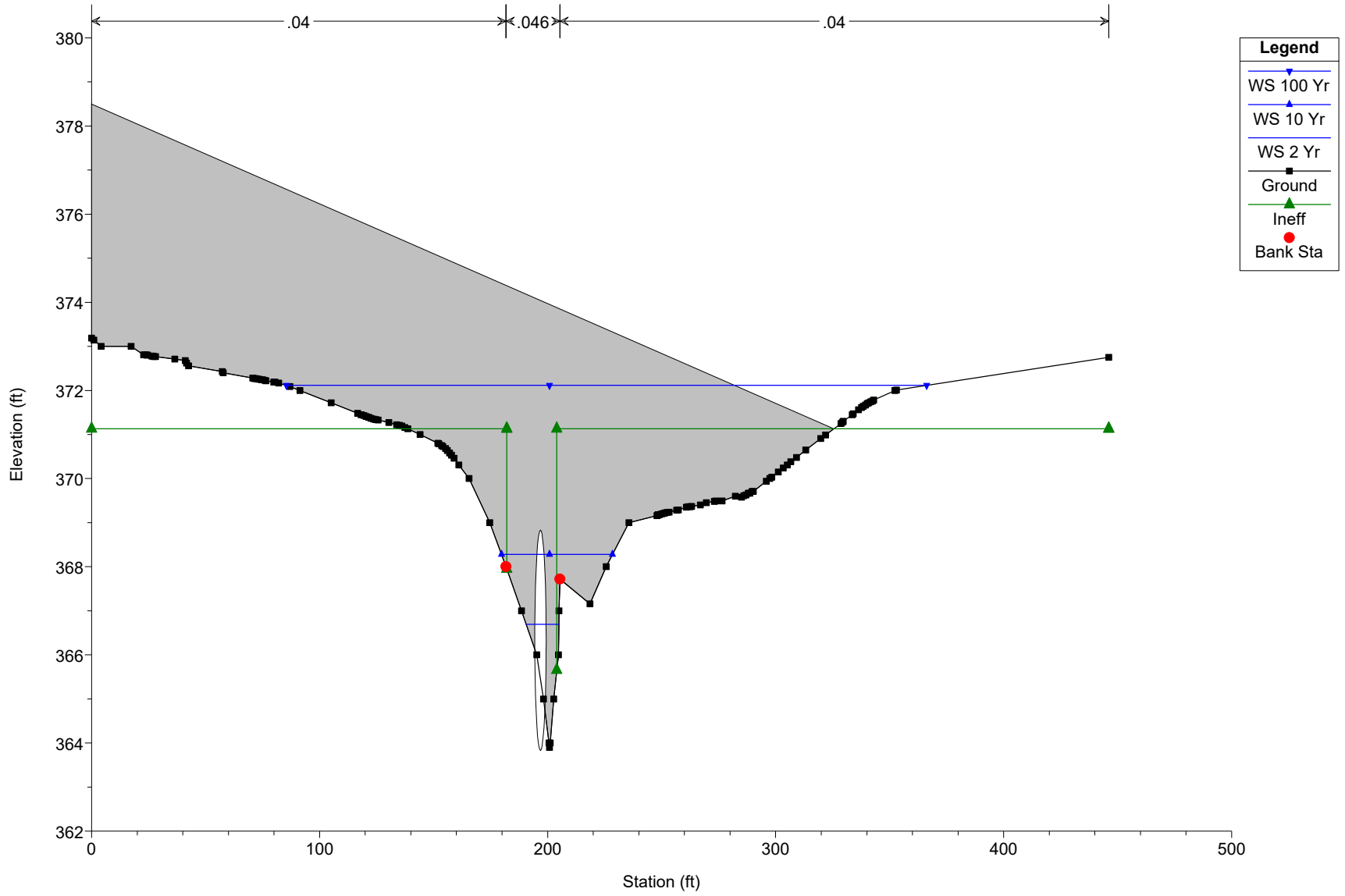
River = JonesFalls Reach = IntsTrib RS = 3



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = IntsTrib RS = 2.5 Culv Intersection Trib Culvert under Park Heights Ave.

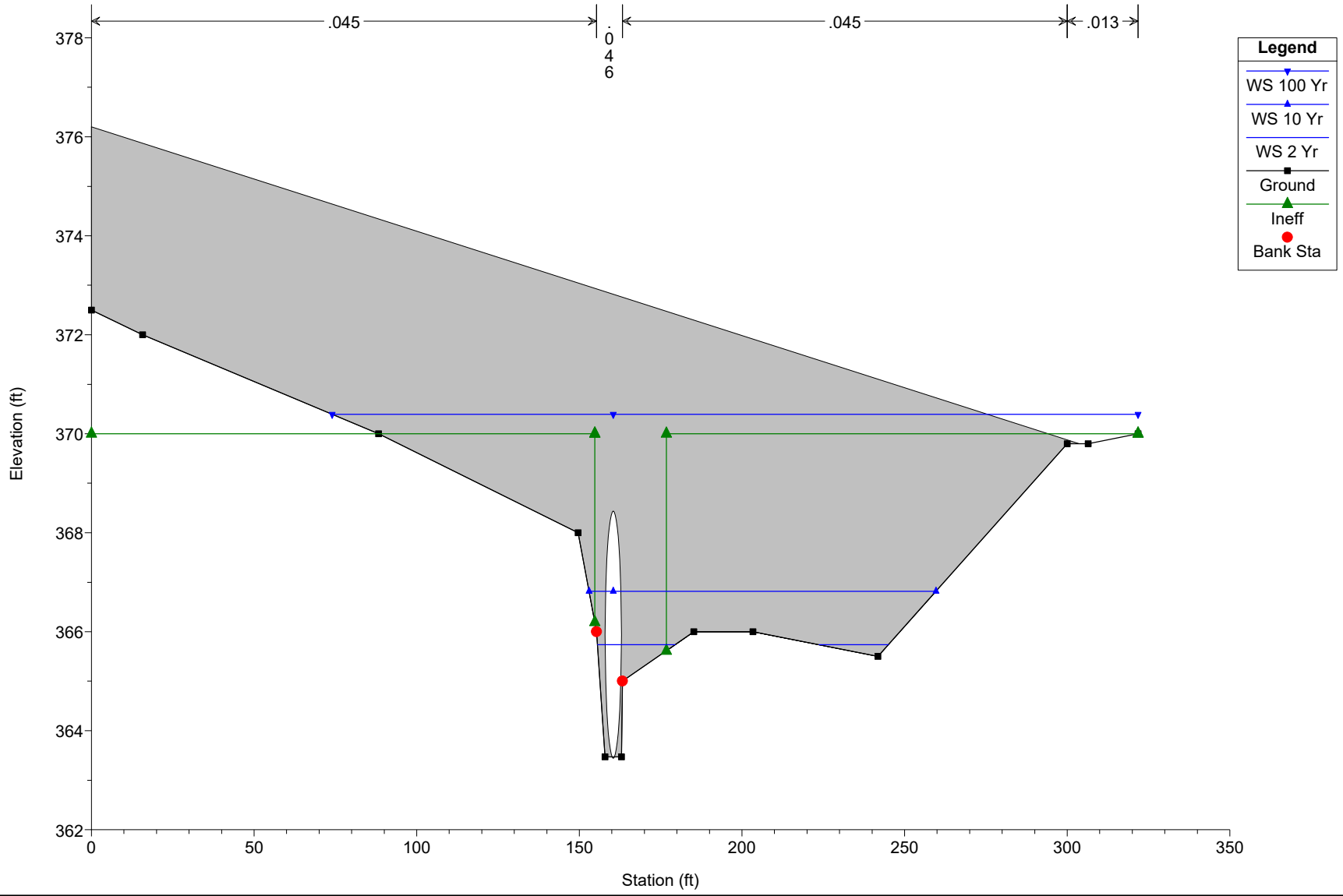


Legend	
WS 100 Yr	Blue line with downward triangle
WS 10 Yr	Blue line with downward triangle
WS 2 Yr	Blue line with downward triangle
Ground	Black line with square
Ineff	Green line with upward triangle
Bank Sta	Red dot

EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = IntsTrib RS = 2.5 Culv Intersection Trib Culvert under Park Heights Ave.

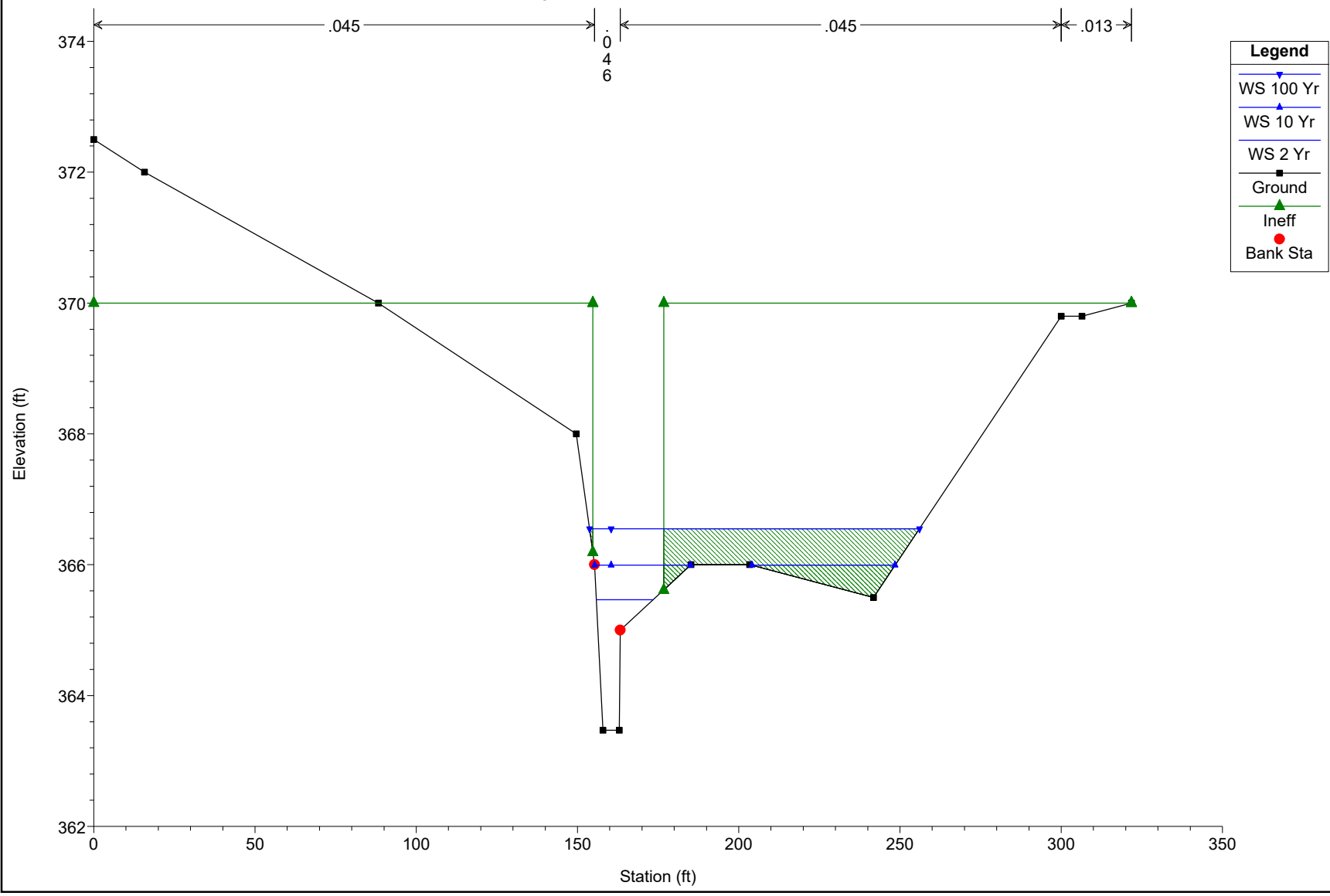




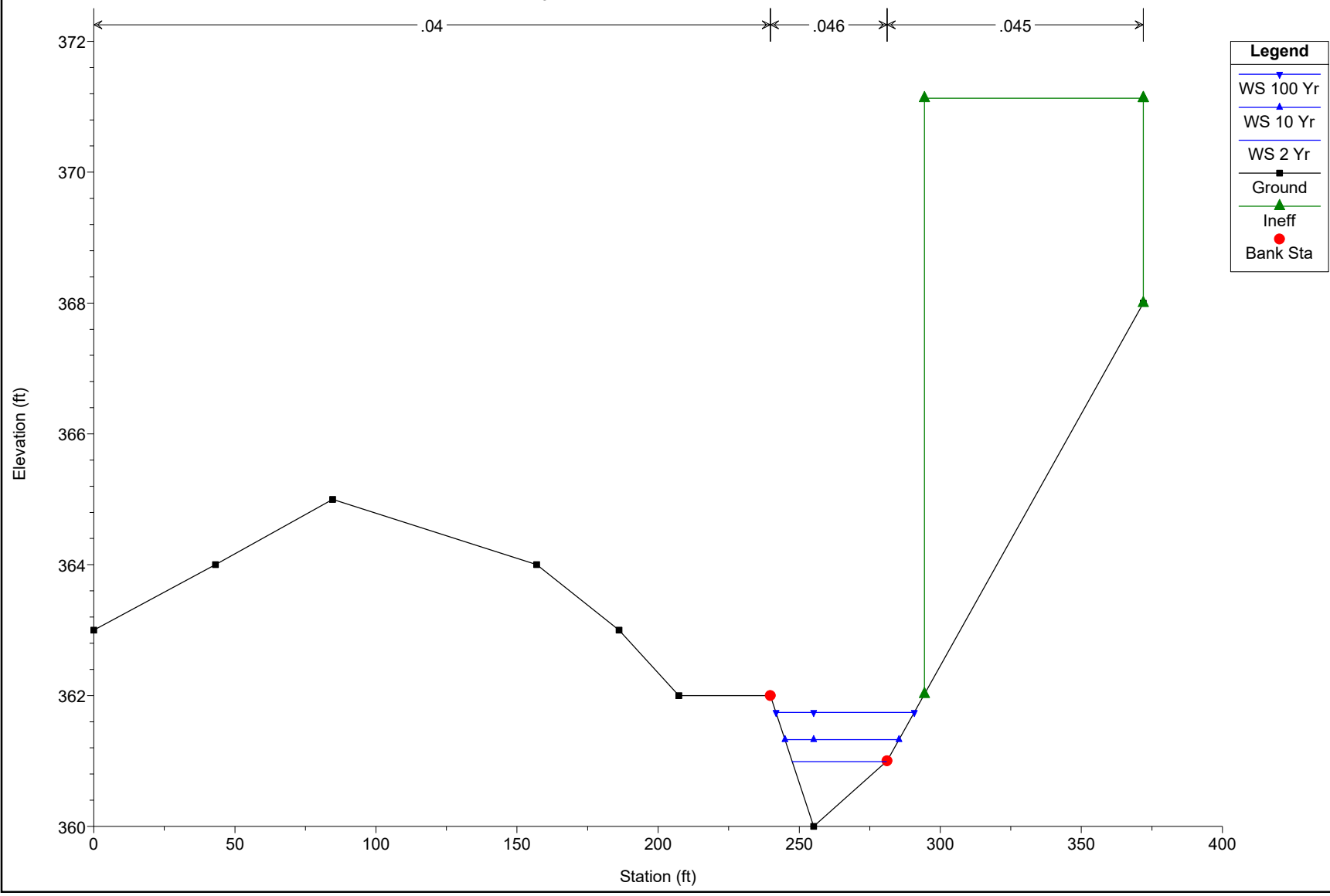
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = IntsTrib RS = 2

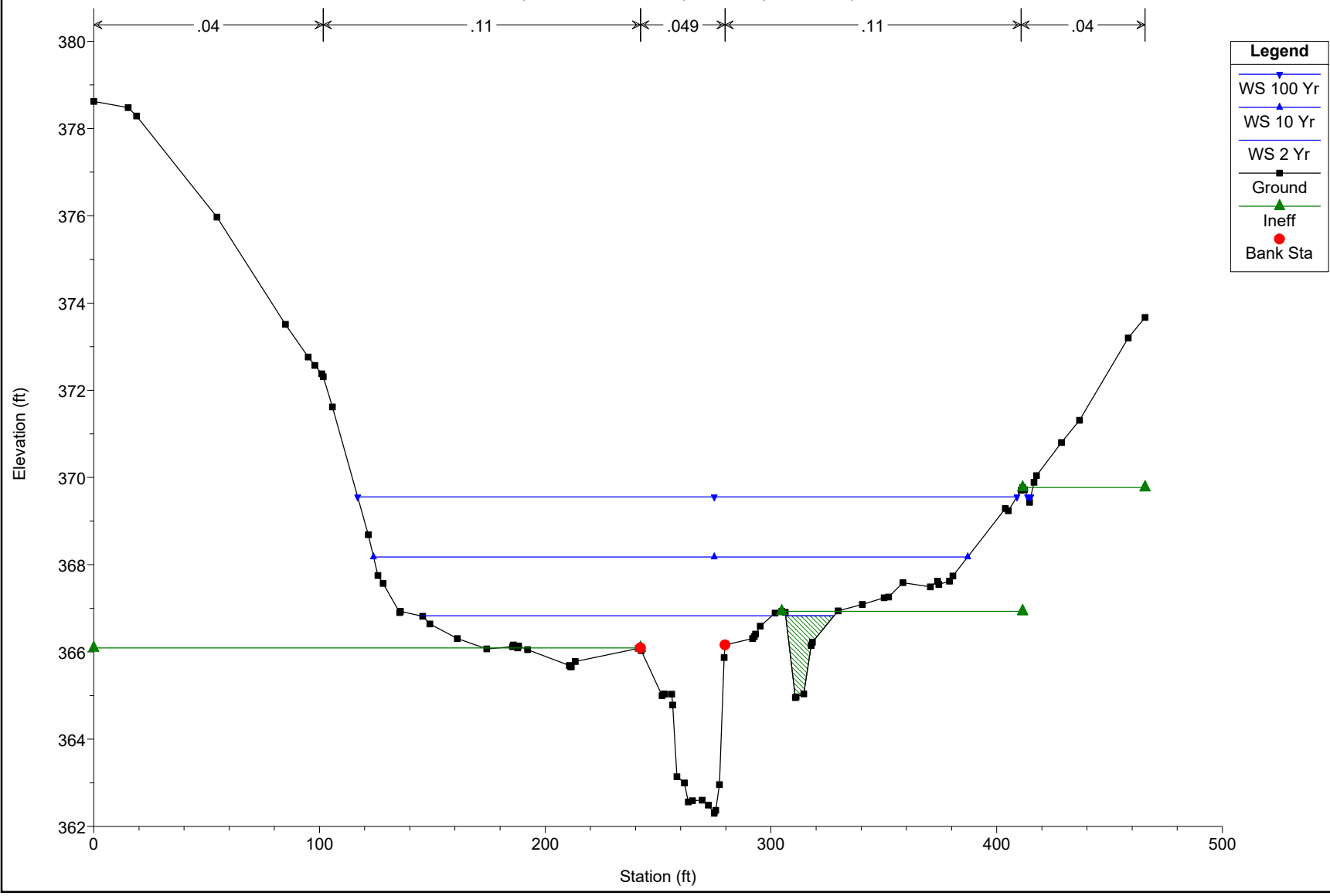


EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = IntsTrib RS = 1

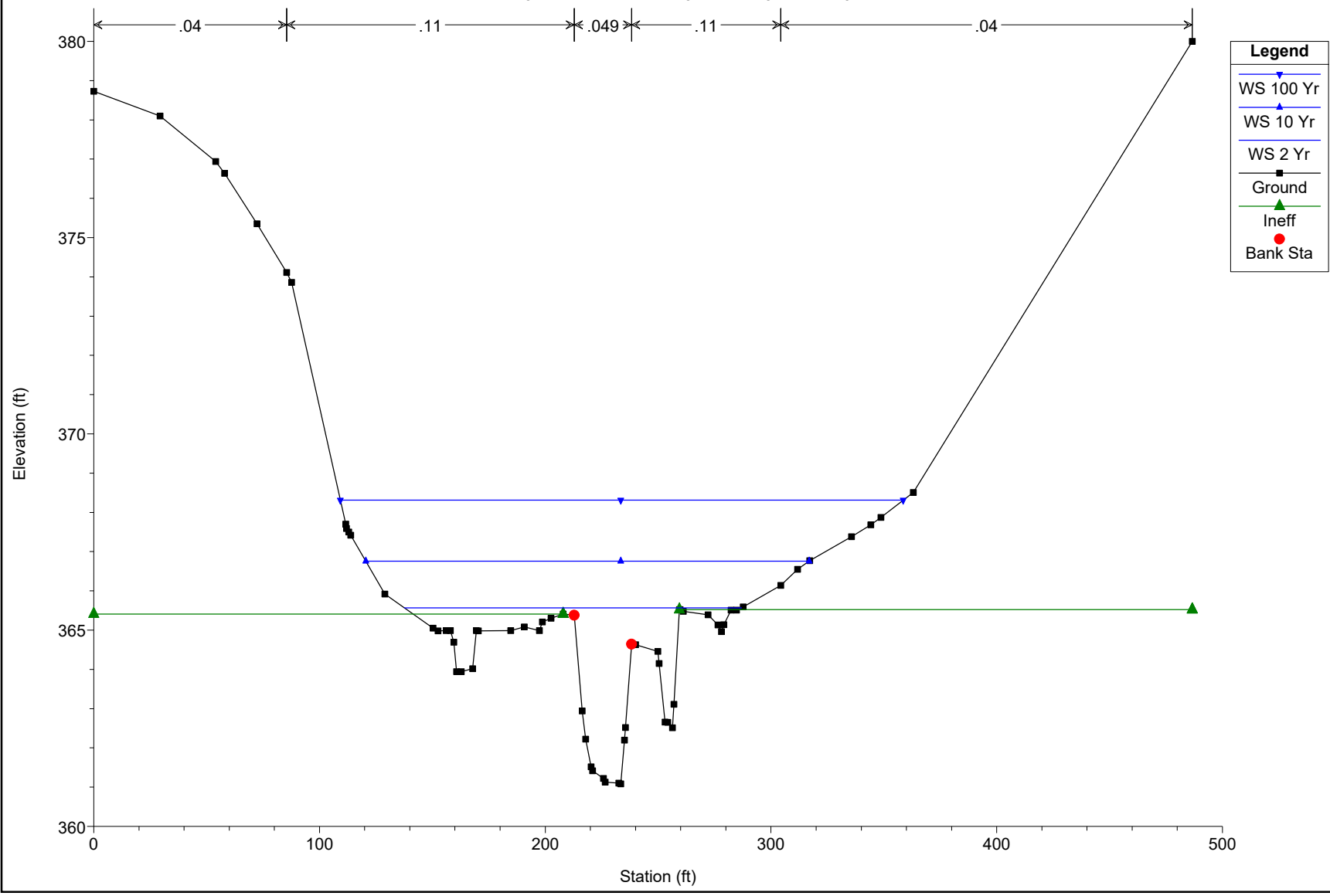


Legend	
WS 100 Yr	Blue line with downward triangle
WS 10 Yr	Blue line with upward triangle
WS 2 Yr	Blue line with downward triangle
Ground	Black line with square
Ineff	Green line with upward triangle
Bank Sta	Red dot

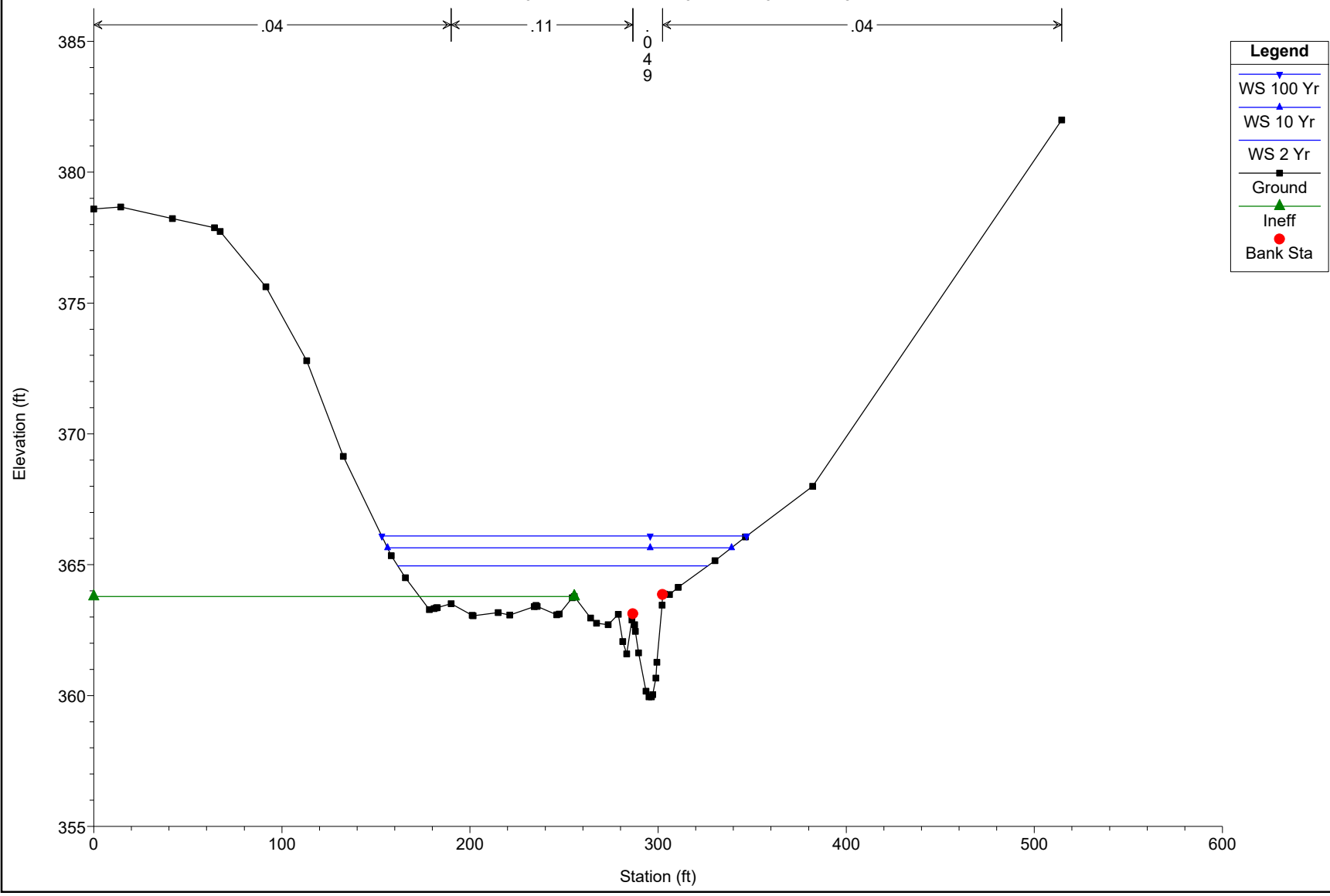
EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 10



EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 9



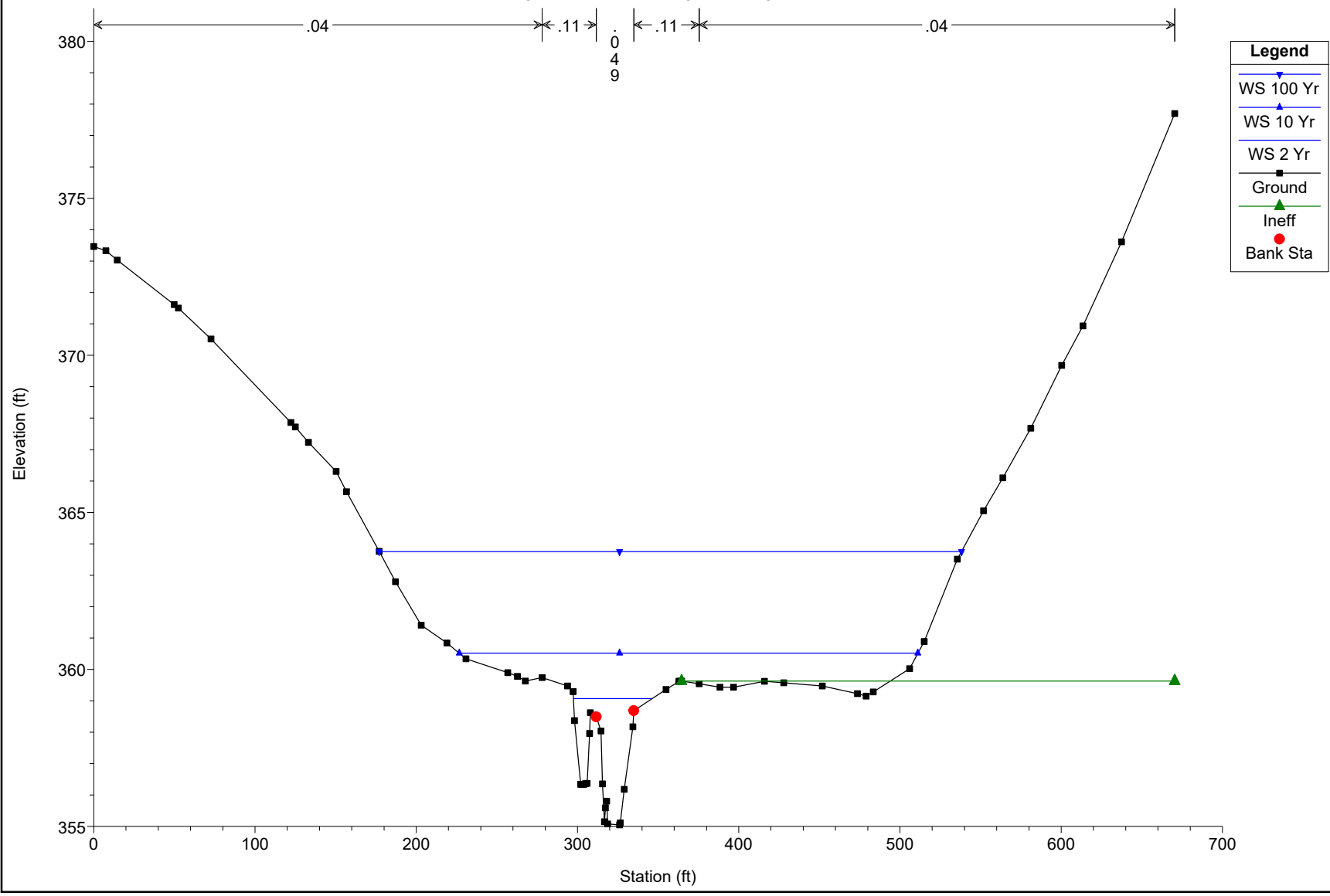
EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 8



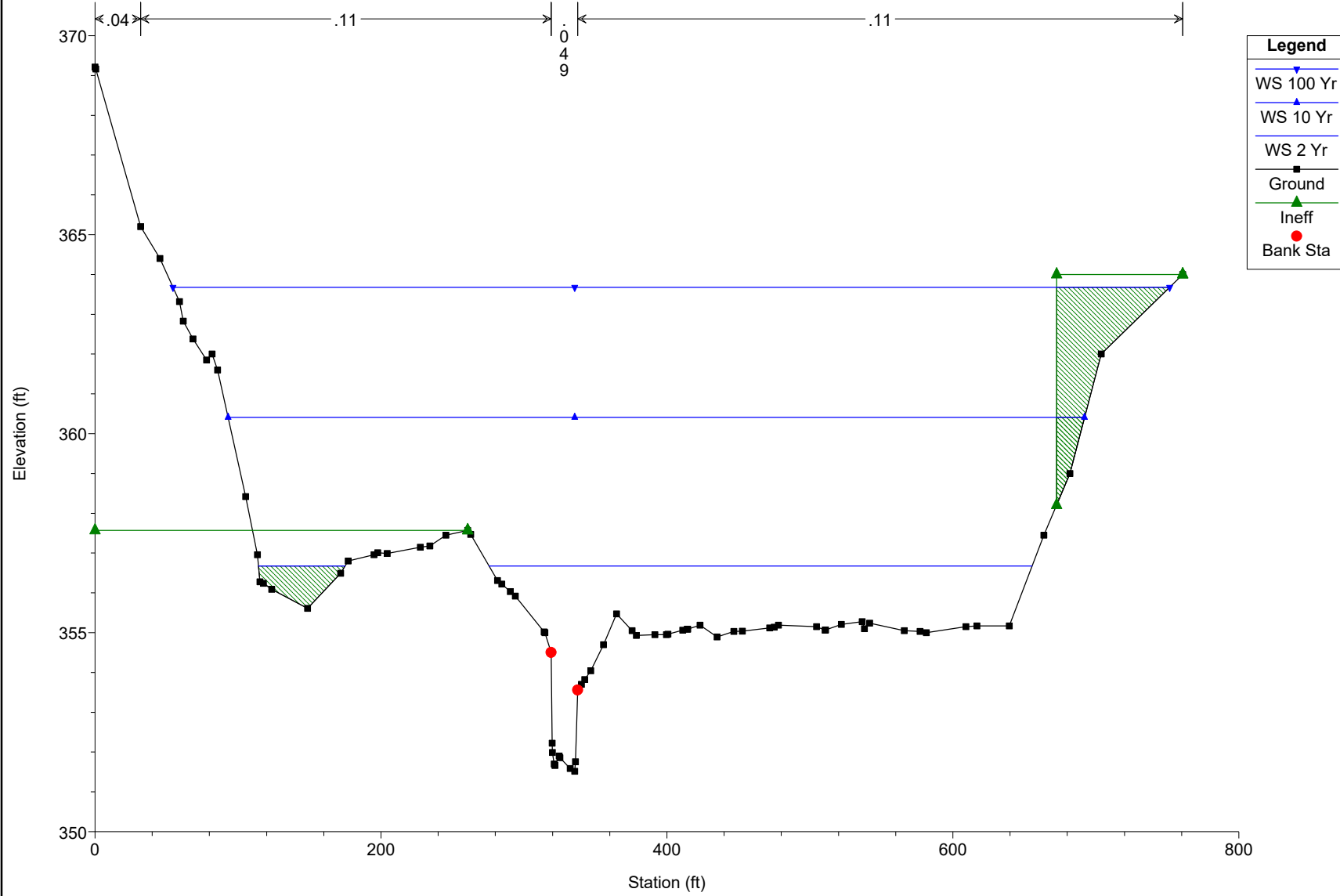
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = JonesFalls3 RS = 7



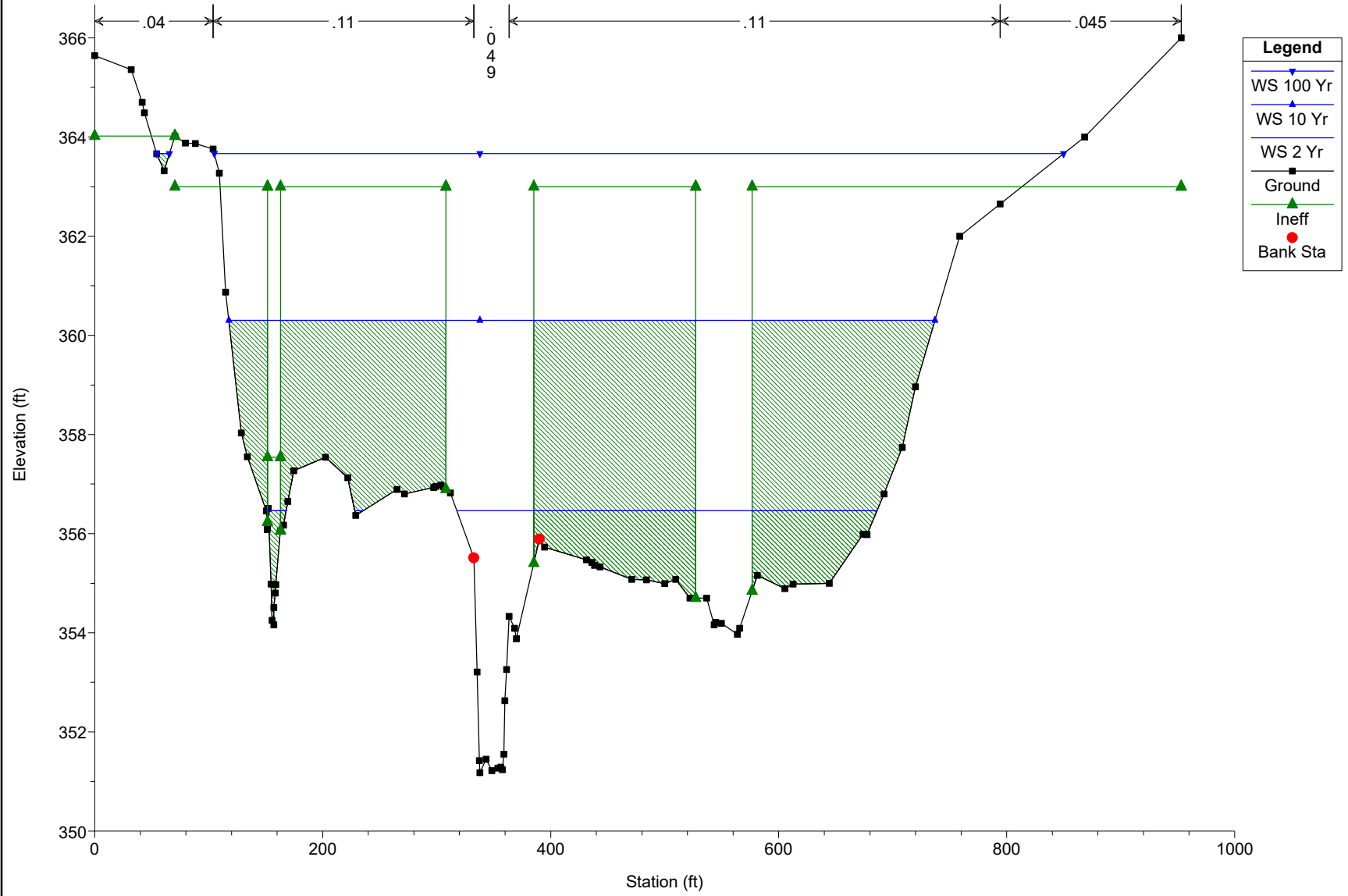
EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 6



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = JonesFalls3 RS = 5

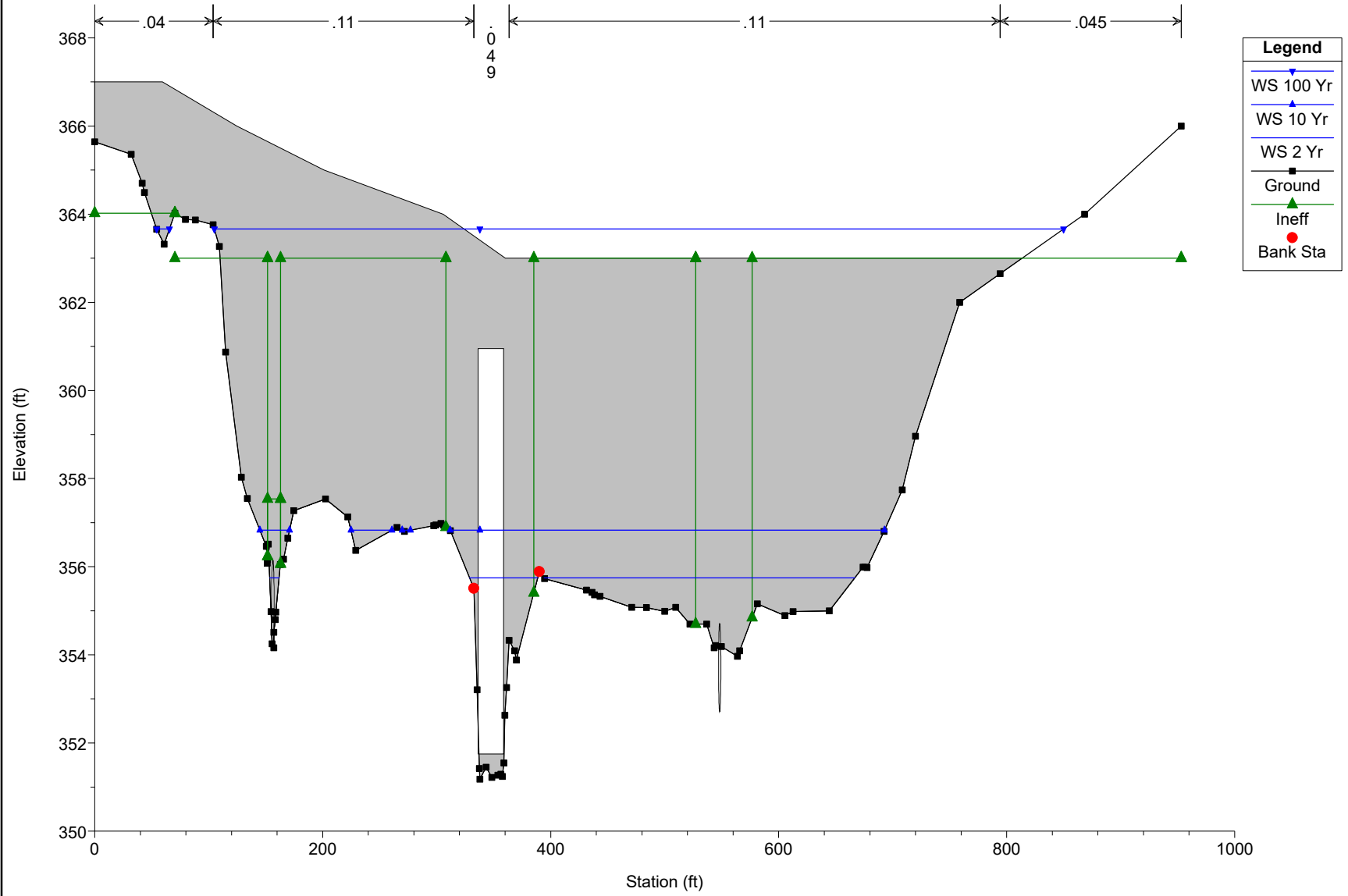




EcclestonMod Plan: existing 1/11/2021

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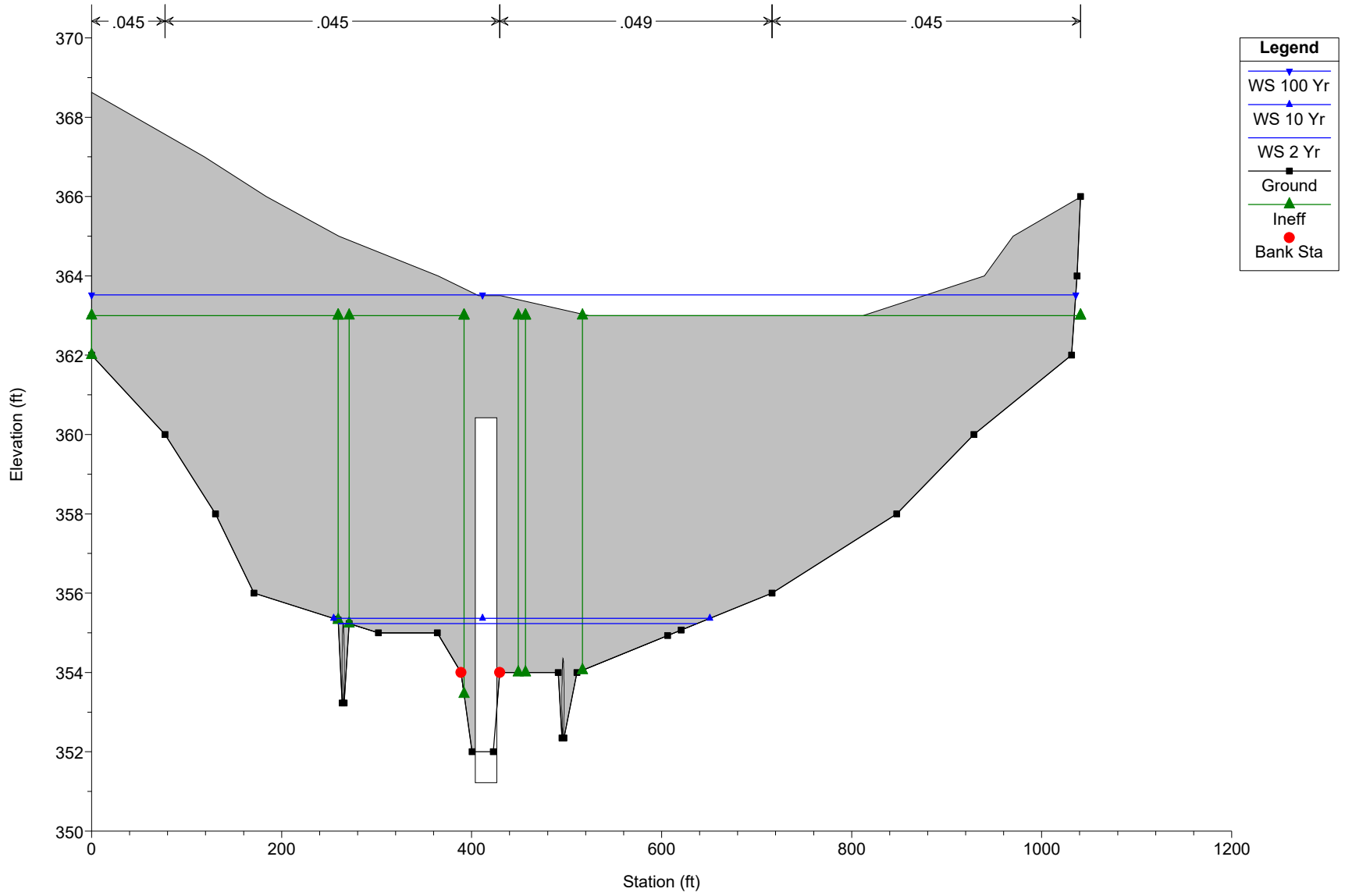
River = JonesFalls Reach = JonesFalls3 RS = 4.5 Culv Bridge over Jones Falls on Park Heights Ave.



EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

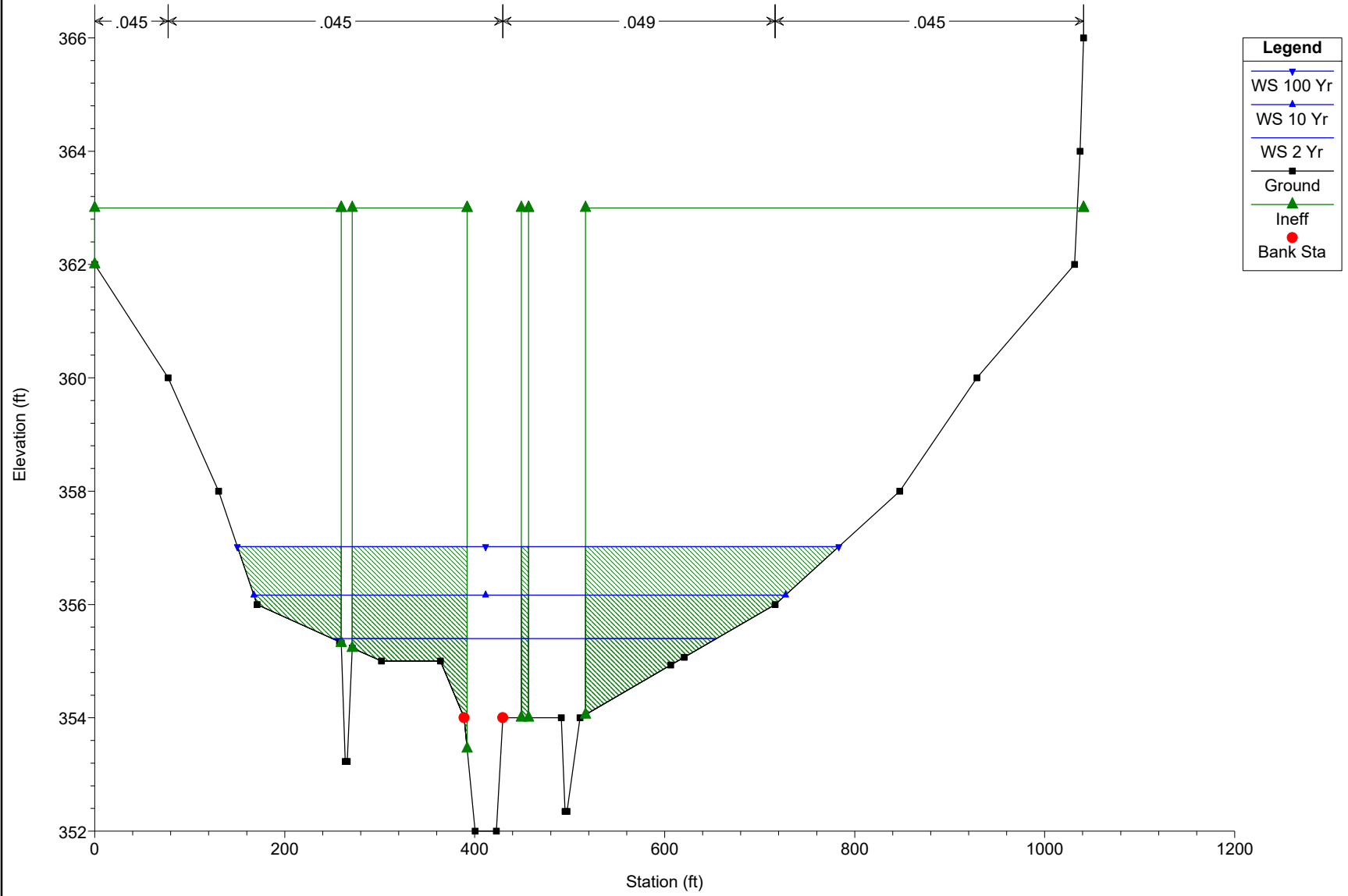
River = JonesFalls Reach = JonesFalls3 RS = 4.5 Culv Bridge over Jones Falls on Park Heights Ave.



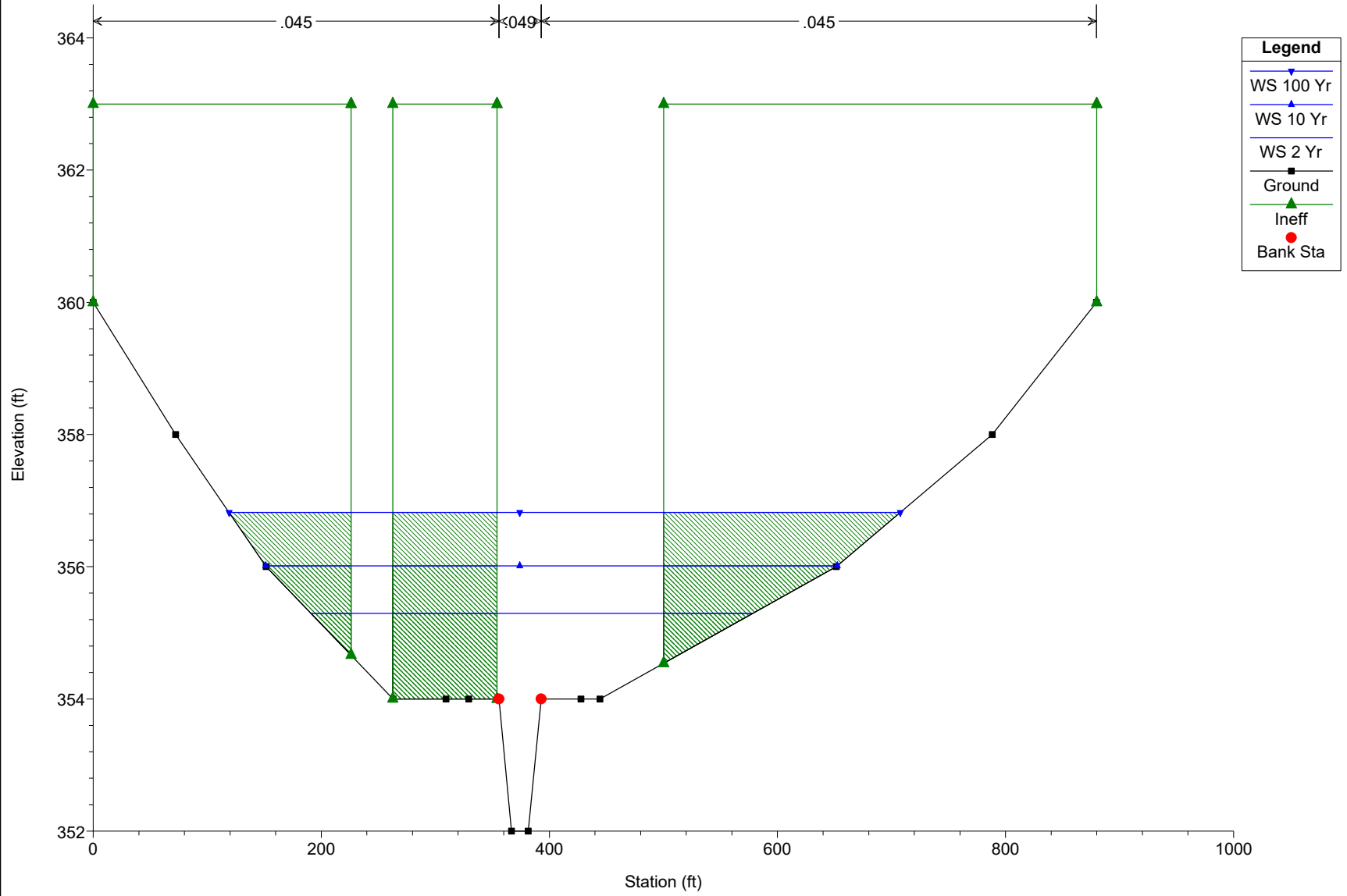
EcclestonMod Plan: existing 1/11/2021

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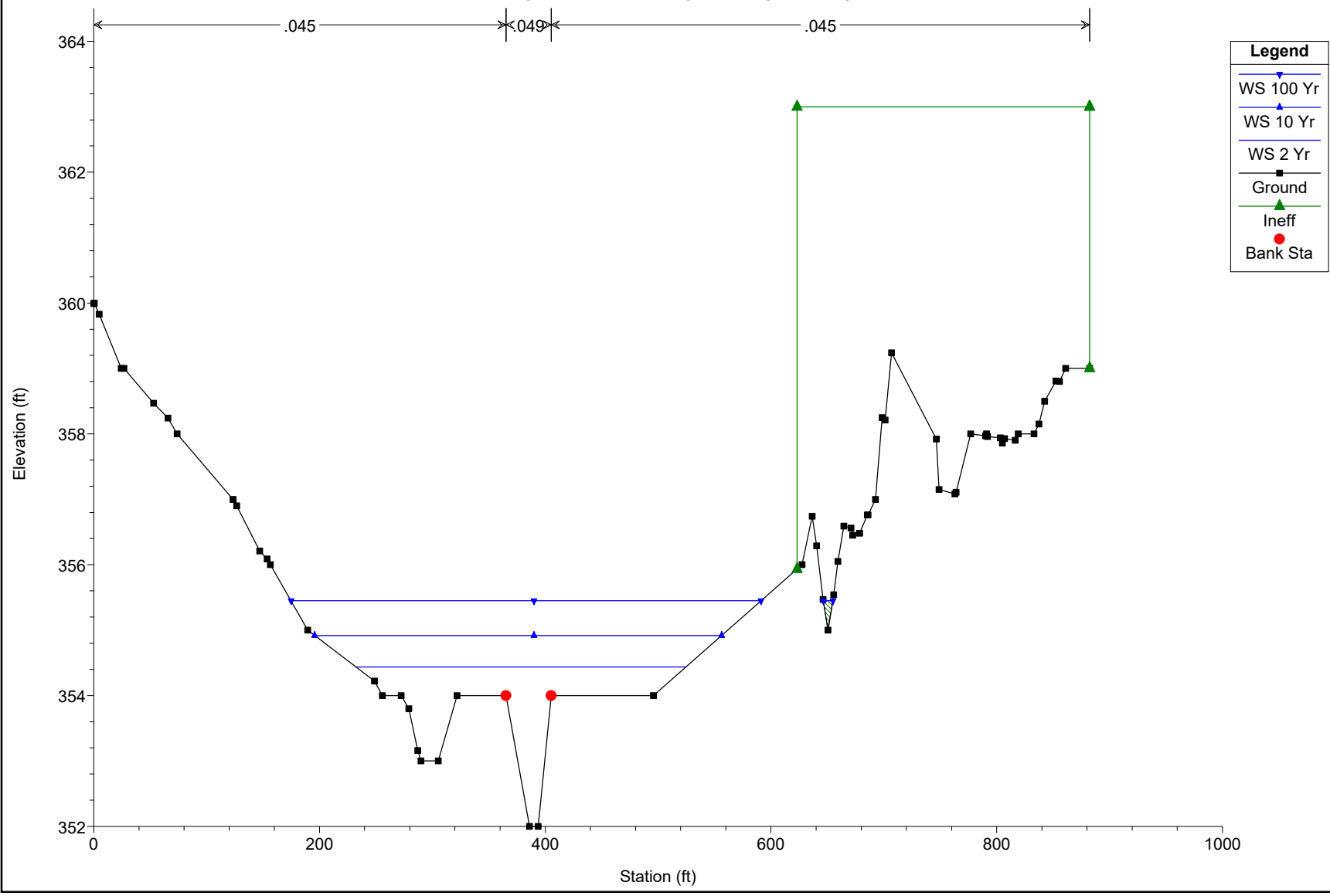
River = JonesFalls Reach = JonesFalls3 RS = 4



EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 3.5



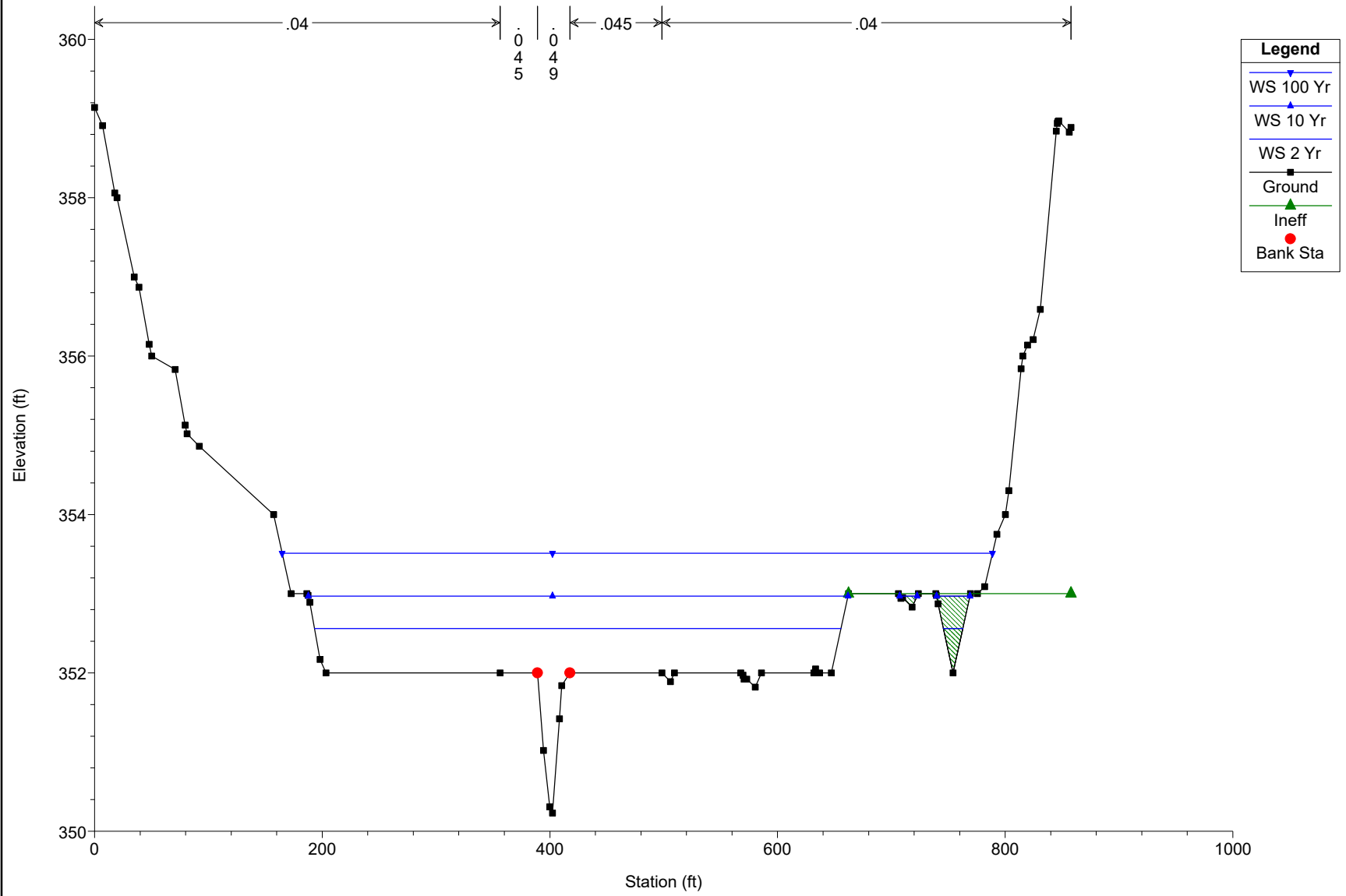
EcclestonMod Plan: existing 1/11/2021  
 Geom: ExistGeoECC  
 River = JonesFalls Reach = JonesFalls3 RS = 3



EcclestonMod Plan: existing 1/11/2021

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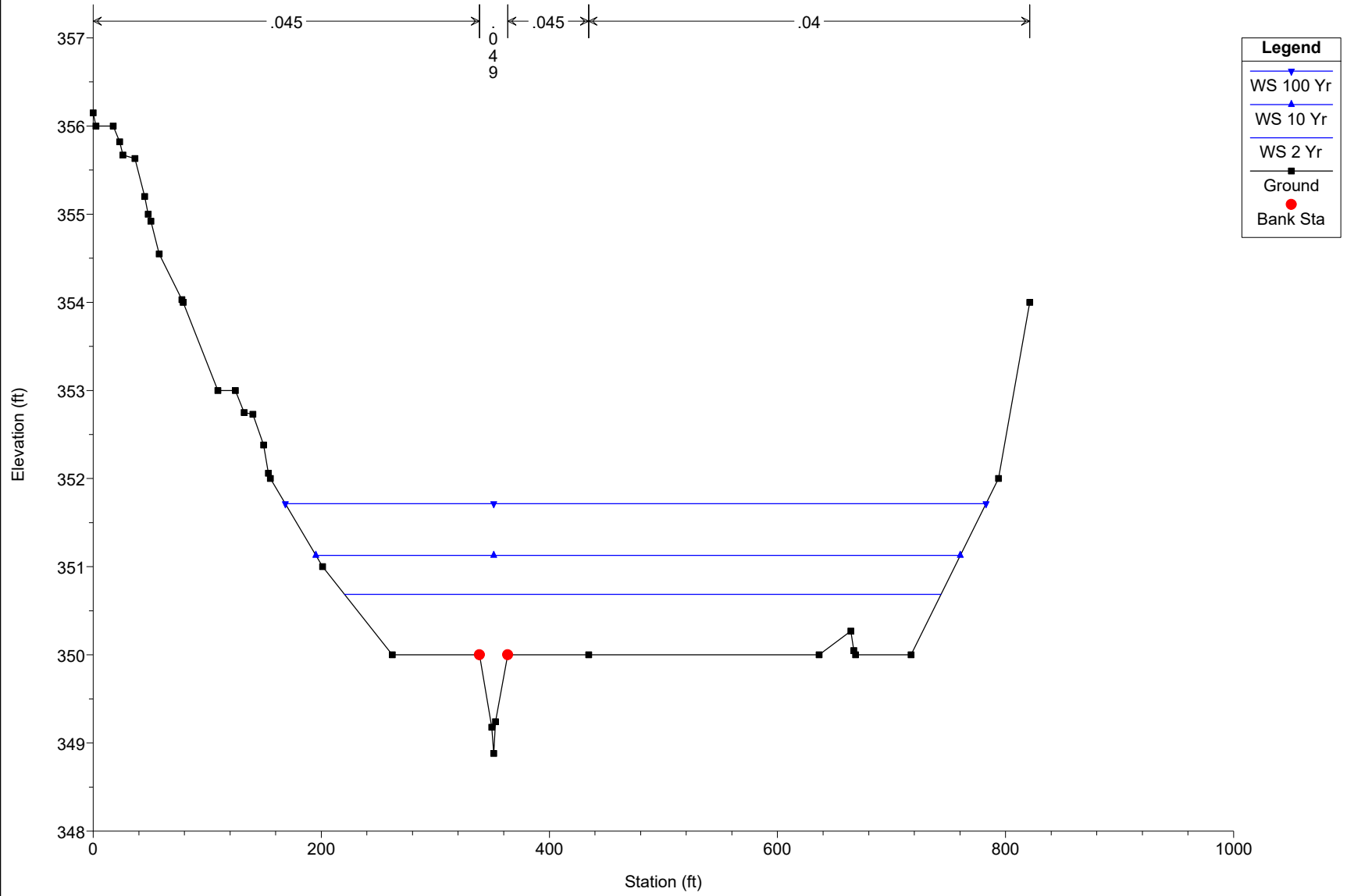
River = JonesFalls Reach = JonesFalls4 RS = 2



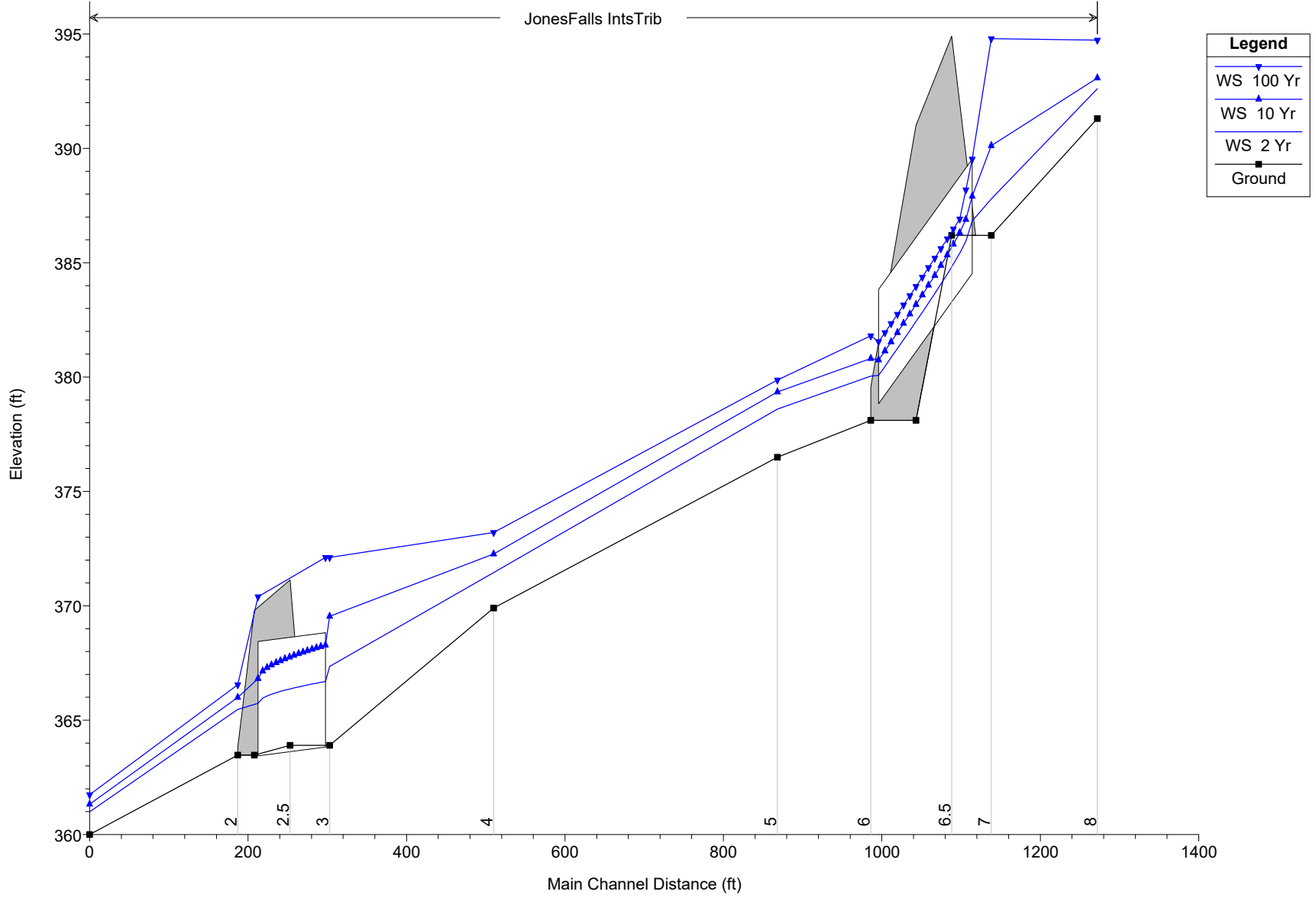
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

River = JonesFalls Reach = JonesFalls4 RS = 1

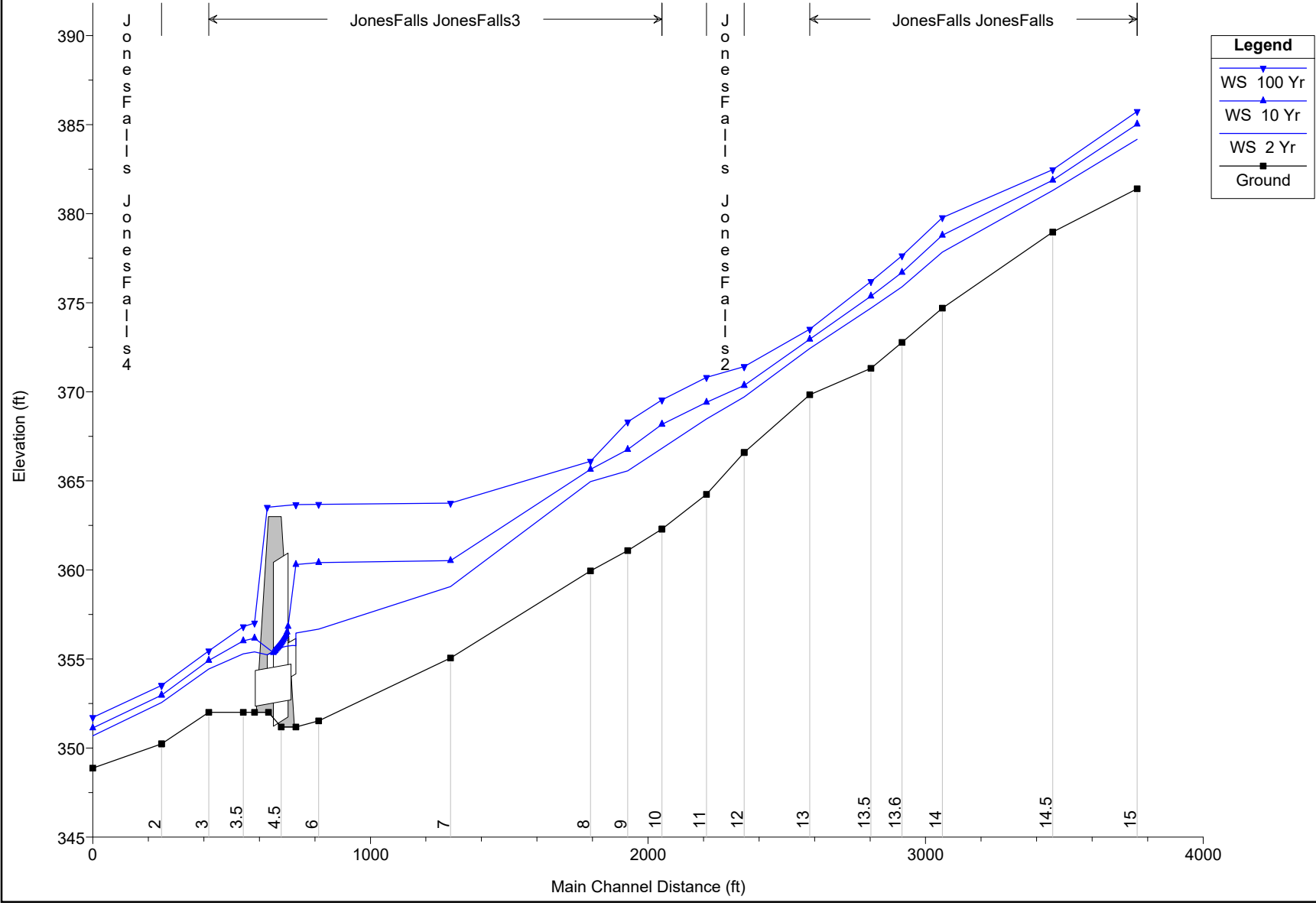


EcclestonMod Plan: existing 1/11/2021  
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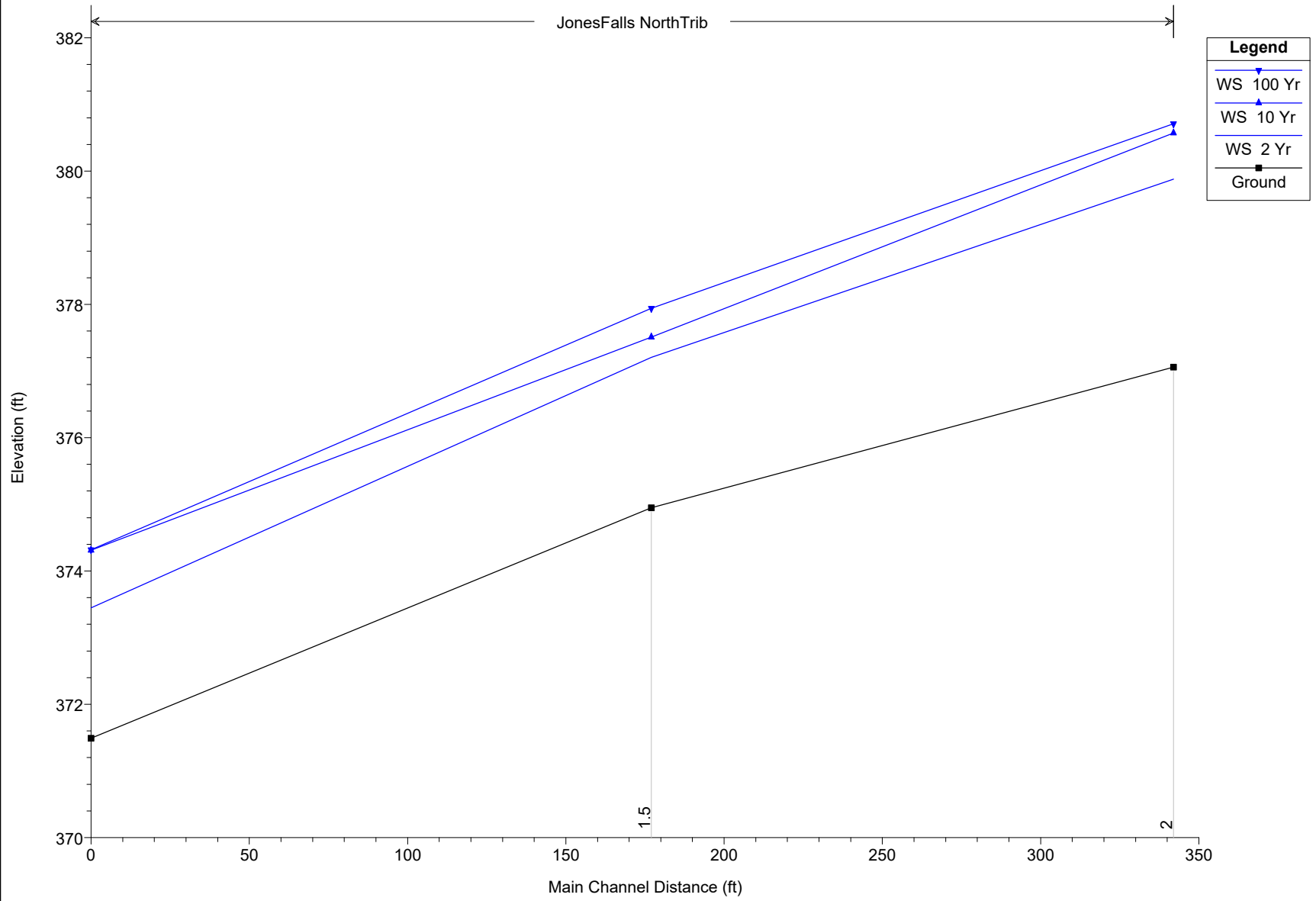
EcclestonMod Plan: existing 1/11/2021  
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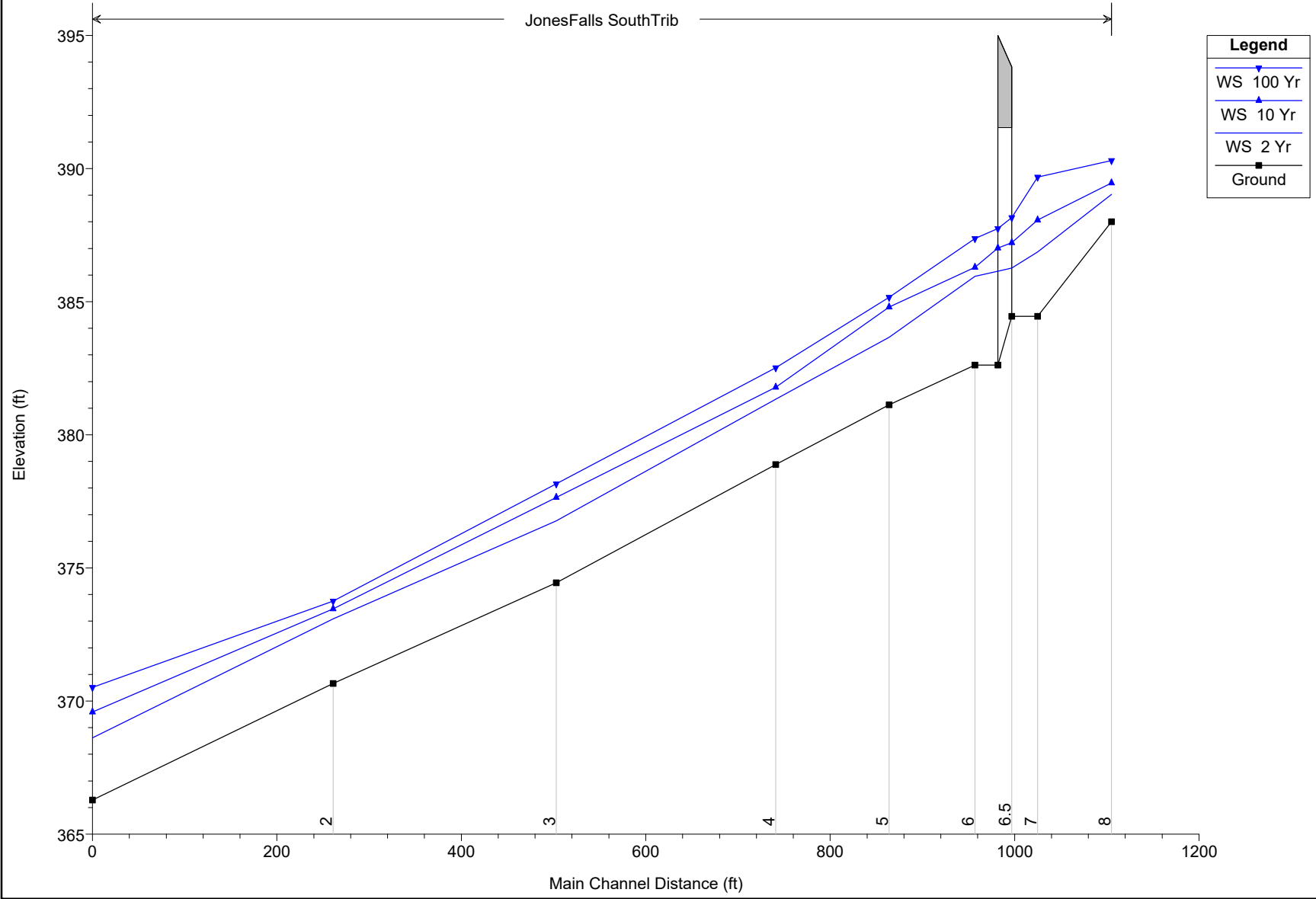
EcclestonMod Plan: existing 1/11/2021

Geom: ExistGeoECC

JonesFalls NorthTrib



EcclestonMod Plan: existing 1/11/2021  
Geom: ExistGeoECC



HEC-RAS Plan: existing

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
JonesFalls	15	2 Yr	363.30	381.40	384.17	383.83	139.48	136.40	0.56	0.58	1.27	0.46	0.54	2.60	4.96	1.12	2.76
JonesFalls	15	10 Yr	798.40	381.40	385.03	384.51	278.02	359.68	0.60	0.87	1.76	0.75	0.85	2.87	6.14	1.55	3.22
JonesFalls	15	100 Yr	1593.40	381.40	385.74	385.28	634.23	440.54	0.62	0.83	2.16	1.06	0.67	2.51	7.02	1.95	2.13
JonesFalls	14.5	2 Yr	363.30	378.97	381.30	380.96	199.86	236.31	0.58	0.58	0.94	0.30	0.63	1.82	3.95	0.82	1.35
JonesFalls	14.5	10 Yr	798.40	378.97	381.88	381.42	342.69	378.62	0.66	0.99	1.54	0.73	1.00	2.33	5.34	1.44	1.79
JonesFalls	14.5	100 Yr	1593.40	378.97	382.47	382.00	632.33	456.61	0.70	1.11	2.07	1.13	1.02	2.52	6.46	1.92	2.00
JonesFalls	14	2 Yr	363.30	374.70	377.85	377.63	191.16	206.05	0.54	0.49	1.17	0.32	0.45	1.90	4.76	0.89	1.12
JonesFalls	14	10 Yr	798.40	374.70	378.79	378.14	446.32	352.77	0.51	0.55	1.28	0.57	0.49	1.79	5.25	1.37	1.23
JonesFalls	14	100 Yr	1593.40	374.70	379.78	378.73	846.33	427.70	0.48	0.65	1.35	0.49	0.67	1.88	5.64	1.28	1.57
JonesFalls	13.6	2 Yr	363.30	372.77	375.90	375.90	108.21	113.34	0.73	0.90	2.20	0.40	0.66	3.36	6.55	0.94	1.30
JonesFalls	13.6	10 Yr	798.40	372.77	376.70	376.70	214.72	157.95	0.84	1.53	3.49	0.89	1.30	3.72	8.64	1.55	1.99
JonesFalls	13.6	100 Yr	1593.40	372.77	377.64	377.64	390.80	216.73	0.91	2.21	4.86	1.50	1.98	4.08	10.63	2.16	2.61
JonesFalls	13.5	2 Yr	363.30	371.32	374.69	374.33	130.61	149.40	0.48	0.34	0.90	0.26	0.18	2.78	4.20	2.22	0.64
JonesFalls	13.5	10 Yr	798.40	371.32	375.37	374.97	240.71	174.79	0.51	0.56	1.22	0.52	0.38	3.32	5.09	3.36	1.04
JonesFalls	13.5	100 Yr	1593.40	371.32	376.19	375.61	397.04	211.20	0.55	0.83	1.67	0.84	0.59	4.01	6.19	4.59	1.37
JonesFalls	13	2 Yr	363.30	369.84	372.43	372.43	105.88	141.36	0.76	0.83	1.73	0.58	0.28	3.43	5.44	1.79	1.97
JonesFalls	13	10 Yr	798.40	369.84	372.95	372.95	187.75	174.11	0.83	1.29	2.48	1.05	0.44	4.25	6.83	2.94	2.63
JonesFalls	13	100 Yr	1593.40	369.84	373.52	373.52	293.48	205.02	0.91	1.94	3.53	1.72	0.84	5.43	8.47	4.29	3.97
NorthTrib	2	2 Yr	175.50	377.06	379.88	379.88	30.02	143.82	0.81	2.23	3.44		0.55	5.85	6.20		2.79
NorthTrib	2	10 Yr	459.70	377.06	380.57	380.57	231.31	206.33	0.27	0.24	0.46	0.22	0.18	1.99	2.39	1.91	1.92
NorthTrib	2	100 Yr	1025.90	377.06	380.71	380.58	260.69	209.16	0.51	0.93	1.74	0.87	0.71	3.94	4.67	3.80	3.92
NorthTrib	1.5	2 Yr	175.50	374.95	377.21	376.91	75.33	175.38	0.44	0.29	1.01	0.14	0.38	2.33	3.41	0.50	2.72
NorthTrib	1.5	10 Yr	459.70	374.95	377.51	377.51	142.43	236.79	0.64	0.69	2.38	0.46	0.96	3.23	5.37	1.32	4.47
NorthTrib	1.5	100 Yr	1025.90	374.95	377.94	377.94	254.11	276.51	0.71	1.22	3.31	1.02	1.41	4.04	6.52	2.40	5.64
NorthTrib	1	2 Yr	175.50	371.49	373.45	373.45	27.89	125.49	1.00	3.94	3.94			6.29	6.29		
NorthTrib	1	10 Yr	459.70	371.49	374.31	374.31	212.41	208.10	0.28	0.24	0.44	0.04	0.21	2.16	2.28	0.27	2.14
NorthTrib	1	100 Yr	1025.90	371.49	374.32	374.32	213.96	208.36	0.63	1.17	2.17	0.21	1.05	4.79	5.04	0.60	4.74
JonesFalls2	12	2 Yr	538.80	366.60	369.72	369.47	252.74	311.28	0.46	0.31	0.81	0.28	0.16	2.13	3.90	1.70	0.60
JonesFalls2	12	10 Yr	1258.10	366.60	370.36	369.47	467.36	345.23	0.48	0.51	1.02	0.50	0.29	2.69	4.57	2.52	0.88
JonesFalls2	12	100 Yr	2619.30	366.60	371.42	369.47	869.68	419.98	0.44	0.58	1.04	0.60	0.36	3.01	4.87	3.09	1.09
JonesFalls2	11	2 Yr	538.80	364.25	368.48	368.48	182.58	225.79	0.62	0.54	1.68	0.38	0.47	2.95	5.79	1.43	1.10
JonesFalls2	11	10 Yr	1258.10	364.25	369.41	369.04	415.18	276.22	0.57	0.78	1.75	0.72	0.53	3.03	6.19	2.39	1.24
JonesFalls2	11	100 Yr	2619.30	364.25	370.81	369.78	851.47	349.70	0.48	0.80	1.53	0.82	0.58	3.08	6.13	2.92	1.42
SouthTrib	8	2 Yr	151.70	388.00	389.03	389.03	34.45	57.16	1.00	2.11	2.11			4.40	4.40		
SouthTrib	8	10 Yr	322.20	388.00	389.46	389.46	63.03	76.80	0.99	2.57	2.57			5.11	5.11		
SouthTrib	8	100 Yr	622.10	388.00	390.30	389.94	147.54	139.26	0.65	1.17	1.55	0.17	0.16	4.22	4.33	1.24	1.24
SouthTrib	7	2 Yr	151.70	384.45	386.87	385.94	48.49	31.81	0.42	0.75	0.75			3.13	3.13		
SouthTrib	7	10 Yr	322.20	384.45	388.06	386.70	86.68	42.04	0.41	0.90	0.94		0.11	3.72	3.73		1.14
SouthTrib	7	100 Yr	622.10	384.45	389.68	387.69	143.40	51.54	0.38	1.06	1.09		0.50	4.34	4.36		3.28
SouthTrib	6.5		Bridge														
SouthTrib	6	2 Yr	151.70	382.62	385.95	384.86	40.81	18.22	0.44	1.00	1.00			3.72	3.72		
SouthTrib	6	10 Yr	322.20	382.62	386.29	385.83	47.16	19.08	0.76	3.28	3.29			6.83	6.83		0.27
SouthTrib	6	100 Yr	622.10	382.62	387.37	387.37	92.00	88.34	0.73	2.08	3.83	0.93	0.59	6.76	7.82	3.85	2.24

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
SouthTrib	5	2 Yr	151.70	381.13	383.66	383.66	20.82	16.21	1.00	4.34	4.34			7.28	7.28		
SouthTrib	5	10 Yr	322.20	381.13	384.80	384.80	84.68	107.81	0.59	0.68	2.06	0.46	0.48	3.81	5.45	2.59	2.63
SouthTrib	5	100 Yr	622.10	381.13	385.16	385.16	125.97	117.38	0.68	1.21	3.04	0.95	0.88	4.94	6.76	4.10	3.74
SouthTrib	4	2 Yr	151.70	378.88	381.33	380.62	42.99	46.96	0.51	0.62	1.08	0.18	0.05	3.53	3.69	1.59	0.50
SouthTrib	4	10 Yr	322.20	378.88	381.79	381.60	66.03	55.43	0.68	1.32	2.26	0.44	0.57	4.88	5.56	2.66	2.21
SouthTrib	4	100 Yr	622.10	378.88	382.52	382.23	122.40	136.79	0.71	0.98	2.99	0.83	0.43	5.08	6.74	4.07	2.25
SouthTrib	3	2 Yr	151.70	374.44	376.77	376.77	28.02	37.79	0.84	1.50	3.13	0.26	0.48	5.41	6.34	1.74	0.94
SouthTrib	3	10 Yr	322.20	374.44	377.65	377.65	90.10	106.62	0.66	0.88	2.59	0.61	0.62	3.58	6.16	3.36	1.62
SouthTrib	3	100 Yr	622.10	374.44	378.16	378.16	148.71	122.45	0.73	1.44	3.53	1.01	1.23	4.18	7.41	4.60	2.50
SouthTrib	2	2 Yr	151.70	370.66	373.09		76.50	116.32	0.41	0.29	0.68	0.26	0.04	1.98	2.92	1.54	0.60
SouthTrib	2	10 Yr	322.20	370.66	373.46		127.06	161.84	0.49	0.47	1.13	0.47	0.16	2.54	3.90	2.16	1.50
SouthTrib	2	100 Yr	622.10	370.66	373.76	373.55	182.66	206.71	0.62	0.81	1.99	0.86	0.37	3.41	5.28	3.04	2.47
SouthTrib	1	2 Yr	151.70	366.28	368.61	368.61	23.58	20.20	1.00	3.33	3.55	0.03		6.43	6.44	0.15	
SouthTrib	1	10 Yr	322.20	366.28	369.59	369.59	100.09	135.21	0.72	0.96	2.45	0.74	0.49	3.22	5.73	1.34	1.02
SouthTrib	1	100 Yr	622.10	366.28	370.51	370.10	265.52	232.30	0.55	0.75	1.81	0.61	0.66	2.34	5.26	1.40	1.40
IntsTrib	8	2 Yr	66.30	391.30	392.61	392.61	14.04	20.65	1.01	1.52	1.52			4.72	4.72		
IntsTrib	8	10 Yr	138.90	391.30	393.07	393.07	25.18	27.38	1.01	1.88	1.88			5.52	5.52		
IntsTrib	8	100 Yr	261.20	391.30	394.73		100.24	78.31	0.35	0.23	0.39	0.07	0.07	2.61	2.87	0.91	0.91
IntsTrib	7	2 Yr	66.30	386.20	387.79	387.79	11.70	12.59	0.93	1.58	2.01		0.27	5.67	5.85		1.54
IntsTrib	7	10 Yr	138.90	386.20	390.11	388.54	86.64	58.39	0.20	0.09	0.19	0.04	0.08	1.60	2.17	0.88	1.35
IntsTrib	7	100 Yr	261.20	386.20	394.80	389.18	464.96	269.56	0.04	0.01	0.02	0.01	0.01	0.56	0.71	0.60	0.51
IntsTrib	6.5		Culvert														
IntsTrib	6	2 Yr	66.30	378.11	380.04	379.71	14.21	14.15	0.68	1.15	1.15			4.67	4.67		
IntsTrib	6	10 Yr	138.90	378.11	380.82	380.43	21.70	16.29	0.75	1.89	1.89			6.40	6.40		
IntsTrib	6	100 Yr	261.20	378.11	381.80	381.40	31.25	18.98	0.82	2.85	2.85			8.36	8.36		
IntsTrib	5	2 Yr	66.30	376.50	378.60	378.21	16.12	12.60	0.64	0.92	0.92			4.11	4.11		
IntsTrib	5	10 Yr	138.90	376.50	379.35	378.91	26.76	15.42	0.69	1.30	1.30			5.19	5.19		
IntsTrib	5	100 Yr	261.20	376.50	379.88	379.73	35.29	17.21	0.91	2.48	2.48			7.40	7.40		
IntsTrib	4	2 Yr	66.30	369.90	371.45	371.45	11.27	10.56	1.00	2.07	2.07			5.88	5.88		
IntsTrib	4	10 Yr	138.90	369.90	372.27	372.27	21.22	15.81	1.00	2.38	2.38			6.54	6.54		
IntsTrib	4	100 Yr	261.20	369.90	373.21	373.21	46.02	44.12	0.80	1.10	1.92	0.30	0.11	5.68	6.23	2.06	0.93
IntsTrib	3	2 Yr	66.30	363.90	367.36	366.19	27.39	25.28	0.34	0.31	0.31			2.42	2.42		
IntsTrib	3	10 Yr	138.90	363.90	369.54	366.96	74.25	109.70	0.18	0.14	0.14			1.87	1.87		
IntsTrib	3	100 Yr	261.20	363.90	372.12	367.77	609.36	280.76	0.05	0.01	0.01	0.00	0.01	0.43	0.63	0.27	0.40
IntsTrib	2.5		Culvert														
IntsTrib	2	2 Yr	66.30	363.47	365.47	365.19	14.87	17.64	0.68	0.78	1.36		0.24	4.46	5.00		1.60
IntsTrib	2	10 Yr	138.90	363.47	365.99	365.99	25.77	74.16	0.78	1.38	2.06		0.87	5.39	6.36		3.65
IntsTrib	2	100 Yr	261.20	363.47	366.55	366.55	37.97	102.29	0.85	2.20	2.89	0.60	1.75	6.88	7.84	2.82	5.73
IntsTrib	1	2 Yr	66.30	360.00	360.99	360.99	16.55	33.41	1.00	1.21	1.21			4.01	4.01		
IntsTrib	1	10 Yr	138.90	360.00	361.32	361.32	28.89	40.42	0.98	1.42	1.55		0.32	4.81	4.88		1.75
IntsTrib	1	100 Yr	261.20	360.00	361.74	361.74	47.53	49.02	0.95	1.64	1.89		0.63	5.50	5.71		2.80
JonesFalls3	10	2 Yr	655.40	362.30	366.83	365.98	192.07	177.03	0.54	0.59	1.33	0.36	0.21	3.41	5.24	0.98	0.69
JonesFalls3	10	10 Yr	1520.50	362.30	368.18	367.49	523.94	263.18	0.52	0.77	1.57	0.74	0.50	2.90	6.07	1.64	1.26

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
JonesFalls3	10	100 Yr	3144.30	362.30	369.55	368.59	904.97	293.45	0.57	1.31	2.27	1.33	0.99	3.47	7.63	2.39	1.95
JonesFalls3	9	2 Yr	655.40	361.08	365.57	364.46	169.85	149.32	0.61	0.66	1.92	0.34	0.45	3.86	6.46	0.90	1.09
JonesFalls3	9	10 Yr	1520.50	361.08	366.76	366.61	379.17	196.38	0.69	1.32	3.02	1.09	1.00	4.01	8.51	1.92	1.98
JonesFalls3	9	100 Yr	3144.30	361.08	368.31	367.88	729.17	249.35	0.67	1.73	3.46	1.72	1.34	4.31	9.56	2.68	3.05
JonesFalls3	8	2 Yr	655.40	359.95	364.96	364.31	284.48	165.00	0.43	0.53	1.03	0.52	0.19	2.30	4.75	1.65	1.91
JonesFalls3	8	10 Yr	1520.50	359.95	365.64	365.13	403.98	182.89	0.62	1.32	2.38	1.38	0.60	3.76	7.44	2.93	3.62
JonesFalls3	8	100 Yr	3144.30	359.95	366.09	366.09	489.32	193.98	0.99	3.66	6.35	3.92	1.79	6.43	12.34	5.15	6.49
JonesFalls3	7	2 Yr	655.40	355.05	359.07	359.07	90.56	48.88	0.95	2.73	4.06	2.29	0.31	7.24	9.02	2.74	0.73
JonesFalls3	7	10 Yr	1520.50	355.05	360.52	360.52	373.81	284.48	0.64	0.83	2.44	0.66	0.67	4.07	7.48	2.13	3.17
JonesFalls3	7	100 Yr	3144.30	355.05	363.76	361.16	1436.20	361.17	0.20	0.20	0.34	0.17	0.20	2.19	3.08	1.76	2.24
JonesFalls3	6	2 Yr	655.40	351.52	356.67	355.73	635.24	440.29	0.25	0.16	0.40	0.09	0.15	1.03	3.07	0.51	0.71
JonesFalls3	6	10 Yr	1520.50	351.52	360.41	356.40	2738.24	599.02	0.09	0.05	0.08	0.04	0.06	0.56	1.52	0.42	0.53
JonesFalls3	6	100 Yr	3144.30	351.52	363.68	357.13	4690.01	697.05	0.09	0.07	0.09	0.05	0.07	0.67	1.71	0.54	0.67
JonesFalls3	5	2 Yr	655.40	351.18	356.46	354.55	294.36	392.98	0.28	0.32	0.43	0.06	0.28	2.23	2.99	0.38	1.02
JonesFalls3	5	10 Yr	1520.50	351.18	360.30	355.85	835.35	619.46	0.17	0.28	0.34	0.20	0.29	1.82	2.61	0.97	1.23
JonesFalls3	5	100 Yr	3144.30	351.18	363.67	357.27	5010.39	756.24	0.07	0.05	0.08	0.05	0.05	0.63	1.25	0.52	0.56
JonesFalls3	4.5		Culvert														
JonesFalls3	4	2 Yr	655.40	352.00	355.40	354.43	259.09	404.85	0.33	0.28	0.42	0.17	0.22	2.53	3.25	1.79	1.97
JonesFalls3	4	10 Yr	1520.50	352.00	356.17	355.20	357.64	559.74	0.48	0.75	1.03	0.53	0.65	4.25	5.29	3.41	3.57
JonesFalls3	4	100 Yr	3144.30	352.00	357.02	356.21	467.53	633.07	0.66	1.77	2.28	1.35	1.59	6.73	8.14	5.76	5.88
JonesFalls3	3.5	2 Yr	655.40	352.00	355.29	354.74	260.90	387.48	0.36	0.29	0.54	0.20	0.23	2.51	3.32	1.85	2.07
JonesFalls3	3.5	10 Yr	1520.50	352.00	356.01	355.30	392.29	501.41	0.46	0.65	1.03	0.52	0.57	3.88	4.77	3.28	3.51
JonesFalls3	3.5	100 Yr	3144.30	352.00	356.82	356.07	540.00	588.61	0.58	1.37	1.94	1.16	1.24	5.82	6.78	5.25	5.49
JonesFalls3	3	2 Yr	655.40	352.00	354.44	353.99	194.79	292.70	0.68	0.58	1.41	0.54	0.34	3.36	4.93	2.85	2.07
JonesFalls3	3	10 Yr	1520.50	352.00	354.92	354.84	351.53	360.59	0.76	0.97	2.08	0.91	0.73	4.33	6.26	3.94	3.39
JonesFalls3	3	100 Yr	3144.30	352.00	355.45	355.39	558.20	424.90	0.85	1.55	3.04	1.53	1.25	5.63	7.85	5.42	4.72
JonesFalls4	2	2 Yr	721.70	350.23	352.56	352.43	278.63	479.52	0.56	0.37	0.83	0.34	0.34	2.59	3.67	2.43	2.41
JonesFalls4	2	10 Yr	1659.40	350.23	352.97	352.72	470.41	518.92	0.58	0.61	1.07	0.57	0.58	3.53	4.35	3.45	3.41
JonesFalls4	2	100 Yr	3405.50	350.23	353.51	353.19	817.03	623.96	0.59	0.76	1.32	0.79	0.69	4.17	5.06	4.33	3.92
JonesFalls4	1	2 Yr	721.70	348.88	350.69	350.42	342.99	523.08	0.43	0.25	0.44	0.22	0.25	2.10	2.61	1.77	2.14
JonesFalls4	1	10 Yr	1659.40	348.88	351.13	350.72	583.50	565.06	0.45	0.40	0.61	0.33	0.40	2.84	3.24	2.36	2.95
JonesFalls4	1	100 Yr	3405.50	348.88	351.71	351.13	929.09	614.41	0.48	0.58	0.83	0.49	0.60	3.67	3.99	3.04	3.85

HEC-RAS HEC-RAS 5.0.7 March 2019  
 U.S. Army Corps of Engineers  
 Hydrologic Engineering Center  
 609 Second Street  
 Davis, California

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X      X  XXXXXX   XXXX       XXXX       XX       XXXX
X      X  X       X   X       X   X       X   X       X
X      X  X       X           X   X       X   X       X
XXXXXXXX XXXX     X           XXX  XXXX     XXXXXX     XXXX
X      X  X       X           X   X       X   X           X
X      X  X       X   X       X   X       X   X           X
X      X  XXXXXX   XXXX       X   X       X   X       XXXXX
  
```

PROJECT DATA

Project Title: EcclestonMod  
 Project File : EcclestonMod.prj  
 Run Date and Time: 1/11/2021 3:48:41 PM

Project in English units

PLAN DATA

Plan Title: existing  
 Plan File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design  
 Data\Existing HECRAS Model\EcclestonMod.p01

Geometry Title: ExistGeoECC

Geometry File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working  
 Data\Design Data\Existing HECRAS Model\EcclestonMod.g05

Flow Title : EccNormalDepth\_ULT

Flow File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working  
 Data\Design Data\Existing HECRAS Model\EcclestonMod.f01

Plan Summary Information:

Number of:	Cross Sections =	38	Multiple Openings =	0
	Culverts =	3	Inline Structures =	0
	Bridges =	1	Lateral Structures =	0

Computational Information

Water surface calculation tolerance = 0.01

Critical depth calculation tolerance = 0.01

Maximum number of iterations = 20  
 Maximum difference tolerance = 0.3  
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary  
 Conveyance Calculation Method: At breaks in n values only  
 Friction Slope Method: Average Conveyance  
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: EccNormalDepth\_ULT  
 Flow File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design  
 Data\Existing HECRAS Model\EcclestonMod.f01

Flow Data (cfs)

River	Reach	RS	2 Yr	10 Yr
100 Yr				
JonesFalls	JonesFalls	15	363.3	798.4
1593.4				
JonesFalls	NorthTrib	2	175.5	459.7
1025.9				
JonesFalls	JonesFalls2	12	538.8	1258.1
2619.3				
JonesFalls	SouthTrib	8	151.7	322.2
622.1				
JonesFalls	IntsTrib	8	66.3	138.9
261.2				
JonesFalls	JonesFalls3	10	655.4	1520.5
3144.3				
JonesFalls	JonesFalls4	2	721.7	1659.4
3405.5				

Boundary Conditions

River	Reach	Profile	Upstream
Downstream			
JonesFalls	JonesFalls4	2 Yr	
Normal S = 0.00613			



JonesFalls JonesFalls4 10 Yr  
 Normal S = 0.00613  
 JonesFalls JonesFalls4 100 Yr  
 Normal S = 0.00613

GEOMETRY DATA

Geometry Title: ExistGeoECC  
 Geometry File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design Data\Existing HECRAS Model\EcclestonMod.g05

Reach Connection Table

River	Reach	Upstream Boundary	Downstream Boundary
JonesFalls	JonesFalls		NorthTrib
JonesFalls	NorthTrib		NorthTrib
JonesFalls	JonesFalls2	NorthTrib	SouthTrib
JonesFalls	SouthTrib		SouthTrib
JonesFalls	IntsTrib		IntersTrib
JonesFalls	JonesFalls3	SouthTrib	IntersTrib
JonesFalls	JonesFalls4	IntersTrib	

JUNCTION INFORMATION

Name: IntersTrib  
 Description:  
 Energy computation Method

Length across Junction		Tributary		Length	Angle
River	Reach	River	Reach		
JonesFalls	IntsTrib	to JonesFalls	JonesFalls4	507	
JonesFalls	JonesFalls3	to JonesFalls	JonesFalls4	170	

Name: NorthTrib  
 Description:  
 Energy computation Method

Length across Junction		Tributary		Length	Angle
River	Reach	River	Reach		
JonesFalls	JonesFalls	to JonesFalls	JonesFalls2	237	
JonesFalls	NorthTrib	to JonesFalls	JonesFalls2	239	

Name: SouthTrib  
 Description:

Energy computation Method

Length across River	Junction Reach	Tributary River	Reach	Length	Angle
JonesFalls	JonesFalls2	to JonesFalls	JonesFalls3	161	
JonesFalls	SouthTrib	to JonesFalls	JonesFalls3	93	0

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 15

INPUT

Description:

Station Elevation Data num= 56

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-64.81	388	-18.85	386	0	384.95	10.51	384.63	28.87	384.1
28.94	384.27	39.73	383.95	45.82	383.82	48.75	383.58	66.63	383.12
77.82	382.91	97.75	383.32	105.52	383.13	107.91	381.73	109.11	381.48
112.78	381.42	113.29	381.4	117.45	381.57	118.5	381.88	118.85	382.16
119.91	383.19	122.49	383.07	124.08	383.04	124.54	382.98	124.86	382.97
127.48	381.9	128.4	381.53	128.82	381.49	129.85	381.68	137.78	383.54
152.38	383.34	182.38	385.15	183.51	385.21	188.97	385.23	201.5	385.4
217.33	385.08	228.51	384.75	256.65	384.31	264.53	384.58	267.67	384.77
289.28	384.76	291.21	384.83	292.85	384.58	303.94	384.73	348.05	384.47
350.59	384.93	352.63	384.92	359.02	384.42	368.24	384.57	401.91	385.11
408.16	385.2	430.17	385.85	434.17	385.88	453.73	385.95	456.22	385.96
579.98	388								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
-64.81	.11	105.52	.049	137.78	.11	408.16	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

105.52	119.91	306	304	297	.1	.3
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
-64.81	28.94	384.27	F
201.5	456.22	385.4	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 14.5

INPUT

Description:

Station Elevation Data num= 31

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	390	31.7	388	48.67	386	69.09	384	78.17	383
92.38	382	107.84	381	129.03	380.5	154.65	381	161.43	381.16
168.17	381	175.06	380	180.75	379	181.81	378.97	185.75	379
187.74	379	189.99	380	197.29	380.95	222.86	380	223.27	379.97
223.67	380	286.46	380.04	315.37	381	368.42	382	376.18	382.08
392.93	382	490.79	381.26	525.16	382	561.64	383	586.8	384
664.47	386								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.11	168.17	.049	197.29	.11	490.79	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

168.17	197.29	422	398	305	.1	.3
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Ineffective Flow num= 3

Sta L	Sta R	Elev	Permanent
0	161.43	381.16	F
197.29	376.18	380.95	F
376.18	664.47	382.08	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 14

INPUT

Description:

Station Elevation Data num= 84

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	393.41	20.17	392.15	27.9	390.54	30.04	390.17	39.43	387.67
44.83	386.6	53.87	385	62.57	383.64	72.64	381.84	74.22	381.59
87.3	380.1	94.05	379.49	96.13	379.19	111.3	379.02	118.68	378.84
126.34	378.57	142.18	378.77	147.29	378.9	150.41	378.84	159.21	378.85
159.87	378.89	171.47	377.04	173.59	376.65	175.12	376.75	194.4	377.52
204.12	377.33	205.53	377.41	207.44	377.33	209.93	375.58	210.41	375.38
211.32	374.79	213.49	374.74	215.02	374.7	218.14	374.72	219.9	375.13
220.95	375.75	225.2	376.82	227	376.9	238	377.41	239.38	377.49
240.43	377.47	258.12	376.52	258.76	376.48	259.19	376.37	262.57	375.62
263.06	375.63	265.85	375.56	266.95	375.69	268.21	375.78	270.83	375.67
279.55	376.4	285.8	376.78	289.08	377.3	289.5	377.38	307.02	377.18
320.85	377.09	342.33	377.15	358.57	377.47	365	377.74	373.4	378.06
394.81	378.73	412.38	378.13	427.04	378.29	429.77	378.48	437.37	378.47
447.09	378.4	452.03	378.45	453.19	378.42	459.45	377.71	460.79	377.39
464.24	378.11	465.27	378.27	467.4	378.28	478.15	377.93	479.58	377.88
488.54	378.49	493.54	379.21	496.61	379.3	511.35	379.6	531.03	380.11
539.1	380.17	542.93	380.26	550.7	380.58	568.17	381.15		

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.11	207.44	.049	225.2	.11	550.7	.04

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
207.44	225.2	145	145	147	.1	.3	
Ineffective Flow		num=	4				
Sta L	Sta R	Elev	Permanent				
0	159.87	378.89	F				
159.87	194.4	377.52	F				
239.38	387.85	377.49	F				
394.81	568.17	378.73	F				

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 13.6

INPUT

Description:

Station	Elevation	Data	num=	37					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	394	12.73	393	23.59	392	34.32	391	45.06	390
58.43	389	71.33	388	83.31	387	95.36	386	108.62	385
124.87	384	135.45	383	140.16	382	144.82	381	149.48	380
179.94	379	198.16	378	201.34	377	204.49	376	207.29	375
208.63	374	209.36	373	214	372.77	218.97	373	221.19	374
224.42	375	252.87	375.43	297.54	375	298.96	374.9	303.71	375
315.82	375.44	318.62	376	378.18	377	437.47	378	493.97	379
513.08	380	544.28	382						

Manning's n Values	num=	5							
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	135.45	.11	207.29	.049	224.42	.11	437.47	.04

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
207.29	224.42	134	112	114	.1	.3	
Ineffective Flow		num=	1				
Sta L	Sta R	Elev	Permanent				
252.87	544.28	375.43	F				

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 13.5

INPUT

Description:

Station	Elevation	Data	num=	34					
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Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	384.48	2.19	385	11.27	385	20.56	384	26.86	383
33.9	382	42.03	381	51.6	380	61.31	379	71.02	378
80.73	377	90.95	376	104.59	375	118.35	374	149.87	373.88
184.35	374	187.9	374.5	191.58	374	194.16	373	196.68	372
202.63	371.32	208.97	372	211.01	373	213.84	374	220.74	374.43
237.57	374	240.36	373.86	242.75	374	265.06	375	289.79	376
344.17	377	374.82	378	429.58	380	525.41	382		

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.11	26.86	.04	184.35	.11	191.58	.049	213.84	.11
344.17	.04								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

191.58	213.84	201	220	219	.1	.3
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	187.9	374.5	F
220.74	525.41	374.43	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls RS: 13

INPUT

Description:

Station Elevation Data num= 45

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	379.26	1.71	379.08	22.36	376.5	28.36	376.04	64.52	374.37
64.89	374.36	66.74	374.31	115.95	373.13	123.63	372.92	136.75	372.75
154.83	372.36	181.44	371.93	199.1	372.01	206.77	371.77	241.09	371.66
258.87	371.83	261.45	372	264.39	371.66	270.73	371.32	271.44	371.06
273.68	370.32	274.75	369.84	276.59	369.92	278.99	370.02	279.62	370
281.82	370.03	282.98	370.08	283.77	370.18	285.41	370.79	288.1	370.95
291.46	371.9	293.92	372.71	301.28	373.39	302.48	373.9	303.77	373.23
312.88	374.16	317.73	374.36	318.24	374.42	331.73	374.78	332.39	374.78
333.19	374.81	335.31	374.85	374.73	375.42	403.19	376.44	433.72	377.65

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	206.77	.11	261.45	.049	291.46	.04

Bank Sta: Left Right Coeff Contr. Expan.

261.45	291.46	.1	.3
--------	--------	----	----

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	261.45	372	F

302.48 433.72 373.9 F

CROSS SECTION

RIVER: JonesFalls  
REACH: NorthTrib RS: 2

INPUT

Description:

Station Elevation Data		num= 50							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-126.44	388	0	380.22	12.77	379.9	22.81	379.65	37.96	379.2
42.95	379.16	54.09	379.05	75.73	378.88	86.45	378.95	88.4	378.92
96.27	378.89	101.95	378.96	110.35	378.93	122.55	379.33	123.54	379.4
140.55	380.56	141	380.56	141.73	380.57	162.9	380.3	165.85	380.32
170.52	380.32	171.95	378.46	173.1	378.22	174.55	377.06	179.21	377.61
182.33	378.27	183.9	378.76	185.85	379.92	189.2	379.5	191.11	379.5
197.41	379.7	201.76	380.87	203.52	381.05	205.89	381.34	207.21	381.59
210.88	381.57	215.88	381.29	222.27	381.44	227.45	381.65	234.42	382.18
243.08	383.82	246.21	384.66	259.25	386.07	269.35	387.28	275.59	387.7
285.54	388.77	290.54	389.14	309.39	390.13	326.94	392.02	343.62	393.56

Manning's n Values		num= 4					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
-126.44	.04	96.27	.11	170.52	.061	185.85	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	170.52	185.85		161	165		.1	.3

Ineffective Flow		num= 1			
Sta L	Sta R	Elev	Permanent		
-126.44	141.73	380.57	F		

CROSS SECTION

RIVER: JonesFalls  
REACH: NorthTrib RS: 1.5

INPUT

Description:

Station Elevation Data		num= 34							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-163.18	382	-79.65	380	0	378.54	24.34	377.77	28.79	377.66
50.42	377.35	56.67	377.22	63.87	377.18	83.78	377.13	87.93	377.1
93.02	377.3	173.53	377	190.32	376	192.2	375	192.61	374.95
193.31	375	196.7	375.36	197.57	376	202.72	376.5	205.72	376
207.16	375.99	208.98	376	253.35	377	298.09	378	318.34	378.91
324.06	379.17	337.47	380.31	343.27	381.12	344.96	381.35	366.36	384.61
368.48	384.91	369	384.98	383.2	387.03	383.97	387.09		

Manning's n Values num= 4  
 Sta n Val Sta n Val Sta n Val Sta n Val  
 -163.18 .04 87.93 .11 190.32 .061 197.57 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 190.32 197.57 177 177 178 .1 .3

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 -163.18 93.02 377.3 F  
 202.72 383.97 376.5 F

CROSS SECTION

RIVER: JonesFalls  
 REACH: NorthTrib RS: 1

INPUT  
 Description:

Station Elevation Data num= 49  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 0 377.19 9.31 376.97 20.2 376.77 38.13 376.37 64.1 375.62  
 67.58 375.54 68.65 375.51 69.6 375.48 80.2 375.96 82.87 376.07  
 88.76 375.93 109.84 375.47 114.28 375.36 115.61 375.31 116.94 375.1  
 117.31 375.08 126.46 374.35 132.72 374.94 133.66 375 136.74 374.64  
 142.13 373.98 142.43 373.94 145.18 371.77 150.64 371.49 157.08 372.66  
 164.47 372.41 166.52 374.31 167.94 374.26 174.2 373.96 181.31 373.85  
 186 373.85 202.36 373.61 216.59 373.28 230.26 372.97 242.48 373.09  
 247.37 373.22 248.87 372.8 251.43 371.99 253.56 372.34 255.84 373.07  
 265.98 373.04 290.65 373.13 295.66 373.17 329.68 373.74 330.6 373.76  
 331.69 373.79 347.53 374.31 366.53 375 378.27 376

Manning's n Values num= 4  
 Sta n Val Sta n Val Sta n Val Sta n Val  
 0 .04 82.87 .11 142.43 .061 166.52 .04

Bank Sta: Left Right Coeff Contr. Expan.  
 142.43 166.52 .1 .3

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 133.66 375 F  
 166.52 378.27 374.31 F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls2 RS: 12

INPUT

Description:

Station Elevation Data num= 103

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	374	1.22	373.96	4.15	373.82	26.04	373	36.15	372.56
51.56	371.88	65.11	371.41	73.25	371.22	81.55	370.87	83.4	370.7
88.93	370.821	90.25	370.85	92.66	370.89	95.97	370.77	102.43	370.19
118.54	368.99	120.56	369.04	122.37	368.98	146.73	369.09	158.79	369.43
163.63	369.46	176.25	368.59	181.71	368.87	183.4	367.95	185.69	366.89
187	367.04	189.54	367.06	192.33	368.07	193.34	369.49	193.83	369.57
198.9	369.37	201.67	369.27	204.46	369.194	213.37	368.95	218.39	369.01
224.38	368.94	245.01	369.05	248.71	369.04	296.44	368.97	297.19	368.99
299.85	369.01	309.14	369.21	315.68	369.2	319.75	368.77	324.39	368.08
326.04	368.23	332.54	369	333.05	368.97	334.39	368.94	338.21	369.09
339.42	369.14	344.77	369.29	349.74	369.11	351.53	369.32	351.87	369.35
360.43	369.28	361.26	369.29	361.73	369.25	366.32	369.22	368.84	368.93
372.99	368.45	373.36	368.29	373.62	368.3	375.29	367.14	376.94	366.6
382.92	366.66	388.03	366.6	388.36	367.11	391.76	369.04	392.17	369.06
392.5	369.05	407.66	369.25	422.52	369.81	423.87	369.85	424.61	369.83
439.34	370.01	442.81	370.08	446.06	370.42	446.97	370.39	447.45	370.36
456.54	370.6	471.21	371	479.31	371.22	482.91	371.36	504.11	371.96
506	372	523.07	372.32	528.83	372.38	551.58	372.83	557.55	372.96
559.22	372.68	560.65	372.4	563.04	372.63	564.87	372.95	568.17	372.75
590.88	373.35	597.57	373.67	602.9	373.96	603.9	374	644.67	375.98
645.22	376	647.92	376.11	675.78	377				

Manning's n Values num= 7

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	88.93	.11	204.46	.04	344.77	.11	366.32	.049
391.76	.11	482.91	.04						

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 366.32 391.76 96 135 134 .1 .3

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls2 RS: 11

INPUT

Description:

Station Elevation Data num= 118

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	376.02	4.31	375.79	33.78	374.32	55.84	372.87	62.67	372.53
80.24	371.6	95.64	371.03	109.79	370.46	118.51	370.48	119.33	370.49
129.15	369.59	140.64	368.92	143.62	369.04	147.81	368.76	156.27	368.25
156.93	368.23	161.13	368.13	181.64	367.92	182.81	367.92	183.26	367.92
184.73	367.92	189.65	367.8	197.25	367.73	198.13	367.69	199.17	367.69
210.84	368.04	222.99	368.01	228.59	366.76	229.44	366.08	229.6	366.98



233.2	366.81	237.35	365.78	238.79	367.61	239.29	367.85	240.74	367.96
243.75	368.04	247.95	368.17	260.77	368.17	267.73	367.99	280.86	367.7
308.45	368.28	309.09	368.26	309.73	368.24	310.1	368.25	311.29	368.01
312.33	368.41	315.68	368.28	323.3	368.21	326.54	368.16	329.14	367.72
330.2	367.66	332.9	367.28	334.25	366.94	334.86	366.8	336.13	366.04
337.49	365.16	338.6	364.8	339	364.54	343.72	364.25	344.22	364.26
348.48	364.33	348.91	364.88	349.61	365.13	350.06	366.27	350.54	367.16
354.31	367.33	358.64	367.61	361.41	367.53	367.59	368.07	370.59	367.97
376.15	368.35	378.39	368.49	389.51	368.81	394.41	369.11	399.9	369.19
414.53	369.57	418.07	369.66	419.91	369.71	430.19	369.96	431.46	369.83
433.15	369.95	434.34	370.31	434.92	370.19	444.81	370.6	452.77	370.93
458.82	370.83	468.43	371.01	471.16	370.96	471.86	370.93	484.07	371.17
486.8	370.912	488.31	370.77	491.69	371.17	492.06	371.25	495.15	371.13
495.71	371.11	503.75	371.33	516.64	371.88	554.72	372.22	557.21	372.27
579.33	373	591.39	373.39	593.5	373.43	595.44	373.45	622.53	373.58
623.11	373.48	627.69	373	628.51	373.13	635.49	373.73	637.48	373.62
647.87	373.39	652.88	373.57	672.43	374.07	693.99	375.01	702.34	375.46
720.42	376.52	724.89	376.7	729.24	377				

Manning's n Values num= 7

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	119.33	.11	247.95	.04	308.45	.11	326.54	.049
350.54	.11	486.8	.04						

Bank Sta: Left Right Coeff Contr. Expan.  
326.54 350.54 .1 .3

Ineffective Flow num= 1  
Sta L Sta R Elev Permanent  
484.07 729.24 371.17 F

CROSS SECTION

RIVER: JonesFalls  
REACH: SouthTrib RS: 8

INPUT

Description:

Station Elevation Data num= 9

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-32.1	392	0	391	110.6	390	123.66	388	133.45	388
212.45	390	272.51	394	382.34	396	450.2	396		

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
-32.1	.045	110.6	.057	212.45	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
110.6 212.45 75 80 75 .1 .3

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
-32.1	56.22	395	F
221.59	450.2	395	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 7

INPUT

Description:

Station Elevation		Data		num= 48					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	393.17	3.09	392.85	4.02	392.85	13.01	392.85	22.5	392.67
52.26	391.97	66.09	392.38	75.87	392.64	92.5	391.43	97.86	391.37
116.16	390.34	120.8	390.5	131.27	390.49	144.57	387.98	144.89	387.97
150.82	384.86	151.07	384.58	152.67	384.79	153.39	384.66	158.59	384.56
159.5	384.54	162.95	384.45	169.67	385.18	173.69	385.82	175.08	386.43
181.88	387.24	184.64	387.76	185.99	387.76	187.23	389.95	188.29	390.53
188.9	390.62	191.39	391.4	195.03	393.13	196.64	393.57	197	393.56
210.05	393.58	212.34	393.64	213.21	393.64	228.47	393.49	263.39	393.37
264.3	393.44	265.06	393.45	303.46	394.18	303.79	394.19	304.29	394.19
309.17	394.07	326.52	393.83	332.83	393.82				

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.045	144.89	.057	184.64	.045

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	144.89	184.64		74	68		.3	.5

Ineffective Flow		num= 2	
Sta L	Sta R	Elev	Permanent
0	151	395	F
186.01	332.83	395	F

BRIDGE

RIVER: JonesFalls  
 REACH: SouthTrib RS: 6.5

INPUT

Description: Cliffholme Rd Bridge  
 Distance from Upstream XS = 28  
 Deck/Roadway Width = 15.25  
 Weir Coefficient = 2.6  
 Upstream Deck/Roadway Coordinates

num= 6					
Sta	Hi	Cord	Lo	Cord	Sta
	Hi	Cord	Lo	Cord	Hi
	Hi	Cord	Lo	Cord	Hi

0	395	385.492	150.892	395	385.492	150.892	395	391.54
170.892	395	391.54	170.892	395	385.492	332.826	395	385.492

Upstream Bridge Cross Section Data

Station Elevation Data				num=	48				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	393.17	3.09	392.85	4.02	392.85	13.01	392.85	22.5	392.67
52.26	391.97	66.09	392.38	75.87	392.64	92.5	391.43	97.86	391.37
116.16	390.34	120.8	390.5	131.27	390.49	144.57	387.98	144.89	387.97
150.82	384.86	151.07	384.58	152.67	384.79	153.39	384.66	158.59	384.56
159.5	384.54	162.95	384.45	169.67	385.18	173.69	385.82	175.08	386.43
181.88	387.24	184.64	387.76	185.99	387.76	187.23	389.95	188.29	390.53
188.9	390.62	191.39	391.4	195.03	393.13	196.64	393.57	197	393.56
210.05	393.58	212.34	393.64	213.21	393.64	228.47	393.49	263.39	393.37
264.3	393.44	265.06	393.45	303.46	394.18	303.79	394.19	304.29	394.19
309.17	394.07	326.52	393.83	332.83	393.82				

Manning's n Values				num=	3
Sta	n Val	Sta	n Val	Sta	n Val
0	.045	144.89	.057	184.64	.045

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	144.89	184.64		.3	.5

Ineffective Flow				num=	2
Sta L	Sta R	Elev	Permanent		
0	151	395	F		
186.01	332.83	395	F		

Downstream Deck/Roadway Coordinates

num=				6					
Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	395	383.862	146.652	395	383.862	146.652	395	391.54	
166.652	395	391.54	166.652	395	383.862	416.81	395	383.862	

Downstream Bridge Cross Section Data

Station Elevation Data				num=	38				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	389.29	12.9	388.91	14.46	388.93	44.68	388.41	54.85	388.23
71.9	387.62	85.72	387.31	87.09	387.29	106.55	386.94	125.13	386.6
131.55	386.66	149.02	386.52	150.34	386.47	155.02	383.09	156.29	382.62
163.7	383.6	166.82	383.62	169.6	386.25	171.65	387.45	173.36	387.6
182.22	387.48	183.87	387.62	186.65	387.95	188.47	388.16	209.06	388.79
225.04	389.33	239.11	389.59	262.68	390.07	270.78	390.18	283.72	390.09
290.95	390.46	303.79	390.15	307.3	390.04	316.08	389.63	331.32	389.14
346.69	388.75	353.83	388.61	356.91	388.55				

Manning's n Values				num=	3
Sta	n Val	Sta	n Val	Sta	n Val
0	.045	150.34	.057	171.65	.045

Bank Sta: Left    Right    Coeff Contr.    Expan.  
                  150.34    169.6                   .3                   .5

Ineffective Flow                   num=                   2  
   Sta L    Sta R    Elev    Permanent  
           0    120.13    395            F  
          178.58    356.91    395            F

Upstream Embankment side slope                   =            0 horiz. to 1.0 vertical  
 Downstream Embankment side slope                =            0 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow =            .98  
 Elevation at which weir flow begins                =  
 Energy head used in spillway design                =  
 Spillway height used in design                     =  
 Weir crest shape                                        = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

  Energy

Selected Low Flow Methods = Highest Energy Answer

High Flow Method

  Energy Only

Additional Bridge Parameters

  Add Friction component to Momentum

  Do not add Weight component to Momentum

  Class B flow critical depth computations use critical depth  
     inside the bridge at the upstream end

  Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: JonesFalls

REACH: SouthTrib                                    RS: 6

INPUT

Description:

Station Elevation Data                   num=                   38

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	389.29	12.9	388.91	14.46	388.93	44.68	388.41	54.85	388.23
71.9	387.62	85.72	387.31	87.09	387.29	106.55	386.94	125.13	386.6
131.55	386.66	149.02	386.52	150.34	386.47	155.02	383.09	156.29	382.62
163.7	383.6	166.82	383.62	169.6	386.25	171.65	387.45	173.36	387.6
182.22	387.48	183.87	387.62	186.65	387.95	188.47	388.16	209.06	388.79
225.04	389.33	239.11	389.59	262.68	390.07	270.78	390.18	283.72	390.09
290.95	390.46	303.79	390.15	307.3	390.04	316.08	389.63	331.32	389.14
346.69	388.75	353.83	388.61	356.91	388.55				

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 0 .045 150.34 .057 171.65 .045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 150.34 169.6 97 93 87 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 120.13 395 F  
 178.58 356.91 395 F

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 5

INPUT

Description:

Station Elevation Data num= 45  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 0 388 3.06 387.92 9.34 387.75 17.98 387.42 51.53 386.88  
 67.76 386.48 105.47 385.13 115.67 384.63 123.63 384.31 124.82 384.17  
 131.31 383.44 134.03 383.8 140.92 384.25 150.43 384.2 153.77 384.2  
 166.03 384.47 171.83 384.47 186.66 384.57 187.42 384.53 193.85 384.19  
 199.44 383.88 201.35 382.72 203.47 381.13 204.93 381.14 208.79 381.18  
 209.72 382.88 210.93 382.77 211.8 383.45 212.75 383.8 215.14 384.14  
 215.93 384.2 217.27 384.23 223.76 385.53 227.75 386.34 228.37 386.36  
 228.99 386.35 230.36 386.36 261.9 386.578 268.07 386.62 274.9 386.49  
 278.6 386.54 305.44 386.38 307.55 386.36 318.43 386.42 334.83 386.57

Manning's n Values num= 5  
 Sta n Val Sta n Val Sta n Val Sta n Val Sta n Val  
 0 .04 124.82 .045 199.44 .057 212.75 .045 261.9 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 199.44 212.75 136 123 111 .1 .3

Ineffective Flow num= 3  
 Sta L Sta R Elev Permanent  
 0 41.24 395 F  
 88.83 186.66 384.57 F  
 223.38 334.83 395 F

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 4

INPUT

Description:

Station Elevation Data num= 42									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	383.78	12.91	383.29	30.89	382.09	34.85	381.82	39.18	381.67
43.74	381.22	45.02	380.76	47.21	381.24	50.03	380.75	53.38	380.06
55.42	379.27	56.61	378.96	65.52	378.89	66.22	378.88	67.2	379.47
72.44	381.28	76.57	381.31	77.54	381.32	82.76	381.32	83.98	380.94
84.29	381.33	86.12	381.15	87.65	381.17	88.59	380.99	89.07	381.19
92.68	382.23	100.29	382.46	108.21	382.34	115.81	382.26	122.45	382.32
131.35	382.48	156.19	382.25	156.9	382.28	161.48	382.53	203.7	384.87
211.14	384.68	216.23	384.62	224.74	385.61	233.39	386.07	254.31	387.36
255.26	387.63	259.77	388.01						

Manning's n Values num= 5									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	47.21	.057	108.21	.11	156.9	.04	233.39	.045

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	47.21	72.44		261 238	226		.1	.3

Ineffective Flow num= 4				
Sta L	Sta R	Elev	Permanent	
0	47.21	381.24	F	
82.76	100	381.32	F	
100.29	131.35	382.46	F	
131.35	180	382.48	F	

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 3

INPUT

Description:

Station Elevation Data num= 37									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	383	5.1	382.75	11.15	382.44	25.65	381.32	34.08	380.79
39.13	380.6	66.2	378.14	66.74	378.1	69.76	377.92	93.58	376.491
93.59	376.49	94.67	375.82	96.87	374.99	97.88	374.6	100.98	374.66
104.1	374.44	105.51	375.05	106.49	376.32	109.67	376.35	130.53	376.98
146.06	377.4	146.13	377.58	148.74	376.8	149.2	376.48	150.67	376.06
151.46	376.52	153.99	377.6	163.18	377.3	167.42	377.37	170.58	377.29
173.78	377.38	185.02	377.81	197.34	379.06	205.65	379.85	215.57	380.65
230.29	382.2	251.86	385.03						

Manning's n Values num= 6									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	93.59	.057	106.49	.11	146.13	.057	153.99	.11
170.58	.04								

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	93.59	106.49		243	242	242		.1	.3
Ineffective Flow	num=		1						
Sta L	Sta R	Elev	Permanent						
146.13	251.86	377.58	F						

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 2

INPUT

Description:

Station Elevation Data	num=		38							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	
0	373.88	13.75	373.69	28.9	373.47	38.3	373.33	40.7	373.3	
42.02	373.3	52.03	373.04	57.4	372.23	57.86	372.16	71.9	372.27	
73.81	372.32	77.18	372.49	88.35	372.18	91.24	372.09	100.68	372.64	
101.12	372.69	102.7	372.68	110.83	372.7	129.08	372.74	133.02	372.75	
135.69	373.03	137.23	373.22	139.2	372.85	140.27	372.74	144.66	371.55	
148.03	370.66	151.94	370.73	155.68	372.76	156.15	372.94	160.16	373	
162.36	372.98	183.93	373.38	193.64	373.48	214.42	373.7	218.91	373.92	
229.69	374.58	248.8	375.98	249.38	376.11					

Manning's n Values	num=		5							
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	
0	.04	57.4	.057	100.68	.11	140.27	.057	156.15	.04	

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	140.27	156.15		259	261	263		.1	.3

CROSS SECTION

RIVER: JonesFalls  
 REACH: SouthTrib RS: 1

INPUT

Description:

Station Elevation Data	num=		58							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	
0	371	2.59	370.94	21.12	370.55	26.1	370.48	38.41	370.31	
50.43	370.22	63.04	370.17	67.93	370.01	72.29	369.86	78.66	370.16	
81.88	370.19	83.39	370.14	84.39	369.69	87.05	369.74	98.85	369.68	
100.11	369.65	101.34	369.49	104.93	368.55	105.67	368.73	110.28	368.87	
114.94	369.12	115.51	369.18	122.22	369.18	130.5	369.18	134.46	369.25	
139.7	369.33	149.62	369.03	155.63	368.85	168.11	368.66	172.12	368.59	
172.53	368.6	175.97	367.06	176.33	366.98	178.85	366.4	182.45	366.28	
185.93	367.82	186.53	367.92	194.77	369.36	198.66	369.35	203.49	369.25	

218.82	368.95	220.29	369.17	221.88	369.17	230.05	369.24	231.8	369.3
237.96	369.74	253.02	370.39	262.15	370.75	269.25	371.07	274.09	371.51
274.57	371.55	277.83	371.46	283.88	371.39	288.6	371.56	291.9	371.9
299.14	372.5	308.4	373.07	312.19	373.31				

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	38.41	.11	172.12	.057	194.77	.11	262.15	.04

Bank Sta: Left Right Coeff Contr. Expan.  
 172.12 194.77 .1 .3

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	139.7	369.33	F
194.77	312.19	369.36	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: IntsTrib RS: 8

INPUT  
 Description:

Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	399	5.8	398.83	34.6	398	134.2	396	208.9	394
224.8	392	230.3	391.3	236.6	392	249.7	394	277.7	396
343.2	398	344.7	398.05	366.1	398.25	456.6	402		

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	5.8	.045	208.9	.046	249.7	.045	344.7	.013
366.1	.045								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 208.9 249.7 135 134 134 .1 .3

CROSS SECTION

RIVER: JonesFalls  
 REACH: IntsTrib RS: 7

INPUT  
 Description:

Station Elevation Data num= 15

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	395	12.5	394.9	23.7	394	59.4	392	189.7	390
198.5	388	199	387.9	203.3	386.2	208.3	386.2	208.4	387.5



214.5 388 239.4 390 260.9 392 277.4 394 292.3 396

Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	12.5	.045	198.5	.046	214.5	.045

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	199	208.4		151 152	154		.3	.5

Ineffective Flow	num=	2	
Sta L	Sta R	Elev	Permanent
0	182.82	395	F
281.48	292.3	395	F

CULVERT

RIVER: JonesFalls

REACH: IntsTrib RS: 6.5

INPUT

Description: Culvert under Greenspring Valley Rd.

Distance from Upstream XS = 50

Deck/Roadway Width = 44.8

Weir Coefficient = 2.6

Upstream Deck/Roadway Coordinates

num= 3

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
12.5		395			235		395			292.3		396		

Upstream Bridge Cross Section Data

Station Elevation Data num= 15

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	395	12.5	394.9	23.7	394	59.4	392	189.7	390
198.5	388	199	387.9	203.3	386.2	208.3	386.2	208.4	387.5
214.5	388	239.4	390	260.9	392	277.4	394	292.3	396

Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	12.5	.045	198.5	.046	214.5	.045

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	199	208.4		.3	.5

Ineffective Flow	num=	2	
Sta L	Sta R	Elev	Permanent
0	182.82	395	F
281.48	292.3	395	F

Downstream Deck/Roadway Coordinates

num= 2

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
-----	----	------	----	------	-----	----	------	----	------

0 395 318.418 395

Downstream Bridge Cross Section Data

Station Elevation Data num= 65

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	392.6	5.79	392.55	12.92	392.33	21.15	392	23.69	391.45
25.57	391	29.86	390	33.99	389	34.45	388.92	35.56	388.63
38.05	388.08	42.25	387	45.22	386.39	50.45	385.31	53.16	385
65.34	384.86	66.76	384.87	70.47	384.78	95.11	384.49	99.06	384.52
109.39	384.24	120.6	384.14	124.89	384.14	139.77	384.26	142.13	384.32
148.31	384.3	169.55	384.48	174.44	383.74	178.8	384	180.12	383
181.44	382	182.82	381	184.26	380	185.21	379.81	185.41	379
189.3	378.66	190.58	378.11	194.3	378.66	197.05	379	198.29	380
199.62	381	200.96	382	202.41	383	203.95	384	205.51	385
207.07	386	208.62	387	214.21	388	217.51	388.33	224.09	389
226.14	389.14	229.1	389.31	243.69	390	249.81	390.11	255.04	390.34
260.58	390.56	265.06	390.77	271.2	391	273.76	391.03	282.12	391.01
288.64	391.03	303.53	390.98	318.42	391.24	321.72	391.18	323.68	391

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	5.79	.04	184.26	.046	199.62	.04

Bank Sta: Left Right Coeff Contr. Expan.  
182.82 199.62 .3 .5

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	187.03	395	F
196.73	323.68	395	F

Upstream Embankment side slope = 3.5 horiz. to 1.0 vertical  
Downstream Embankment side slope = 5 horiz. to 1.0 vertical  
Maximum allowable submergence for weir flow = .98  
Elevation at which weir flow begins =  
Energy head used in spillway design =  
Spillway height used in design =  
Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span  
GrnsprngVal Circular 5  
FHWA Chart # 1 - Concrete Pipe Culvert  
FHWA Scale # 1 - Square edge entrance with headwall  
Solution Criteria = Highest U.S. EG  
Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef  
Exit Loss Coef

1 24 118.04 .013 .013 0 .5

Upstream Elevation = 384.53

Centerline Station = 205.8  
 Downstream Elevation = 378.84  
 Centerline Station = 191.8

CULVERT OUTPUT Profile #2 Yr Culv Group: GrnsprngVal

Q Culv Group (cfs)	66.30	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	7.54
Q Barrel (cfs)	66.30	Culv Vel DS (ft/s)	17.61
E.G. US. (ft)	388.15	Culv Inv El Up (ft)	384.53
W.S. US. (ft)	387.79	Culv Inv El Dn (ft)	378.84
E.G. DS (ft)	380.38	Culv Frctn Ls (ft)	2.82
W.S. DS (ft)	380.04	Culv Exit Loss (ft)	4.51
Delta EG (ft)	7.77	Culv Entr Loss (ft)	0.44
Delta WS (ft)	7.74	Q Weir (cfs)	
E.G. IC (ft)	387.70	Weir Sta Lft (ft)	
E.G. OC (ft)	388.15	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	386.83	Weir Max Depth (ft)	
Culv WS Outlet (ft)	380.07	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	1.15	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	2.30	Min El Weir Flow (ft)	395.01

Warning: During subcritical analysis, the water surface upstream of culvert went to critical depth.

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section

downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CULVERT OUTPUT Profile #10 Yr Culv Group: GrnsprngVal

Q Culv Group (cfs)	138.90	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	9.85
Q Barrel (cfs)	138.90	Culv Vel DS (ft/s)	20.22
E.G. US. (ft)	390.17	Culv Inv El Up (ft)	384.53
W.S. US. (ft)	390.11	Culv Inv El Dn (ft)	378.84
E.G. DS (ft)	381.45	Culv Frctn Ls (ft)	2.31
W.S. DS (ft)	380.82	Culv Exit Loss (ft)	5.65
Delta EG (ft)	8.71	Culv Entr Loss (ft)	0.75
Delta WS (ft)	9.30	Q Weir (cfs)	
E.G. IC (ft)	389.78	Weir Sta Lft (ft)	
E.G. OC (ft)	390.17	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	387.91	Weir Max Depth (ft)	
Culv WS Outlet (ft)	380.74	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	1.68	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	3.38	Min El Weir Flow (ft)	395.01

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CULVERT OUTPUT Profile #100 Yr Culv Group: GrnsprngVal

Q Culv Group (cfs)	261.20	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	13.30
Q Barrel (cfs)	261.20	Culv Vel DS (ft/s)	24.10
E.G. US. (ft)	394.80	Culv Inv El Up (ft)	384.53
W.S. US. (ft)	394.80	Culv Inv El Dn (ft)	378.84
E.G. DS (ft)	382.89	Culv Frctn Ls (ft)	2.86
W.S. DS (ft)	381.80	Culv Exit Loss (ft)	7.68
Delta EG (ft)	11.92	Culv Entr Loss (ft)	1.38
Delta WS (ft)	13.00	Q Weir (cfs)	
E.G. IC (ft)	394.80	Weir Sta Lft (ft)	
E.G. OC (ft)	393.63	Weir Sta Rgt (ft)	
Culvert Control	Inlet	Weir Submerg	
Culv WS Inlet (ft)	389.53	Weir Max Depth (ft)	
Culv WS Outlet (ft)	381.54	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	2.37	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	4.49	Min El Weir Flow (ft)	395.01

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CROSS SECTION

RIVER: JonesFalls

REACH: IntsTrib RS: 6

INPUT

Description:

Station Elevation Data	num=	65							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	392.6	5.79	392.55	12.92	392.33	21.15	392	23.69	391.45
25.57	391	29.86	390	33.99	389	34.45	388.92	35.56	388.63
38.05	388.08	42.25	387	45.22	386.39	50.45	385.31	53.16	385
65.34	384.86	66.76	384.87	70.47	384.78	95.11	384.49	99.06	384.52
109.39	384.24	120.6	384.14	124.89	384.14	139.77	384.26	142.13	384.32
148.31	384.3	169.55	384.48	174.44	383.74	178.8	384	180.12	383
181.44	382	182.82	381	184.26	380	185.21	379.81	185.41	379
189.3	378.66	190.58	378.11	194.3	378.66	197.05	379	198.29	380
199.62	381	200.96	382	202.41	383	203.95	384	205.51	385

207.07	386	208.62	387	214.21	388	217.51	388.33	224.09	389
226.14	389.14	229.1	389.31	243.69	390	249.81	390.11	255.04	390.34
260.58	390.56	265.06	390.77	271.2	391	273.76	391.03	282.12	391.01
288.64	391.03	303.53	390.98	318.42	391.24	321.72	391.18	323.68	391

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	5.79	.04	184.26	.046	199.62	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

182.82	199.62	117	118	119	.3	.5
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	187.03	395	F
196.73	323.68	395	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: IntsTrib RS: 5

INPUT  
 Description:

Station Elevation Data num= 47

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	388.8	4.2	388.19	7.99	387.54	12.01	387	12.85	386.76
15.48	386	19	385	19.18	384.96	22.57	384	25.33	383.24
26.16	383	28.01	382.52	31.37	382	35.9	381.61	41.67	380.87
58.33	380.99	62	381	88.64	381.1	103.8	381.18	120.46	381.31
121.57	381.37	131.02	381.3	134.12	381.28	141.59	380.95	142.99	381
149.28	380.4	150.83	380	152.06	379	153.29	378	154.64	377
158.97	376.5	160.23	377	163.53	378	166.26	379	168.47	380
169.86	381	171.23	382	215.58	382.48	222.36	382.69	226.15	382.79
229.3	383	236.72	383.49	244.96	384	255.39	384.62	262.2	385
270.55	385.47	288.89	386						

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	7.99	.04	149.28	.046	163.53	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

149.28	171.23	363	358	354	.1	.3
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	121.57	395	F
197.44	288.89	395	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: IntsTrib RS: 4

INPUT  
 Description:

Station Elevation Data		num= 51							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	379.87	5.44	379.82	14.76	379.66	25.44	379.2	31.41	379
31.63	378.9	35.44	378.05	36.19	378	39.89	377	40.99	376.64
43.6	376	47.3	375	48.49	374.69	51.03	374	55.44	373.86
65.36	373.06	69.47	373	82.23	372.87	85.44	372.55	90.78	372
91.86	371	93.44	370	95.92	369.9	96.98	370	99.1	370.4
101.04	371	103	372	106.76	373	110.85	374	111.94	374.14
116.32	374	125.44	373.84	132.83	373.73	141.16	373.69	155.44	373.84
161.77	374	163.8	374.08	188.37	374.9	191.34	375	200.3	375.33
205.44	375.57	212.93	375.95	214.25	376	215.44	376.1	217.16	376.21
225.44	376.85	227.48	377	240.72	378	250.9	378.77	253.95	379
259.72	379.5								

Manning's n Values		num= 4			
Sta	n Val	Sta	n Val	Sta	n Val
0	.013	31.63	.04	85.44	.046
				111.94	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	85.44	106.76		188	207		.1	.3
Ineffective Flow			num=	1				
Sta L	Sta R	Elev	Permanent					
111.94	259.72	374.14	F					

CROSS SECTION

RIVER: JonesFalls  
 REACH: IntsTrib RS: 3

INPUT  
 Description:

Station Elevation Data		num= 131							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	373.19	1.01	373.14	4.2	373	17.26	373	22.84	372.81
23.68	372.81	24.13	372.8	24.59	372.8	26.42	372.78	26.86	372.78
27.28	372.77	28.09	372.77	36.45	372.71	41.06	372.68	41.64	372.62
42.5	372.56	57.3	372.43	57.77	372.4	70.69	372.28	71.37	372.27
72.03	372.27	72.82	372.26	73.63	372.25	74.46	372.24	75.04	372.24
75.87	372.23	76.42	372.22	79.86	372.19	80.37	372.19	82.06	372.17
86.98	372.09	91.37	372	105.06	371.72	116.72	371.48	118.17	371.45
119.45	371.43	120.4	371.41	121.54	371.39	122.52	371.37	123.55	371.35
124.56	371.34	125.63	371.33	130.35	371.27	133.77	371.22	134.92	371.21
136.11	371.19	137.35	371.16	138.68	371.13	144.05	371	151.8	370.8

152.23	370.79	153.47	370.75	154.04	370.73	155.2	370.68	155.97	370.64
157	370.58	157.86	370.53	158.94	370.46	161.05	370.31	165.48	370
174.68	369	181.72	368	188.57	367	195.27	366	198.17	365
200.57	364	200.83	363.9	201.12	364	202.62	365	204.76	366
204.97	367	205.38	367.72	218.55	367.16	225.69	368	235.63	369
247.89	369.16	248.23	369.17	248.6	369.18	249.02	369.18	249.98	369.2
250.91	369.21	251.43	369.22	252.35	369.23	253.24	369.24	256.57	369.28
257.35	369.29	260.89	369.35	261.68	369.36	262.45	369.36	263.19	369.37
266.93	369.4	269.66	369.45	273.09	369.48	273.8	369.49	276.53	369.49
282.27	369.6	285.09	369.58	286.01	369.61	286.99	369.63	288.04	369.67
288.62	369.67	289.71	369.71	290.22	369.71	295.92	369.94	297.44	370
298.26	370.03	301.11	370.15	303.31	370.24	305.08	370.31	306.68	370.38
309.12	370.48	313.21	370.65	319.83	370.91	321.86	370.99	328.67	371.25
329.28	371.28	329.64	371.3	333.56	371.45	334.01	371.47	336.29	371.56
337.68	371.61	338.36	371.64	339.43	371.67	340.11	371.7	340.86	371.72
341.67	371.74	342.41	371.76	343.06	371.78	352.37	372	352.99	372.01
446.13	372.75								

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 0 .04 181.72 .046 205.38 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 181.72 205.38 112 116 119 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 182.06 371.13 F  
 204.02 446.13 371.13 F

CULVERT

RIVER: JonesFalls  
 REACH: IntsTrib RS: 2.5

INPUT

Description: Intersection Trib Culvert under Park Heights Ave.  
 Distance from Upstream XS = 50  
 Deck/Roadway Width = 45  
 Weir Coefficient = 2.6  
 Upstream Deck/Roadway Coordinates

num= 2  
 Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord  
 0 378.5 446.13 368.4

Upstream Bridge Cross Section Data

Station Elevation Data num= 131  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 0 373.19 1.01 373.14 4.2 373 17.26 373 22.84 372.81  
 23.68 372.81 24.13 372.8 24.59 372.8 26.42 372.78 26.86 372.78

27.28	372.77	28.09	372.77	36.45	372.71	41.06	372.68	41.64	372.62
42.5	372.56	57.3	372.43	57.77	372.4	70.69	372.28	71.37	372.27
72.03	372.27	72.82	372.26	73.63	372.25	74.46	372.24	75.04	372.24
75.87	372.23	76.42	372.22	79.86	372.19	80.37	372.19	82.06	372.17
86.98	372.09	91.37	372	105.06	371.72	116.72	371.48	118.17	371.45
119.45	371.43	120.4	371.41	121.54	371.39	122.52	371.37	123.55	371.35
124.56	371.34	125.63	371.33	130.35	371.27	133.77	371.22	134.92	371.21
136.11	371.19	137.35	371.16	138.68	371.13	144.05	371	151.8	370.8
152.23	370.79	153.47	370.75	154.04	370.73	155.2	370.68	155.97	370.64
157	370.58	157.86	370.53	158.94	370.46	161.05	370.31	165.48	370
174.68	369	181.72	368	188.57	367	195.27	366	198.17	365
200.57	364	200.83	363.9	201.12	364	202.62	365	204.76	366
204.97	367	205.38	367.72	218.55	367.16	225.69	368	235.63	369
247.89	369.16	248.23	369.17	248.6	369.18	249.02	369.18	249.98	369.2
250.91	369.21	251.43	369.22	252.35	369.23	253.24	369.24	256.57	369.28
257.35	369.29	260.89	369.35	261.68	369.36	262.45	369.36	263.19	369.37
266.93	369.4	269.66	369.45	273.09	369.48	273.8	369.49	276.53	369.49
282.27	369.6	285.09	369.58	286.01	369.61	286.99	369.63	288.04	369.67
288.62	369.67	289.71	369.71	290.22	369.71	295.92	369.94	297.44	370
298.26	370.03	301.11	370.15	303.31	370.24	305.08	370.31	306.68	370.38
309.12	370.48	313.21	370.65	319.83	370.91	321.86	370.99	328.67	371.25
329.28	371.28	329.64	371.3	333.56	371.45	334.01	371.47	336.29	371.56
337.68	371.61	338.36	371.64	339.43	371.67	340.11	371.7	340.86	371.72
341.67	371.74	342.41	371.76	343.06	371.78	352.37	372	352.99	372.01
446.13	372.75								

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.04	181.72	.046	205.38	.04

Bank Sta: Left Right Coeff Contr. Expan.

181.72	205.38		.3	.5
--------	--------	--	----	----

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	182.06	371.13	F
204.02	446.13	371.13	F

Downstream Deck/Roadway Coordinates num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	376.2		303.7	369.8	

Downstream Bridge Cross Section Data Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	372.5	15.7	372	88.3	370	149.6	368	155.3	366
157.93	363.47	162.93	363.47	163.3	365	185.2	366	203.4	366
241.82	365.5	300	369.8	306.44	369.8	321.8	370		

Manning's n Values num= 4



Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	155.3	.046	163.3	.045	300	.013

Bank Sta: Left Right Coeff Contr. Expan.  
 155.3 163.3 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 154.76 370 F  
 176.73 321.8 370 F

Upstream Embankment side slope = 2.4 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 3.5 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins =  
 Energy head used in spillway design =  
 Spillway height used in design =  
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span

ParkHgtsAve Circular 5  
 FHWA Chart # 1 - Concrete Pipe Culvert  
 FHWA Scale # 1 - Square edge entrance with headwall

Solution Criteria = Highest U.S. EG

Culvert Upstrm Dist	Length	Top n	Bottom n	Depth Blocked	Entrance Loss Coef
Exit Loss Coef					
	5.44	84.85	.024	.024	0
					.5

1

Upstream Elevation = 363.83  
 Centerline Station = 196.84  
 Downstream Elevation = 363.44  
 Centerline Station = 160.43

CULVERT OUTPUT Profile #2 Yr Culv Group: ParkHgtsAve

Q Culv Group (cfs)	66.30	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	5.71
Q Barrel (cfs)	66.30	Culv Vel DS (ft/s)	7.54
E.G. US. (ft)	367.45	Culv Inv El Up (ft)	363.83
W.S. US. (ft)	367.36	Culv Inv El Dn (ft)	363.44
E.G. DS (ft)	365.83	Culv Frctn Ls (ft)	0.58
W.S. DS (ft)	365.47	Culv Exit Loss (ft)	0.78
Delta EG (ft)	1.62	Culv Entr Loss (ft)	0.25
Delta WS (ft)	1.89	Q Weir (cfs)	
E.G. IC (ft)	367.11	Weir Sta Lft (ft)	
E.G. OC (ft)	367.45	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	366.69	Weir Max Depth (ft)	
Culv WS Outlet (ft)	365.74	Weir Avg Depth (ft)	

Culv Nml Depth (ft)	3.06	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	2.30	Min El Weir Flow (ft)	371.14

CULVERT OUTPUT Profile #10 Yr Culv Group: ParkHgtsAve

Q Culv Group (cfs)	138.90	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	7.53
Q Barrel (cfs)	138.90	Culv Vel DS (ft/s)	9.85
E.G. US. (ft)	369.60	Culv Inv El Up (ft)	363.83
W.S. US. (ft)	369.54	Culv Inv El Dn (ft)	363.44
E.G. DS (ft)	366.52	Culv Frctn Ls (ft)	0.84
W.S. DS (ft)	365.99	Culv Exit Loss (ft)	1.81
Delta EG (ft)	3.08	Culv Entr Loss (ft)	0.44
Delta WS (ft)	3.55	Q Weir (cfs)	
E.G. IC (ft)	369.19	Weir Sta Lft (ft)	
E.G. OC (ft)	369.60	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	368.28	Weir Max Depth (ft)	
Culv WS Outlet (ft)	366.82	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	5.00	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	3.38	Min El Weir Flow (ft)	371.14

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

CULVERT OUTPUT Profile #100 Yr Culv Group: ParkHgtsAve

Q Culv Group (cfs)	200.51	Culv Full Len (ft)	54.89
# Barrels	1	Culv Vel US (ft/s)	10.21
Q Barrel (cfs)	200.51	Culv Vel DS (ft/s)	11.79
E.G. US. (ft)	372.12	Culv Inv El Up (ft)	363.83
W.S. US. (ft)	372.12	Culv Inv El Dn (ft)	363.44
E.G. DS (ft)	367.34	Culv Frctn Ls (ft)	1.67
W.S. DS (ft)	366.55	Culv Exit Loss (ft)	2.30
Delta EG (ft)	4.78	Culv Entr Loss (ft)	0.81
Delta WS (ft)	5.57	Q Weir (cfs)	60.69
E.G. IC (ft)	371.99	Weir Sta Lft (ft)	283.46
E.G. OC (ft)	372.12	Weir Sta Rgt (ft)	362.14
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	368.83	Weir Max Depth (ft)	0.95
Culv WS Outlet (ft)	367.48	Weir Avg Depth (ft)	0.42
Culv Nml Depth (ft)	5.00	Weir Flow Area (sq ft)	32.89
Culv Crt Depth (ft)	4.04	Min El Weir Flow (ft)	371.14

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the

height of the culvert.

CROSS SECTION

RIVER: JonesFalls  
REACH: IntsTrib RS: 2

INPUT

Description:

Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	372.5	15.7	372	88.3	370	149.6	368	155.3	366
157.93	363.47	162.93	363.47	163.3	365	185.2	366	203.4	366
241.82	365.5	300	369.8	306.44	369.8	321.8	370		

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	155.3	.046	163.3	.045	300	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
155.3 163.3 209 187 167 .3 .5

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	154.76	370	F
176.73	321.8	370	F

CROSS SECTION

RIVER: JonesFalls  
REACH: IntsTrib RS: 1

INPUT

Description:

Station Elevation Data num= 10

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	363	43.1	364	84.7	365	156.97	364	186.15	363
207.34	362	239.78	362	255.12	360	281.17	361	371.98	368

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.04	239.78	.046	281.17	.045

Bank Sta: Left Right Coeff Contr. Expan.  
239.78 281.17 .1 .3

Ineffective Flow num= 1

Sta L	Sta R	Elev	Permanent
294.42	371.98	371.13	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 10

INPUT

Description:

Station Elevation Data		num= 81		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.62	15.21	378.48	18.9	378.29	54.55	375.97	84.9	373.51		
95.03	372.76	97.98	372.57	100.98	372.38	101.63	372.31	105.7	371.62		
121.61	368.69	125.9	367.75	128.19	367.57	135.5	366.9	135.83	366.93		
145.67	366.82	148.92	366.64	160.99	366.31	174.04	366.07	185.43	366.12		
185.86	366.16	187.73	366.09	188.24	366.14	192.16	366.05	210.65	365.69		
211.19	365.66	211.6	365.66	213.27	365.78	241.19	366.08	242.18	366.09		
242.58	366.03	251.79	365	252.54	365.03	252.93	365.03	256.01	365.03		
256.46	364.78	258.31	363.14	261.63	363	263.37	362.56	265.18	362.59		
269.48	362.6	272.25	362.49	274.82	362.3	275.51	362.37	277.22	362.96		
279.23	365.87	279.68	366.16	291.87	366.31	292.57	366.35	293.11	366.41		
295.24	366.59	301.87	366.89	304.81	366.93	306.32	366.91	310.8	364.95		
311.16	364.97	314.53	365.04	317.8	366.15	318.31	366.22	329.71	366.94		
340.47	367.09	350.11	367.24	352.08	367.26	358.44	367.59	370.66	367.49		
373.8	367.62	374.38	367.55	379.04	367.62	380.55	367.74	403.78	369.29		
405.1	369.24	410.83	369.71	411.47	369.77	412.41	369.72	414.56	369.43		
416.55	369.89	417.57	370.04	428.72	370.8	436.64	371.31	458.3	373.2		
465.75	373.67										

Manning's n Values		num= 5		Sta		n Val		Sta		n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	101.63	.11	242.18	.049	279.68	.11	410.83	.04		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	242.18	279.68		111	123	127		.1	.3

Ineffective Flow		num= 3		Sta		Elev		Permanent	
Sta L	Sta R	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	242.18	366.09							
304.81	411.47	366.93							
411.47	465.75	369.77							

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 9

INPUT

Description:

Station Elevation Data		num= 65		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev

0	378.73	29.38	378.1	54.01	376.94	57.98	376.64	72.34	375.35
85.47	374.11	87.63	373.86	111.6	367.7	111.93	367.59	112.92	367.5
113.84	367.42	128.96	365.92	150.29	365.05	152.46	364.98	156.15	364.99
157.96	364.99	159.53	364.69	160.71	363.94	162.79	363.94	167.82	364.02
169.49	364.99	170.4	364.98	184.75	364.99	190.7	365.08	197.46	364.99
198.74	365.21	202.54	365.3	207.91	365.41	212.88	365.38	216.32	362.94
217.87	362.22	220.29	361.52	220.93	361.42	225.82	361.22	226.61	361.13
232.59	361.1	233.44	361.08	235.13	362.2	235.52	362.52	238.23	364.64
240.04	364.63	249.84	364.46	250.47	364.15	253.05	362.66	254.28	362.65
256.34	362.51	256.96	363.11	259.38	365.52	259.72	365.52	261.16	365.48
272.11	365.39	276.55	365.13	278.03	364.96	279.12	365.14	282.37	365.51
284.75	365.51	287.68	365.59	304.33	366.14	311.77	366.55	317.18	366.77
335.62	367.38	344.2	367.68	348.71	367.87	363.04	368.51	486.63	380

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	85.47	.11	212.88	.049	238.23	.11	304.33	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

212.88	238.23	129	134	140	.1	.3
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	207.91	365.41	F
259.38	486.63	365.52	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 8

INPUT

Description:

Station Elevation Data num= 52

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.6	14.28	378.67	41.86	378.23	64.11	377.88	67.17	377.74
91.51	375.62	113.24	372.8	132.69	369.14	158.16	365.34	165.6	364.5
178.36	363.28	180.93	363.32	182.02	363.35	182.58	363.36	190.01	363.51
201.12	363.07	201.62	363.05	214.96	363.17	221.07	363.08	234.1	363.4
235.06	363.44	235.76	363.41	246.15	363.09	247.37	363.11	254.4	363.74
255.43	363.78	264.07	362.96	267.26	362.76	273.43	362.71	278.9	363.1
281.26	362.07	283.26	361.59	286	362.89	286.6	363.12	287.49	362.71
287.97	362.45	289.67	361.63	293.51	360.17	295.11	359.95	296.42	359.95
297.11	360.03	298.87	360.67	299.33	361.27	302.13	363.45	302.33	363.86
303.32	363.88	306.02	363.86	310.69	364.13	330.3	365.15	346.45	366.06
382.15	368	514.52	382						

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	190.01	.11	286.6	.049	302.33	.04

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
286.6	302.33	492	504	506		.1	.3
Ineffective Flow		num=	1				
Sta L	Sta R	Elev	Permanent				
0	255.43	363.78	F				

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 7

INPUT

Description:

Station Elevation Data	num=	65							
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev						Sta Elev Sta Elev			
0 373.47 7.46 373.33 14.51 373.03 49.91 371.62 52.51 371.51						150.37 366.3			
72.66 370.52 122.26 367.86 124.93 367.72 133.19 367.23 218.95 360.84						203.1 361.41 218.95 360.84			
156.71 365.66 176.99 363.76 187.11 362.79 267.65 359.63 278.06 359.74						302.02 356.34 302.9 356.34			
230.83 360.34 256.68 359.9 262.74 359.78 298.15 358.37 307.44 357.96 307.99 358.62						316.69 355.15 317.28 355.59			
293.91 359.47 297.27 359.29 298.15 358.37 306.05 356.37 307.44 357.96 307.99 358.62						326.58 355.11 329.05 356.18			
304.05 356.34 304.93 356.36 306.05 356.37 315.48 356.35 316.69 355.15 317.28 355.59						326.58 355.11 329.05 356.18			
311.71 358.49 314.57 358.04 315.48 356.35 326.1 355.05 326.58 355.11 329.05 356.18						362.89 359.61 363.42 359.63			
318.06 355.8 318.66 355.08 326.1 355.05 354.97 359.36 362.89 359.61 363.42 359.63						375.4 359.63 375.4 359.63			
334.34 358.17 334.98 358.68 354.97 359.36 388.33 359.43 396.85 359.43 415.9 359.62						427.89 359.57 451.79 359.47 473.59 359.23 479.06 359.15 483.51 359.28			
364.5 359.63 375.4 359.539 388.33 359.43 396.85 359.43 415.9 359.62						505.96 360.02 515 360.89 535.61 363.51 551.87 365.05 563.87 366.1			
427.89 359.57 451.79 359.47 473.59 359.23 479.06 359.15 483.51 359.28						581.17 367.68 600.29 369.68 613.52 370.94 637.42 373.62 670.42 377.7			
505.96 360.02 515 360.89 535.61 363.51 551.87 365.05 563.87 366.1									
581.17 367.68 600.29 369.68 613.52 370.94 637.42 373.62 670.42 377.7									

Manning's n Values	num=	5							
Sta n Val Sta n Val Sta n Val Sta n Val Sta n Val						Sta n Val Sta n Val			
0 .04 278.06 .11 311.71 .049 334.98 .11 375.4 .04									

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
311.71	334.98	469	475	444		.1	.3
Ineffective Flow		num=	1				
Sta L	Sta R	Elev	Permanent				
364.5	670.42	359.63	F				

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 6

INPUT

Description:

Station Elevation Data	num=	81							
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Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	369.21	.44	369.16	31.84	365.2	45.45	364.4	58.99	363.32
61.77	362.83	68.58	362.38	78.06	361.85	81.92	362	85.78	361.6
105.28	358.42	113.57	356.96	115.29	356.28	117.75	356.24	123.55	356.09
148.61	355.61	171.77	356.49	176.97	356.8	195.14	356.96	197.78	357.01
204.4	356.99	227.46	357.15	234.14	357.18	245.38	357.45	260.69	357.57
262.71	357.47	281.48	356.31	284.5	356.22	290.47	356.03	293.88	355.92
314.03	355.02	314.58	355	319.02	354.5	319.76	352.22	319.97	351.99
321.14	351.7	321.57	351.66	324.58	351.9	325.21	351.86	332.41	351.59
335.5	351.52	336.05	351.76	337.65	353.56	338.63	353.61	340.26	353.7
342.46	353.82	346.69	354.04	355.6	354.7	364.82	355.47	375.63	355.05
378.57	354.93	391.58	354.95	399.63	354.95	400.65	354.96	411.03	355.06
414.21	355.09	414.59	355.09	423.2	355.19	435.1	354.89	446.74	355.03
452.75	355.04	471.93	355.12	474.95	355.14	478	355.19	504.7	355.15
510.63	355.06	511.02	355.07	522.06	355.21	536.46	355.28	538.09	355.1
541.87	355.24	566.01	355.05	577.12	355.03	581.35	355	609.07	355.15
616.82	355.17	639.36	355.17	663.67	357.45	681.8	359	703.74	362
760.76	364								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	31.84	.11	319.02	.049	337.65	.11

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

319.02	337.65	122	82	93	.1	.3
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Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	260.69	357.57	F
672.5	760.76	364	F

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 5

INPUT

Description:

Station Elevation Data num= 84

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	365.64	31.93	365.36	41.59	364.7	43.55	364.49	54.4	363.66
60.83	363.32	70.36	364.02	79.54	363.88	88.3	363.87	103.77	363.76
109.4	363.27	114.85	360.87	128.72	358.03	133.94	357.55	150.5	356.46
151.42	356.08	151.54	356.16	152.32	356.51	154.71	354.98	155.46	354.25
157.15	354.16	157.16	354.51	158.44	354.8	158.8	354.97	162.81	356.05
165.6	356.17	169.47	356.65	174.56	357.27	202.46	357.54	221.96	357.13
229.01	356.37	265.13	356.89	271.6	356.8	297.25	356.93	299.31	356.95
303.73	356.98	306.05	356.94	312.06	356.82	332.55	355.51	335.48	353.21
337.33	351.42	337.88	351.18	343.34	351.45	348.32	351.22	353.53	351.27
356.18	351.29	357.52	351.24	358.81	351.55	359.78	352.63	361.37	353.26

363.42	354.33	368.32	354.09	369.82	353.88	389.94	355.89	394.61	355.73
431.33	355.47	436.16	355.42	438.49	355.36	443.36	355.33	471.08	355.08
484.01	355.07	500.1	354.99	509.6	355.08	522.13	354.7	536.9	354.7
543.3	354.16	544.67	354.21	549.67	354.19	563.81	353.97	565.69	354.09
581.39	355.16	605.31	354.89	612.72	354.98	644.44	355	673.96	355.99
675.6	355.99	677.42	355.98	692.34	356.8	708.36	357.74	719.93	358.96
758.69	362	794.3	362.65	868.21	364	953.07	366		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	103.77	.11	332.55	.049	363.42	.11	794.3	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

332.55	389.94	132	149	230	.3	.5
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Ineffective Flow num= 6

Sta L	Sta R	Elev	Permanent
0	70.36	364.02	F
70.36	151.7	363	F
151.7	163.09	357.54	F
163.09	308.17	363	F
385.08	527.24	363	F
576.78	953.07	363	F

CULVERT

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 4.5

INPUT

Description: Bridge over Jones Falls on Park Heights Ave.  
 Distance from Upstream XS = 53  
 Deck/Roadway Width = 46  
 Weir Coefficient = 2.6

Upstream Deck/Roadway Coordinates

num= 14

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0		367		27.063	367		59.34		367					
124.7		366		201.23	365		305.74		364					
360.23		363		360.23	363		382.84		363					
382.84		363		464.76	363		736.72		363					
839		363		883.95	364									

Upstream Bridge Cross Section Data

Station Elevation Data num= 84

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	365.64	31.93	365.36	41.59	364.7	43.55	364.49	54.4	363.66
60.83	363.32	70.36	364.02	79.54	363.88	88.3	363.87	103.77	363.76
109.4	363.27	114.85	360.87	128.72	358.03	133.94	357.55	150.5	356.46
151.42	356.08	151.54	356.16	152.32	356.51	154.71	354.98	155.46	354.25



157.15	354.16	157.16	354.51	158.44	354.8	158.8	354.97	162.81	356.05
165.6	356.17	169.47	356.65	174.56	357.27	202.46	357.54	221.96	357.13
229.01	356.37	265.13	356.89	271.6	356.8	297.25	356.93	299.31	356.95
303.73	356.98	306.05	356.94	312.06	356.82	332.55	355.51	335.48	353.21
337.33	351.42	337.88	351.18	343.34	351.45	348.32	351.22	353.53	351.27
356.18	351.29	357.52	351.24	358.81	351.55	359.78	352.63	361.37	353.26
363.42	354.33	368.32	354.09	369.82	353.88	389.94	355.89	394.61	355.73
431.33	355.47	436.16	355.42	438.49	355.36	443.36	355.33	471.08	355.08
484.01	355.07	500.1	354.99	509.6	355.08	522.13	354.7	536.9	354.7
543.3	354.16	544.67	354.21	549.67	354.19	563.81	353.97	565.69	354.09
581.39	355.16	605.31	354.89	612.72	354.98	644.44	355	673.96	355.99
675.6	355.99	677.42	355.98	692.34	356.8	708.36	357.74	719.93	358.96
758.69	362	794.3	362.65	868.21	364	953.07	366		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	103.77	.11	332.55	.049	363.42	.11	794.3	.045

Bank Sta: Left Right Coeff Contr. Expan.  
332.55 389.94 .3 .5

Ineffective Flow num= 6

Sta L	Sta R	Elev	Permanent
0	70.36	364.02	F
70.36	151.7	363	F
151.7	163.09	357.54	F
163.09	308.17	363	F
385.08	527.24	363	F
576.78	953.07	363	F

Downstream Deck/Roadway Coordinates num= 16

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
-67		370			-27		369			118.54		367		
183.84		366			260.22		365			364.65		364		
407.21		363.5			407.21		363.5			430.39		363.5		
430.39		363.5			523.52		363			793.58		363		
812.03		363			939.68		364			970		365		
1041.63		366												

Downstream Bridge Cross Section Data Station Elevation Data num= 27

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	362	77.33	360	130.4	358	170.95	356	257.66	355.34
259.53	355.32	263.78	353.23	265.78	353.23	271.18	355.23	301.91	355
363.95	355	388.75	354	400.53	352	422.68	352	429.75	354
491.27	354	495.18	352.35	497.18	352.35	510.8	354	606.35	354.93
620.43	355.07	716.33	356	847.36	358	928.57	360	1031.52	362
1037.3	364	1040.91	366						

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	77.33	.045	429.75	.049	716.33	.045

Bank Sta: Left Right Coeff Contr. Expan.  
 388.75 429.75 .3 .5

Ineffective Flow num= 4  
 Sta L Sta R Elev Permanent  
 0 259.53 363 F  
 271.18 391.95 363 F  
 449.03 456.73 363 F  
 516.56 1040.91 363 F

Upstream Embankment side slope = 4 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 4 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins = 363  
 Energy head used in spillway design =  
 Spillway height used in design =  
 Weir crest shape = Broad Crested

Number of Culverts = 3

Culvert Name	Shape	Rise	Span				
Culvert #4	Circular	2					
FHWA Chart # 1 - Concrete Pipe Culvert							
FHWA Scale # 1 - Square edge entrance with headwall							
Solution Criteria = Highest U.S. EG							
Culvert Upstrm Dist	Length	Top n	Bottom n	Depth Blocked	Entrance Loss Coef	Exit Loss Coef	
0	103.71	.013	.013	0		.5	

1  
 Upstream Elevation = 354.16  
 Centerline Station = 156.31  
 Downstream Elevation = 353.23  
 Centerline Station = 264.78

Culvert Name	Shape	Rise	Span				
PHAVE 24 N	Box	9.2	22.5				
FHWA Chart # 8 - flared wingwalls							
FHWA Scale # 1 - Wingwall flared 30 to 75 deg.							
Solution Criteria = Highest U.S. EG							
Culvert Upstrm Dist	Length	Top n	Bottom n	Depth Blocked	Entrance Loss Coef	Exit Loss Coef	
28.7	52.9	.013	.013	0		.4	

1  
 Upstream Elevation = 351.75  
 Centerline Station = 347.47  
 Downstream Elevation = 351.22  
 Centerline Station = 415.1

Culvert Name      Shape      Rise      Span  
 PHAve 24 S      Circular      2  
 FHWA Chart # 1 - Concrete Pipe Culvert  
 FHWA Scale # 1 - Square edge entrance with headwall  
 Solution Criteria = Highest U.S. EG  
 Culvert Upstrm Dist   Length      Top n      Bottom n      Depth Blocked      Entrance Loss Coef  
 Exit Loss Coef  
                  18.14   128.15      .013      .013      0      .5

1  
 Upstream      Elevation = 352.71  
                  Centerline Station = 548.25  
 Downstream      Elevation = 352.35  
                  Centerline Station = 496.18

CULVERT OUTPUT Profile #2 Yr Culv Group: Culvert #4

Q Culv Group (cfs)	16.04	Culv Full Len (ft)	43.39
# Barrels	1	Culv Vel US (ft/s)	6.01
Q Barrel (cfs)	16.04	Culv Vel DS (ft/s)	5.11
E.G. US. (ft)	356.58	Culv Inv El Up (ft)	354.16
W.S. US. (ft)	356.46	Culv Inv El Dn (ft)	353.23
E.G. DS (ft)	355.52	Culv Frctn Ls (ft)	0.50
W.S. DS (ft)	355.40	Culv Exit Loss (ft)	0.29
Delta EG (ft)	1.06	Culv Entr Loss (ft)	0.28
Delta WS (ft)	1.06	Q Weir (cfs)	
E.G. IC (ft)	356.53	Weir Sta Lft (ft)	
E.G. OC (ft)	356.59	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	355.74	Weir Max Depth (ft)	
Culv WS Outlet (ft)	355.23	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	1.29	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	1.44	Min El Weir Flow (ft)	363.01

CULVERT OUTPUT Profile #10 Yr Culv Group: Culvert #4

Q Culv Group (cfs)	30.98	Culv Full Len (ft)	103.71
# Barrels	1	Culv Vel US (ft/s)	9.86
Q Barrel (cfs)	30.98	Culv Vel DS (ft/s)	9.86
E.G. US. (ft)	360.38	Culv Inv El Up (ft)	354.16
W.S. US. (ft)	360.30	Culv Inv El Dn (ft)	353.23
E.G. DS (ft)	356.48	Culv Frctn Ls (ft)	1.95
W.S. DS (ft)	356.17	Culv Exit Loss (ft)	1.20
Delta EG (ft)	3.90	Culv Entr Loss (ft)	0.76
Delta WS (ft)	4.14	Q Weir (cfs)	
E.G. IC (ft)	359.36	Weir Sta Lft (ft)	
E.G. OC (ft)	360.38	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	356.16	Weir Max Depth (ft)	
Culv WS Outlet (ft)	355.23	Weir Avg Depth (ft)	

Culv Nml Depth (ft)	2.00	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	2.00	Min El Weir Flow (ft)	363.01

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

Note: Culvert critical depth exceeds the height of the culvert.

CULVERT OUTPUT Profile #100 Yr Culv Group: Culvert #4

Q Culv Group (cfs)	38.92	Culv Full Len (ft)	103.71
# Barrels	1	Culv Vel US (ft/s)	12.39
Q Barrel (cfs)	38.92	Culv Vel DS (ft/s)	12.39
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	354.16
W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	353.23
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	3.07
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	1.62
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.19
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	361.60	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.67	Weir Sta Rgt (ft)	850.78
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	356.16	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	355.23	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	2.00	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	2.00	Min El Weir Flow (ft)	363.01

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

Note: Culvert critical depth exceeds the height of the culvert.

CULVERT OUTPUT Profile #2 Yr Culv Group: PHAve 24 N

Q Culv Group (cfs)	623.59	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	8.45
Q Barrel (cfs)	623.59	Culv Vel DS (ft/s)	6.63
E.G. US. (ft)	356.58	Culv Inv El Up (ft)	351.75
W.S. US. (ft)	356.46	Culv Inv El Dn (ft)	351.22
E.G. DS (ft)	355.52	Culv Frctn Ls (ft)	0.06
W.S. DS (ft)	355.40	Culv Exit Loss (ft)	0.56
Delta EG (ft)	1.06	Culv Entr Loss (ft)	0.44
Delta WS (ft)	1.06	Q Weir (cfs)	
E.G. IC (ft)	356.26	Weir Sta Lft (ft)	
E.G. OC (ft)	356.58	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	355.03	Weir Max Depth (ft)	
Culv WS Outlet (ft)	355.40	Weir Avg Depth (ft)	

Culv Nml Depth (ft)	1.81	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	2.88	Min El Weir Flow (ft)	363.01

CULVERT OUTPUT Profile #10 Yr Culv Group: PHAve 24 N

Q Culv Group (cfs)	1460.10	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	12.78
Q Barrel (cfs)	1460.10	Culv Vel DS (ft/s)	15.66
E.G. US. (ft)	360.38	Culv Inv El Up (ft)	351.75
W.S. US. (ft)	360.30	Culv Inv El Dn (ft)	351.22
E.G. DS (ft)	356.48	Culv Frctn Ls (ft)	0.19
W.S. DS (ft)	356.17	Culv Exit Loss (ft)	2.69
Delta EG (ft)	3.90	Culv Entr Loss (ft)	1.02
Delta WS (ft)	4.14	Q Weir (cfs)	
E.G. IC (ft)	359.88	Weir Sta Lft (ft)	
E.G. OC (ft)	360.38	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	356.83	Weir Max Depth (ft)	
Culv WS Outlet (ft)	355.36	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	3.13	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	5.08	Min El Weir Flow (ft)	363.01

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CULVERT OUTPUT Profile #100 Yr Culv Group: PHAve 24 N

Q Culv Group (cfs)	2366.86	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	15.01
Q Barrel (cfs)	2366.86	Culv Vel DS (ft/s)	17.90
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	351.75
W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	351.22
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	0.18
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	4.29
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.42
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	363.67	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.66	Weir Sta Rgt (ft)	850.78
Culvert Control	Inlet	Weir Submerg	0.00
Culv WS Inlet (ft)	358.76	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	357.10	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	4.31	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	7.01	Min El Weir Flow (ft)	363.01

Warning: Since the culvert has supercritical flow, the program should be run in

mixed flow in order to check if the cross section  
downstream of the culvert has supercritical flow.  
Note: The flow in the culvert is entirely supercritical.

CULVERT OUTPUT Profile #2 Yr Culv Group: PHAve 24 S

Q Culv Group (cfs)	15.77	Culv Full Len (ft)	128.15
# Barrels	1	Culv Vel US (ft/s)	5.02
Q Barrel (cfs)	15.77	Culv Vel DS (ft/s)	5.02
E.G. US. (ft)	356.58	Culv Inv El Up (ft)	352.71
W.S. US. (ft)	356.46	Culv Inv El Dn (ft)	352.35
E.G. DS (ft)	355.52	Culv Frctn Ls (ft)	0.60
W.S. DS (ft)	355.40	Culv Exit Loss (ft)	0.27
Delta EG (ft)	1.06	Culv Entr Loss (ft)	0.20
Delta WS (ft)	1.06	Q Weir (cfs)	
E.G. IC (ft)	355.05	Weir Sta Lft (ft)	
E.G. OC (ft)	356.61	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	354.71	Weir Max Depth (ft)	
Culv WS Outlet (ft)	354.35	Weir Avg Depth (ft)	
Culv Nml Depth (ft)		Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	1.43	Min El Weir Flow (ft)	363.01

CULVERT OUTPUT Profile #10 Yr Culv Group: PHAve 24 S

Q Culv Group (cfs)	29.42	Culv Full Len (ft)	128.15
# Barrels	1	Culv Vel US (ft/s)	9.37
Q Barrel (cfs)	29.42	Culv Vel DS (ft/s)	9.37
E.G. US. (ft)	360.38	Culv Inv El Up (ft)	352.71
W.S. US. (ft)	360.30	Culv Inv El Dn (ft)	352.35
E.G. DS (ft)	356.48	Culv Frctn Ls (ft)	2.17
W.S. DS (ft)	356.17	Culv Exit Loss (ft)	1.05
Delta EG (ft)	3.90	Culv Entr Loss (ft)	0.68
Delta WS (ft)	4.14	Q Weir (cfs)	
E.G. IC (ft)	357.54	Weir Sta Lft (ft)	
E.G. OC (ft)	360.38	Weir Sta Rgt (ft)	
Culvert Control	Outlet	Weir Submerg	
Culv WS Inlet (ft)	354.71	Weir Max Depth (ft)	
Culv WS Outlet (ft)	354.35	Weir Avg Depth (ft)	
Culv Nml Depth (ft)		Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	1.85	Min El Weir Flow (ft)	363.01

CULVERT OUTPUT Profile #100 Yr Culv Group: PHAve 24 S

Q Culv Group (cfs)	36.97	Culv Full Len (ft)	128.15
# Barrels	1	Culv Vel US (ft/s)	11.77
Q Barrel (cfs)	36.97	Culv Vel DS (ft/s)	11.77
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	352.71

W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	352.35
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	3.43
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	1.39
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.08
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	359.56	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.67	Weir Sta Rgt (ft)	850.78
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	354.71	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	354.35	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	2.00	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	2.00	Min El Weir Flow (ft)	363.01

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

Note: Culvert critical depth exceeds the height of the culvert.

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 4

INPUT

Description:

Station Elevation Data	num=	27
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
0 362 77.33 360 130.4 358 170.95 356 257.66 355.34		
259.53 355.32 263.78 353.23 265.78 353.23 271.18 355.23 301.91 355		
363.95 355 388.75 354 400.53 352 422.68 352 429.75 354		
491.27 354 495.18 352.35 497.18 352.35 510.8 354 606.35 354.93		
620.43 355.07 716.33 356 847.36 358 928.57 360 1031.52 362		
1037.3 364 1040.91 366		

Manning's n Values	num=	4
Sta n Val Sta n Val Sta n Val Sta n Val		
0 .045 77.33 .045 429.75 .049 716.33 .045		

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
388.75 429.75 32 41 42 .3 .5

Ineffective Flow	num=	4
Sta L Sta R Elev Permanent		
0 259.53 363 F		
271.18 391.95 363 F		
449.03 456.73 363 F		
516.56 1040.91 363 F		

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 3.5

INPUT

Description:

Station Elevation Data num= 15									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	360	72.39	358	151.55	356	263.03	354	309.27	354
329.24	354	355.77	354	366.7	352	381.39	352	392.81	354
427.66	354	444.38	354	651.69	356	788.38	358	879.65	360

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.045	355.77	.049	392.81	.045

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	355.77	392.81		129	124		.1	.3

Ineffective Flow num= 3				
Sta L	Sta R	Elev	Permanent	
0	226.12	363	F	
262.79	354.06	363	F	
500.22	879.95	363	F	

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls3 RS: 3

INPUT

Description:

Station Elevation Data num= 64									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	359.99	.34	360	4.87	359.83	24.26	359	26.66	359
52.98	358.47	65.8	358.24	73.86	358	123.46	357	126.73	356.9
147.04	356.21	153.61	356.09	156.52	356	189.59	355	248.58	354.22
255.72	354	272.26	354	279.04	353.8	287.01	353.16	289.84	353
305.32	353	321.86	354	365.33	354	386.15	352	393.78	352
405.44	354	495.82	354	627.55	356	636.55	356.74	640.53	356.29
646.19	355.47	650.59	355	655.67	355.54	659.28	356.05	664.62	356.59
670.81	356.56	672.36	356.45	678.66	356.48	685.44	356.76	685.95	356.76
692.7	357	698.53	358.25	701.09	358.21	706.73	359.24	746.51	357.92
748.84	357.15	762.88	357.08	763.95	357.11	776.92	358	790.11	357.97
790.96	358	791.93	357.96	803.2	357.94	804.99	357.86	807.07	357.93
816.28	357.9	819.03	358	833.07	358	837.35	358.15	842.45	358.5
852.49	358.81	855.53	358.8	861.14	359	882.29	359		

Manning's n Values num= 3					
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Sta	n Val	Sta	n Val	Sta	n Val
0	.045	365.33	.049	405.44	.045

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	365.33	405.44		.1	.3
Ineffective Flow	num=		1		
	Sta L	Sta R	Elev	Permanent	
	623.22	882.29	363	F	

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls4 RS: 2

INPUT

Description:

Station Elevation Data	num=		67							
	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
	0	359.14	7.06	358.91	17.67	358.06	19.63	358	34.93	357
	38.91	356.87	47.9	356.15	50.24	356	70.75	355.83	79.5	355.13
	81.37	355.02	91.99	354.86	157.37	354	172.68	353	186.5	353
	187.53	352.98	189.15	352.89	198.15	352.17	203.29	352	356.33	352
	389.23	352	394.41	351.02	399.85	350.31	402.25	350.23	408.45	351.42
	410.46	351.84	417.55	352	498.36	352	506.01	351.89	509.38	352
	567.66	352	569.7	351.97	570.6	351.92	572.93	351.92	580.32	351.82
	585.91	352	631.82	352	633.4	352.05	636.96	352	647.13	352
	662.43	353	706.27	353	708.35	352.94	709.99	352.95	718.33	352.83
	723.65	353	738.96	353	739.56	352.98	740.92	352.87	754.26	352
	769.57	353	775.57	353	782.02	353.09	792.64	353.75	800.17	354
	803.25	354.3	813.87	355.84	815.48	356	819.64	356.14	824.48	356.21
	830.78	356.59	844.87	358.84	845.72	358.94	846.09	358.96	847.05	358.97
	856.33	358.83	857.88	358.89						

Manning's n Values	num=		5							
	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
	0	.04	356.33	.045	389.23	.049	417.55	.045	498.36	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.	
	389.23	417.55		261	248		.1	.3	
Ineffective Flow	num=		1						
	Sta L	Sta R	Elev	Permanent					
	662.43	857.88	353	F					

CROSS SECTION

RIVER: JonesFalls  
 REACH: JonesFalls4 RS: 1

INPUT

Description:

Station Elevation Data num= 34

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	356.15	2.31	356	17.6	356	23.22	355.82	26.04	355.67
36.66	355.63	45.22	355.2	48.19	355	50.53	354.92	57.9	354.55
77.84	354.03	78.78	354	109.37	353	124.67	353	132.26	352.75
139.96	352.73	149.5	352.38	153.5	352.06	155.26	352	201.14	351
262.33	350	338.83	350	349.44	349.18	351.13	348.88	352.83	349.24
363.44	350	434.53	350	636.49	350	664.25	350.27	667.02	350.05
668.56	350	717.1	350	793.76	352	821.31	354		

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	338.83	.049	363.44	.045	434.53	.04

Bank Sta: Left Right Coeff Contr. Expan.

338.83	363.44		.1	.3
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SUMMARY OF MANNING'S N VALUES

River: JonesFalls

Reach n6	River Sta. n7	n1	n2	n3	n4	n5
JonesFalls	15	.11	.049	.11	.04	
JonesFalls	14.5	.11	.049	.11	.04	
JonesFalls	14	.11	.049	.11	.04	
JonesFalls	13.6	.04	.11	.049	.11	.04
JonesFalls	13.5	.11	.04	.11	.049	.11
.04 JonesFalls	13	.04	.11	.049	.04	
NorthTrib	2	.04	.11	.061	.04	
NorthTrib	1.5	.04	.11	.061	.04	
NorthTrib	1	.04	.11	.061	.04	
JonesFalls2	12	.04	.11	.04	.11	.049
.11	.04					

JonesFalls2	11		.04	.11	.04	.11	.049
.11		.04					
SouthTrib	8		.045	.057	.045		
SouthTrib	7		.045	.057	.045		
SouthTrib	6.5	Bridge					
SouthTrib	6		.045	.057	.045		
SouthTrib	5		.04	.045	.057	.045	.04
SouthTrib	4		.04	.057	.11	.04	.045
SouthTrib	3		.04	.057	.11	.057	.11
.04							
SouthTrib	2		.04	.057	.11	.057	.04
SouthTrib	1		.04	.11	.057	.11	.04
IntsTrib	8		.013	.045	.046	.045	.013
.045							
IntsTrib	7		.013	.045	.046	.045	
IntsTrib	6.5	Culvert					
IntsTrib	6		.013	.04	.046	.04	
IntsTrib	5		.013	.04	.046	.04	
IntsTrib	4		.013	.04	.046	.04	
IntsTrib	3		.04	.046	.04		
IntsTrib	2.5	Culvert					
IntsTrib	2		.045	.046	.045	.013	
IntsTrib	1		.04	.046	.045		
JonesFalls3	10		.04	.11	.049	.11	.04
JonesFalls3	9		.04	.11	.049	.11	.04
JonesFalls3	8		.04	.11	.049	.04	
JonesFalls3	7		.04	.11	.049	.11	.04
JonesFalls3	6		.04	.11	.049	.11	

JonesFalls3	5	.04	.11	.049	.11	.045
JonesFalls3	4.5	Culvert				
JonesFalls3	4	.045	.045	.049	.045	
JonesFalls3	3.5	.045	.049	.045		
JonesFalls3	3	.045	.049	.045		
JonesFalls4	2	.04	.045	.049	.045	.04
JonesFalls4	1	.045	.049	.045	.04	

SUMMARY OF REACH LENGTHS

River: JonesFalls

Reach	River Sta.	Left	Channel	Right
JonesFalls	15	306	304	297
JonesFalls	14.5	422	398	305
JonesFalls	14	145	145	147
JonesFalls	13.6	134	112	114
JonesFalls	13.5	201	220	219
JonesFalls	13			
NorthTrib	2	161	165	177
NorthTrib	1.5	177	177	178
NorthTrib	1			
JonesFalls2	12	96	135	134
JonesFalls2	11			
SouthTrib	8	75	80	75
SouthTrib	7	74	68	66
SouthTrib	6.5	Bridge		
SouthTrib	6	97	93	87
SouthTrib	5	136	123	111
SouthTrib	4	261	238	226
SouthTrib	3	243	242	242
SouthTrib	2	259	261	263
SouthTrib	1			
IntsTrib	8	135	134	134
IntsTrib	7	151	152	154
IntsTrib	6.5	Culvert		
IntsTrib	6	117	118	119
IntsTrib	5	363	358	354

IntsTrib	4	188	207	219
IntsTrib	3	112	116	119
IntsTrib	2.5	Culvert		
IntsTrib	2	209	187	167
IntsTrib	1			
JonesFalls3	10	111	123	127
JonesFalls3	9	129	134	140
JonesFalls3	8	492	504	506
JonesFalls3	7	469	475	444
JonesFalls3	6	122	82	93
JonesFalls3	5	132	149	230
JonesFalls3	4.5	Culvert		
JonesFalls3	4	32	41	42
JonesFalls3	3.5	129	124	114
JonesFalls3	3			
JonesFalls4	2	261	248	241
JonesFalls4	1			

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS  
River: JonesFalls

Reach	River Sta.	Contr.	Expan.
JonesFalls	15	.1	.3
JonesFalls	14.5	.1	.3
JonesFalls	14	.1	.3
JonesFalls	13.6	.1	.3
JonesFalls	13.5	.1	.3
JonesFalls	13	.1	.3
NorthTrib	2	.1	.3
NorthTrib	1.5	.1	.3
NorthTrib	1	.1	.3
JonesFalls2	12	.1	.3
JonesFalls2	11	.1	.3
SouthTrib	8	.1	.3
SouthTrib	7	.3	.5
SouthTrib	6.5	Bridge	
SouthTrib	6	.3	.5
SouthTrib	5	.1	.3
SouthTrib	4	.1	.3
SouthTrib	3	.1	.3
SouthTrib	2	.1	.3
SouthTrib	1	.1	.3
IntsTrib	8	.1	.3
IntsTrib	7	.3	.5
IntsTrib	6.5	Culvert	

IntsTrib	6	.3	.5
IntsTrib	5	.1	.3
IntsTrib	4	.1	.3
IntsTrib	3	.3	.5
IntsTrib	2.5	Culvert	
IntsTrib	2	.3	.5
IntsTrib	1	.1	.3
JonesFalls3	10	.1	.3
JonesFalls3	9	.1	.3
JonesFalls3	8	.1	.3
JonesFalls3	7	.1	.3
JonesFalls3	6	.1	.3
JonesFalls3	5	.3	.5
JonesFalls3	4.5	Culvert	
JonesFalls3	4	.3	.5
JonesFalls3	3.5	.1	.3
JonesFalls3	3	.1	.3
JonesFalls4	2	.1	.3
JonesFalls4	1	.1	.3

HEC-RAS Plan: existing

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
JonesFalls	15	2 Yr	363.30	381.40	384.17	383.83	384.40	0.009027	4.96	139.48	136.40	0.56
JonesFalls	15	10 Yr	798.40	381.40	385.03	384.51	385.31	0.009219	6.14	278.02	359.68	0.60
JonesFalls	15	100 Yr	1593.40	381.40	385.74	385.28	385.98	0.009262	7.02	634.23	440.54	0.62
JonesFalls	14.5	2 Yr	363.30	378.97	381.30	380.96	381.43	0.010588	3.95	199.86	236.31	0.58
JonesFalls	14.5	10 Yr	798.40	378.97	381.88	381.42	382.08	0.012382	5.34	342.69	378.62	0.66
JonesFalls	14.5	100 Yr	1593.40	378.97	382.47	382.00	382.71	0.012862	6.46	632.33	456.61	0.70
JonesFalls	14	2 Yr	363.30	374.70	377.85	377.63	378.05	0.008322	4.76	191.16	206.05	0.54
JonesFalls	14	10 Yr	798.40	374.70	378.79	378.14	378.97	0.006506	5.25	446.32	352.77	0.51
JonesFalls	14	100 Yr	1593.40	374.70	379.78	378.73	379.95	0.005290	5.64	846.33	427.70	0.48
JonesFalls	13.6	2 Yr	363.30	372.77	375.90	375.90	376.42	0.015259	6.55	108.21	113.34	0.73
JonesFalls	13.6	10 Yr	798.40	372.77	376.70	376.70	377.43	0.018267	8.64	214.72	157.95	0.84
JonesFalls	13.6	100 Yr	1593.40	372.77	377.64	377.64	378.54	0.019755	10.63	390.80	216.73	0.91
JonesFalls	13.5	2 Yr	363.30	371.32	374.69	374.33	374.89	0.006220	4.20	130.61	149.40	0.48
JonesFalls	13.5	10 Yr	798.40	371.32	375.37	374.97	375.64	0.006581	5.09	240.71	174.79	0.51
JonesFalls	13.5	100 Yr	1593.40	371.32	376.19	375.61	376.58	0.007109	6.19	397.04	211.20	0.55
JonesFalls	13	2 Yr	363.30	369.84	372.43	372.43	372.77	0.017747	5.44	105.88	141.36	0.76
JonesFalls	13	10 Yr	798.40	369.84	372.95	372.95	373.41	0.019168	6.83	187.75	174.11	0.83
JonesFalls	13	100 Yr	1593.40	369.84	373.52	373.52	374.15	0.021486	8.47	293.48	205.02	0.91
NorthTrib	2	2 Yr	175.50	377.06	379.88	379.88	380.45	0.033869	6.20	30.02	143.82	0.81
NorthTrib	2	10 Yr	459.70	377.06	380.57	380.57	380.63	0.003398	2.39	231.31	206.33	0.27
NorthTrib	2	100 Yr	1025.90	377.06	380.71	380.58	380.96	0.012075	4.67	260.69	209.16	0.51
NorthTrib	1.5	2 Yr	175.50	374.95	377.21	376.91	377.33	0.009009	3.41	75.33	175.38	0.44
NorthTrib	1.5	10 Yr	459.70	374.95	377.51	377.51	377.80	0.018339	5.37	142.43	236.79	0.64
NorthTrib	1.5	100 Yr	1025.90	374.95	377.94	377.94	378.33	0.021317	6.52	254.11	276.51	0.71
NorthTrib	1	2 Yr	175.50	371.49	373.45	373.45	374.06	0.053530	6.29	27.89	125.49	1.00
NorthTrib	1	10 Yr	459.70	371.49	374.31	374.31	374.38	0.003793	2.28	212.41	208.10	0.28
NorthTrib	1	100 Yr	1025.90	371.49	374.32	374.32	374.68	0.018467	5.04	213.96	208.36	0.63
JonesFalls2	12	2 Yr	538.80	366.60	369.72	369.47	369.84	0.006233	3.90	252.74	311.28	0.46
JonesFalls2	12	10 Yr	1258.10	366.60	370.36		370.52	0.006074	4.57	467.36	345.23	0.48

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
JonesFalls2	12	100 Yr	2619.30	366.60	371.42		371.60	0.004511	4.87	869.68	419.98	0.44
JonesFalls2	11	2 Yr	538.80	364.25	368.48	368.48	368.85	0.010906	5.79	182.58	225.79	0.62
JonesFalls2	11	10 Yr	1258.10	364.25	369.41	369.04	369.72	0.008419	6.19	415.18	276.22	0.57
JonesFalls2	11	100 Yr	2619.30	364.25	370.81	369.78	371.06	0.005338	6.13	851.47	349.70	0.48
SouthTrib	8	2 Yr	151.70	388.00	389.03	389.03	389.33	0.056146	4.40	34.45	57.16	1.00
SouthTrib	8	10 Yr	322.20	388.00	389.46	389.46	389.86	0.050150	5.11	63.03	76.80	0.99
SouthTrib	8	100 Yr	622.10	388.00	390.30	389.94	390.59	0.017787	4.33	147.54	139.26	0.65
SouthTrib	7	2 Yr	151.70	384.45	386.87	385.94	387.02	0.006960	3.13	48.49	31.81	0.42
SouthTrib	7	10 Yr	322.20	384.45	388.06	386.70	388.28	0.005918	3.73	86.68	42.04	0.41
SouthTrib	7	100 Yr	622.10	384.45	389.68	387.69	389.98	0.004206	4.36	143.41	51.54	0.38
SouthTrib	6.5	Bridge										
SouthTrib	6	2 Yr	151.70	382.62	385.95	384.86	386.16	0.007976	3.72	40.81	18.22	0.44
SouthTrib	6	10 Yr	322.20	382.62	386.29	385.83	387.02	0.023720	6.83	47.16	19.08	0.76
SouthTrib	6	100 Yr	622.10	382.62	387.35	387.35	388.32	0.021794	8.24	84.28	87.40	0.78
SouthTrib	5	2 Yr	151.70	381.13	383.66	383.66	384.49	0.048871	7.28	20.82	16.21	1.00
SouthTrib	5	10 Yr	322.20	381.13	384.80	384.80	385.12	0.014255	5.45	84.68	107.81	0.59
SouthTrib	5	100 Yr	622.10	381.13	385.16	385.16	385.62	0.018490	6.76	125.97	117.38	0.68
SouthTrib	4	2 Yr	151.70	378.88	381.33	380.62	381.54	0.011132	3.69	42.99	46.96	0.51
SouthTrib	4	10 Yr	322.20	378.88	381.79	381.60	382.23	0.018107	5.56	66.03	55.43	0.68
SouthTrib	4	100 Yr	622.10	378.88	382.52	382.23	383.09	0.017731	6.74	122.40	136.79	0.71
SouthTrib	3	2 Yr	151.70	374.44	376.77	376.77	377.37	0.030554	6.34	28.02	37.79	0.84
SouthTrib	3	10 Yr	322.20	374.44	377.65	377.65	378.07	0.016898	6.16	90.10	106.62	0.66
SouthTrib	3	100 Yr	622.10	374.44	378.16	378.16	378.66	0.019363	7.41	148.71	122.45	0.73
SouthTrib	2	2 Yr	151.70	370.66	373.09		373.17	0.007219	2.92	76.50	116.32	0.41
SouthTrib	2	10 Yr	322.20	370.66	373.46		373.59	0.009721	3.90	127.06	161.84	0.49
SouthTrib	2	100 Yr	622.10	370.66	373.76	373.55	373.98	0.014779	5.28	182.66	206.71	0.62
SouthTrib	1	2 Yr	151.70	366.28	368.61	368.61	369.26	0.046333	6.44	23.58	20.20	1.00
SouthTrib	1	10 Yr	322.20	366.28	369.59	369.59	369.99	0.020928	5.73	100.09	135.21	0.72



HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
SouthTrib	1	100 Yr	622.10	366.28	370.51	370.10	370.76	0.010503	5.26	265.52	232.30	0.55
IntsTrib	8	2 Yr	66.30	391.30	392.61	392.61	392.96	0.036162	4.72	14.04	20.65	1.01
IntsTrib	8	10 Yr	138.90	391.30	393.07	393.07	393.55	0.032965	5.52	25.18	27.38	1.01
IntsTrib	8	100 Yr	261.20	391.30	394.73		394.85	0.002938	2.87	100.24	78.31	0.35
IntsTrib	7	2 Yr	66.30	386.20	387.79	387.79	388.31	0.030548	5.85	11.70	12.59	0.93
IntsTrib	7	10 Yr	138.90	386.20	390.11	388.54	390.16	0.001031	2.17	86.64	58.39	0.20
IntsTrib	7	100 Yr	261.20	386.20	394.80	389.18	394.80	0.000035	0.71	464.96	269.56	0.04
IntsTrib	6.5		Culvert									
IntsTrib	6	2 Yr	66.30	378.11	380.04	379.71	380.38	0.012859	4.67	14.21	14.15	0.68
IntsTrib	6	10 Yr	138.90	378.11	380.82	380.43	381.45	0.013753	6.40	21.70	16.29	0.75
IntsTrib	6	100 Yr	261.20	378.11	381.80	381.40	382.89	0.014429	8.36	31.25	18.98	0.82
IntsTrib	5	2 Yr	66.30	376.50	378.60	378.21	378.86	0.012403	4.11	16.12	12.60	0.64
IntsTrib	5	10 Yr	138.90	376.50	379.35	378.91	379.77	0.013043	5.19	26.76	15.42	0.69
IntsTrib	5	100 Yr	261.20	376.50	379.88	379.73	380.73	0.021230	7.40	35.29	17.21	0.91
IntsTrib	4	2 Yr	66.30	369.90	371.45	371.45	371.99	0.033163	5.88	11.27	10.56	1.00
IntsTrib	4	10 Yr	138.90	369.90	372.27	372.27	372.93	0.030337	6.54	21.22	15.81	1.00
IntsTrib	4	100 Yr	261.20	369.90	373.21	373.21	373.79	0.017346	6.23	46.02	44.12	0.80
IntsTrib	3	2 Yr	66.30	363.90	367.36	366.19	367.45	0.003409	2.42	27.39	25.28	0.34
IntsTrib	3	10 Yr	138.90	363.90	369.54	366.96	369.60	0.000701	1.87	74.25	109.70	0.18
IntsTrib	3	100 Yr	261.20	363.90	372.12	367.77	372.12	0.000041	0.63	609.36	280.76	0.05
IntsTrib	2.5		Culvert									
IntsTrib	2	2 Yr	66.30	363.47	365.47	365.19	365.83	0.016496	5.00	14.87	17.64	0.68
IntsTrib	2	10 Yr	138.90	363.47	365.99	365.99	366.52	0.020307	6.36	25.77	74.16	0.78
IntsTrib	2	100 Yr	261.20	363.47	366.55	366.55	367.34	0.022530	7.84	37.97	102.29	0.85
IntsTrib	1	2 Yr	66.30	360.00	360.99	360.99	361.24	0.039364	4.01	16.55	33.41	1.00
IntsTrib	1	10 Yr	138.90	360.00	361.32	361.32	361.69	0.031997	4.88	28.89	40.42	0.98
IntsTrib	1	100 Yr	261.20	360.00	361.74	361.74	362.23	0.027162	5.71	47.53	49.02	0.95

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
JonesFalls3	10	2 Yr	655.40	362.30	366.83	365.98	367.21	0.007734	5.24	192.07	177.03	0.54
JonesFalls3	10	10 Yr	1520.50	362.30	368.18	367.49	368.56	0.006275	6.07	523.94	263.18	0.52
JonesFalls3	10	100 Yr	3144.30	362.30	369.55	368.59	370.06	0.006841	7.63	904.97	293.45	0.57
JonesFalls3	9	2 Yr	655.40	361.08	365.57	364.46	366.14	0.009527	6.46	169.85	149.32	0.61
JonesFalls3	9	10 Yr	1520.50	361.08	366.76	366.61	367.53	0.011192	8.51	379.17	196.38	0.69
JonesFalls3	9	100 Yr	3144.30	361.08	368.31	367.88	369.06	0.009647	9.56	729.17	249.35	0.67
JonesFalls3	8	2 Yr	655.40	359.95	364.96	364.31	365.13	0.004978	4.75	284.48	165.00	0.43
JonesFalls3	8	10 Yr	1520.50	359.95	365.64	365.13	366.03	0.009748	7.44	403.98	182.89	0.62
JonesFalls3	8	100 Yr	3144.30	359.95	366.09	366.09	367.12	0.023572	12.34	489.32	193.98	0.99
JonesFalls3	7	2 Yr	655.40	355.05	359.07	359.07	360.22	0.025875	9.02	90.56	48.88	0.95
JonesFalls3	7	10 Yr	1520.50	355.05	360.52	360.52	361.01	0.010251	7.48	373.81	284.48	0.64
JonesFalls3	7	100 Yr	3144.30	355.05	363.76	361.16	363.84	0.000819	3.08	1436.20	361.17	0.20
JonesFalls3	6	2 Yr	655.40	351.52	356.67	355.73	356.74	0.001507	3.07	635.24	440.29	0.25
JonesFalls3	6	10 Yr	1520.50	351.52	360.41	356.40	360.42	0.000172	1.52	2738.24	599.02	0.09
JonesFalls3	6	100 Yr	3144.30	351.52	363.68	357.13	363.69	0.000141	1.71	4690.01	697.05	0.09
JonesFalls3	5	2 Yr	655.40	351.18	356.46	354.55	356.58	0.002091	2.99	294.36	392.98	0.28
JonesFalls3	5	10 Yr	1520.50	351.18	360.30	355.85	360.38	0.000771	2.61	835.35	619.46	0.17
JonesFalls3	5	100 Yr	3144.30	351.18	363.67	357.27	363.67	0.000123	1.25	5010.39	756.24	0.07
JonesFalls3	4.5		Culvert									
JonesFalls3	4	2 Yr	655.40	352.00	355.40	354.43	355.52	0.002226	3.25	259.09	404.85	0.33
JonesFalls3	4	10 Yr	1520.50	352.00	356.17	355.20	356.48	0.004366	5.29	357.64	559.74	0.48
JonesFalls3	4	100 Yr	3144.30	352.00	357.02	356.21	357.78	0.007893	8.14	467.53	633.07	0.66
JonesFalls3	3.5	2 Yr	655.40	352.00	355.29	354.74	355.41	0.003238	3.32	260.90	387.48	0.36
JonesFalls3	3.5	10 Yr	1520.50	352.00	356.01	355.30	356.26	0.004888	4.77	392.29	501.41	0.46
JonesFalls3	3.5	100 Yr	3144.30	352.00	356.82	356.07	357.37	0.007438	6.78	540.00	588.61	0.58
JonesFalls3	3	2 Yr	655.40	352.00	354.44	353.99	354.68	0.013936	4.93	194.79	292.70	0.68
JonesFalls3	3	10 Yr	1520.50	352.00	354.92	354.84	355.27	0.015908	6.26	351.53	360.59	0.76
JonesFalls3	3	100 Yr	3144.30	352.00	355.45	355.39	356.01	0.018554	7.85	558.20	424.90	0.85

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
JonesFalls4	2	2 Yr	721.70	350.23	352.56	352.43	352.67	0.009899	3.67	278.63	479.52	0.56
JonesFalls4	2	10 Yr	1659.40	350.23	352.97	352.72	353.17	0.009790	4.35	470.41	518.92	0.58
JonesFalls4	2	100 Yr	3405.50	350.23	353.51	353.19	353.79	0.009255	5.06	817.03	623.96	0.59
JonesFalls4	1	2 Yr	721.70	348.88	350.69	350.42	350.76	0.006131	2.61	342.99	523.08	0.43
JonesFalls4	1	10 Yr	1659.40	348.88	351.13	350.72	351.26	0.006135	3.24	583.50	565.06	0.45
JonesFalls4	1	100 Yr	3405.50	348.88	351.71	351.13	351.93	0.006131	3.99	929.09	614.41	0.48

HEC-RAS Plan: existing

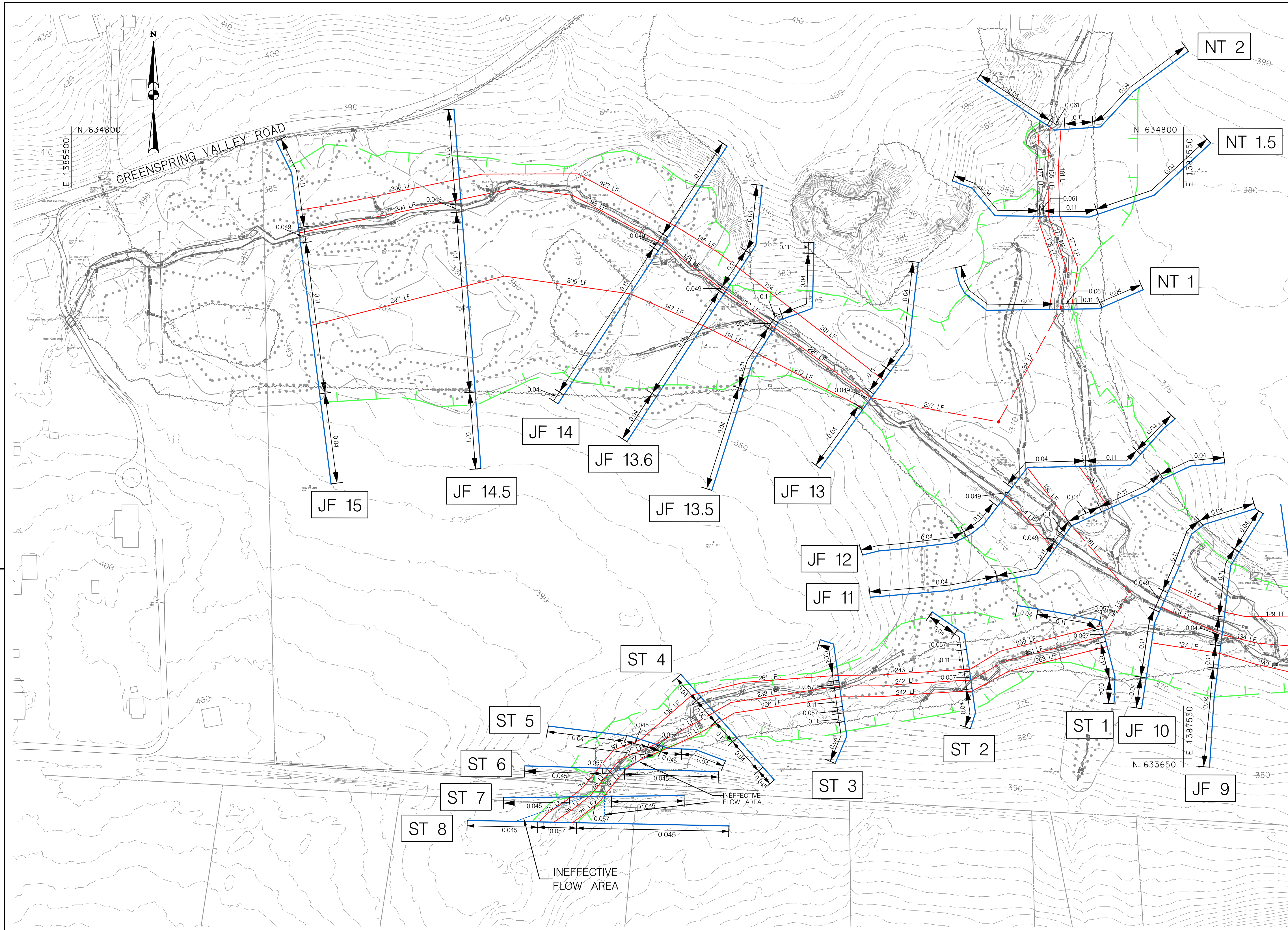
Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Vel Head (ft)	Frctn Loss (ft)	C & E Loss (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Top Width (ft)
JonesFalls	15	2 Yr	384.40	384.17	0.23	2.94	0.03	67.38	172.30	123.62	136.40
JonesFalls	15	10 Yr	385.31	385.03	0.28	3.20	0.02	218.07	289.15	291.18	359.68
JonesFalls	15	100 Yr	385.98	385.74	0.24	3.26	0.00	431.01	401.85	760.54	440.54
JonesFalls	14.5	2 Yr	381.43	381.30	0.13	3.37	0.01	24.48	166.36	172.47	236.31
JonesFalls	14.5	10 Yr	382.08	381.88	0.20	3.11	0.01	100.70	314.50	383.21	378.62
JonesFalls	14.5	100 Yr	382.71	382.47	0.24	2.74	0.02	222.61	491.18	879.61	456.61
JonesFalls	14	2 Yr	378.05	377.85	0.21	1.60	0.03	22.48	203.15	137.66	206.05
JonesFalls	14	10 Yr	378.97	378.79	0.18	1.49	0.05	91.58	311.83	394.99	352.77
JonesFalls	14	100 Yr	379.95	379.78	0.16	1.34	0.07	224.27	434.52	934.61	427.70
JonesFalls	13.6	2 Yr	376.42	375.90	0.52	1.08	0.10	1.07	278.06	84.18	113.34
JonesFalls	13.6	10 Yr	377.43	376.70	0.73	1.21	0.14	6.39	484.87	307.14	157.95
JonesFalls	13.6	100 Yr	378.54	377.64	0.90	1.32	0.15	22.11	767.61	803.68	216.73
JonesFalls	13.5	2 Yr	374.89	374.69	0.20	2.10	0.01	124.60	225.49	13.21	149.40
JonesFalls	13.5	10 Yr	375.64	375.37	0.26	2.21	0.02	389.04	350.79	58.56	174.79
JonesFalls	13.5	100 Yr	376.58	376.19	0.39	2.40	0.03	897.17	539.06	157.17	211.20
JonesFalls	13	2 Yr	372.77	372.43	0.34	2.11	0.07	103.10	259.37	0.83	141.36
JonesFalls	13	10 Yr	373.41	372.95	0.46	2.09	0.09	359.66	433.71	5.03	174.11
JonesFalls	13	100 Yr	374.15	373.52	0.64	1.69	0.14	887.66	680.25	25.49	205.02
NorthTrib	2	2 Yr	380.45	379.88	0.57	2.66	0.13		166.78	8.72	143.82
NorthTrib	2	10 Yr	380.63	380.57	0.06	1.11	0.02	346.60	89.25	23.85	206.33
NorthTrib	2	100 Yr	380.96	380.71	0.25	2.62	0.01	783.94	184.82	57.13	209.16
NorthTrib	1.5	2 Yr	377.33	377.21	0.13	3.21	0.05	8.74	47.36	119.40	175.38
NorthTrib	1.5	10 Yr	377.80	377.51	0.29	1.27	0.06	79.70	86.38	293.62	236.79
NorthTrib	1.5	100 Yr	378.33	377.94	0.39	3.52	0.01	314.92	125.29	585.70	276.51
NorthTrib	1	2 Yr	374.06	373.45	0.62	2.12	0.15		175.50		125.49
NorthTrib	1	10 Yr	374.38	374.31	0.07	1.27	0.01	0.15	109.55	350.00	208.10
NorthTrib	1	100 Yr	374.68	374.32	0.36	1.47	0.05	0.34	243.23	782.33	208.36
JonesFalls2	12	2 Yr	369.84	369.72	0.12	0.96	0.03	315.15	216.44	7.21	311.28
JonesFalls2	12	10 Yr	370.52	370.36	0.16	0.79	0.01	893.05	329.19	35.86	345.23
JonesFalls2	12	100 Yr	371.60	371.42	0.18	0.53	0.01	2008.08	480.56	130.66	419.98
JonesFalls2	11	2 Yr	368.85	368.48	0.37	1.44	0.00	140.43	377.44	20.93	225.79
JonesFalls2	11	10 Yr	369.72	369.41	0.30	1.15	0.01	644.04	542.03	72.03	276.22
JonesFalls2	11	100 Yr	371.06	370.81	0.25	0.98	0.03	1632.65	741.99	244.66	349.70
SouthTrib	8	2 Yr	389.33	389.03	0.30	1.22	0.04		151.70		57.16
SouthTrib	8	10 Yr	389.86	389.46	0.41	1.05	0.06		322.20		76.80
SouthTrib	8	100 Yr	390.59	390.30	0.29	0.61	0.00	6.07	615.21	0.82	139.26
SouthTrib	7	2 Yr	387.02	386.87	0.15	0.32	0.07		151.70		31.81
SouthTrib	7	10 Yr	388.28	388.06	0.22	0.29	0.13		321.73	0.47	42.04
SouthTrib	7	100 Yr	389.98	389.68	0.29	0.25	0.28		613.46	8.64	51.54
SouthTrib	6.5		Bridge								
SouthTrib	6	2 Yr	386.16	385.95	0.21	1.51	0.18		151.70		18.22
SouthTrib	6	10 Yr	387.02	386.29	0.73	1.69	0.20		322.20	0.00	19.08
SouthTrib	6	100 Yr	388.32	387.35	0.97	1.89	0.26	63.87	555.83	2.40	87.40
SouthTrib	5	2 Yr	384.49	383.66	0.82	2.51	0.18		151.70		16.21
SouthTrib	5	10 Yr	385.12	384.80	0.32	2.00	0.01	116.12	195.44	10.64	107.81
SouthTrib	5	100 Yr	385.62	385.16	0.46	2.28	0.01	320.18	275.41	26.50	117.38
SouthTrib	4	2 Yr	381.54	381.33	0.21	4.12	0.04	1.96	149.05	0.69	46.96
SouthTrib	4	10 Yr	382.23	381.79	0.44	4.16	0.01	12.08	288.97	21.15	55.43
SouthTrib	4	100 Yr	383.09	382.52	0.58	4.42	0.02	69.78	473.79	78.53	136.79
SouthTrib	3	2 Yr	377.37	376.77	0.60	3.17	0.16	1.11	146.58	4.00	37.79
SouthTrib	3	10 Yr	378.07	377.65	0.42	3.05	0.09	37.68	212.62	71.91	106.62
SouthTrib	3	100 Yr	378.66	378.16	0.51	4.08	0.08	106.58	303.99	211.53	122.45
SouthTrib	2	2 Yr	373.17	373.09	0.08	3.87	0.06	77.04	74.04	0.61	116.32
SouthTrib	2	10 Yr	373.59	373.46	0.13	3.58	0.03	186.67	121.58	13.94	161.84
SouthTrib	2	100 Yr	373.98	373.76	0.23	3.22	0.00	373.11	189.85	59.14	206.71

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Vel Head (ft)	Frctn Loss (ft)	C & E Loss (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Top Width (ft)
SouthTrib	1	2 Yr	369.26	368.61	0.64	0.91	0.08	0.00	151.70		20.20
SouthTrib	1	10 Yr	369.99	369.59	0.40	0.69	0.01	54.44	252.08	15.68	135.21
SouthTrib	1	100 Yr	370.76	370.51	0.25	0.68	0.03	193.49	341.86	86.75	232.30
IntsTrib	8	2 Yr	392.96	392.61	0.35	4.45	0.02		66.30		20.65
IntsTrib	8	10 Yr	393.55	393.07	0.47	0.40	0.13		138.90		27.38
IntsTrib	8	100 Yr	394.85	394.73	0.12	0.02	0.04	9.11	248.68	3.41	78.31
IntsTrib	7	2 Yr	388.31	387.79	0.53				65.53	0.77	12.59
IntsTrib	7	10 Yr	390.16	390.11	0.05			9.94	71.78	57.18	58.39
IntsTrib	7	100 Yr	394.80	394.80	0.01			52.13	54.58	154.49	269.56
IntsTrib	6.5		Culvert								
IntsTrib	6	2 Yr	380.38	380.04	0.34	1.49	0.04		66.30		14.15
IntsTrib	6	10 Yr	381.45	380.82	0.64	1.58	0.11		138.90		16.29
IntsTrib	6	100 Yr	382.89	381.80	1.09	2.05	0.12		261.20		18.98
IntsTrib	5	2 Yr	378.86	378.60	0.26	6.84	0.03		66.30		12.60
IntsTrib	5	10 Yr	379.77	379.35	0.42	6.81	0.02		138.90		15.42
IntsTrib	5	100 Yr	380.73	379.88	0.85	6.85	0.08		261.20		17.21
IntsTrib	4	2 Yr	371.99	371.45	0.54	1.62	0.13		66.30		10.56
IntsTrib	4	10 Yr	372.93	372.27	0.67	0.44	0.18		138.90		15.81
IntsTrib	4	100 Yr	373.79	373.21	0.58	0.03	0.17	12.34	248.78	0.08	44.12
IntsTrib	3	2 Yr	367.45	367.36	0.09				66.30		25.28
IntsTrib	3	10 Yr	369.60	369.54	0.05				138.90		109.70
IntsTrib	3	100 Yr	372.12	372.12	0.00			30.59	88.52	142.10	280.76
IntsTrib	2.5		Culvert								
IntsTrib	2	2 Yr	365.83	365.47	0.37	4.53	0.06		62.50	3.80	17.64
IntsTrib	2	10 Yr	366.52	365.99	0.53				105.27	33.63	74.16
IntsTrib	2	100 Yr	367.34	366.55	0.79	4.52	0.15	0.69	164.90	95.60	102.29
IntsTrib	1	2 Yr	361.24	360.99	0.25	5.47	0.04		66.30		33.41
IntsTrib	1	10 Yr	361.69	361.32	0.37	5.32	0.05		137.70	1.20	40.42
IntsTrib	1	100 Yr	362.23	361.74	0.49	4.98	0.07		251.21	9.99	49.02
JonesFalls3	10	2 Yr	367.21	366.83	0.38	1.05	0.02	70.99	578.13	6.28	177.03
JonesFalls3	10	10 Yr	368.56	368.18	0.38	0.99	0.04	368.64	977.45	174.41	263.18
JonesFalls3	10	100 Yr	370.06	369.55	0.50	0.97	0.02	935.21	1621.64	587.45	293.45
JonesFalls3	9	2 Yr	366.14	365.57	0.57	0.89	0.12	38.79	575.37	41.24	149.32
JonesFalls3	9	10 Yr	367.53	366.76	0.77	1.39	0.12	278.15	1014.16	228.19	196.38
JonesFalls3	9	100 Yr	369.06	368.31	0.75	1.91	0.03	799.45	1514.84	830.01	249.35
JonesFalls3	8	2 Yr	365.13	364.96	0.17	4.81	0.10	347.60	279.11	28.69	165.00
JonesFalls3	8	10 Yr	366.03	365.64	0.39	5.00	0.01	872.91	517.24	130.35	182.89
JonesFalls3	8	100 Yr	367.12	366.09	1.03	1.16	0.28	1844.41	946.23	353.66	193.98
JonesFalls3	7	2 Yr	360.22	359.07	1.15	1.82	0.33	62.54	591.25	1.60	48.88
JonesFalls3	7	10 Yr	361.01	360.52	0.49	0.25	0.15	190.17	743.13	587.20	284.48
JonesFalls3	7	100 Yr	363.84	363.76	0.08	0.13	0.02	812.70	538.61	1792.99	361.17
JonesFalls3	6	2 Yr	356.74	356.67	0.07	0.15	0.01	21.20	274.73	359.47	440.29
JonesFalls3	6	10 Yr	360.42	360.41	0.01	0.03	0.01	354.02	241.67	924.81	599.02
JonesFalls3	6	100 Yr	363.69	363.68	0.01	0.01	0.00	879.95	375.73	1888.62	697.05
JonesFalls3	5	2 Yr	356.58	356.46	0.12			2.66	546.16	106.58	392.98
JonesFalls3	5	10 Yr	360.38	360.30	0.08			151.88	1005.02	363.59	619.46
JonesFalls3	5	100 Yr	363.67	363.67	0.01			756.86	751.46	1635.98	756.24
JonesFalls3	4.5		Culvert								
JonesFalls3	4	2 Yr	355.52	355.40	0.12	0.11	0.00	27.73	374.81	252.86	404.85
JonesFalls3	4	10 Yr	356.48	356.17	0.32	0.19	0.03	83.05	762.79	674.66	559.74
JonesFalls3	4	100 Yr	357.78	357.02	0.76	0.31	0.11	197.63	1436.59	1510.08	633.07
JonesFalls3	3.5	2 Yr	355.41	355.29	0.12	0.72	0.01	69.10	330.39	255.91	387.48
JonesFalls3	3.5	10 Yr	356.26	356.01	0.25	0.99	0.01	213.16	602.39	704.96	501.41
JonesFalls3	3.5	100 Yr	357.37	356.82	0.55	1.35	0.00	503.72	1059.23	1581.34	588.61

HEC-RAS Plan: existing (Continued)

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Vel Head (ft)	Frctn Loss (ft)	C & E Loss (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Top Width (ft)
JonesFalls3	3	2 Yr	354.68	354.44	0.24	1.97	0.04	237.93	322.45	95.03	292.70
JonesFalls3	3	10 Yr	355.27	354.92	0.35	2.07	0.05	615.05	529.72	375.73	360.59
JonesFalls3	3	100 Yr	356.01	355.45	0.56	2.13	0.08	1367.98	831.21	945.11	424.90
JonesFalls4	2	2 Yr	352.67	352.56	0.11	1.90	0.01	260.40	140.82	320.48	479.52
JonesFalls4	2	10 Yr	353.17	352.97	0.20	1.89	0.02	650.92	217.49	790.99	518.92
JonesFalls4	2	100 Yr	353.79	353.51	0.28	1.84	0.02	1333.12	330.68	1741.70	623.96
JonesFalls4	1	2 Yr	350.76	350.69	0.07			118.22	74.43	529.05	523.08
JonesFalls4	1	10 Yr	351.26	351.13	0.13			295.10	127.52	1236.78	565.06
JonesFalls4	1	100 Yr	351.93	351.71	0.21			659.43	214.45	2531.62	614.41



**LEGEND**

- REACH LENGTHS —
- EXISTING HEC-RAS CROSS SECTIONS —
- EXISTING FLOODPLAIN —
- INEFFECTIVE FLOW AREA - - - -

**PLAN**  
SCALE: 1"=100'



OWNER / DEVELOPER INFORMATION  
MARYLAND TRANSPORTATION AUTHORITY  
2310 BRACING HWY  
BALTIMORE, MD 21224

MARYLAND COORDINATE SYSTEM - HOR. NAD 83/91 MD STATE PLANE VERT. NAVD 88

GREENSPRING VALLEY ROAD  
SW CORNER PARK HEIGHTS AVE  
OWINGS MILLS, MD 21117

**FLOODPLAIN MAP - EXISTING CONDITIONS**

SCALE AS SHOWN	DATE SEPTEMBER 2020	PROJECT NO. 17-10977-002
MDE PROJECT NO. 21-SF-0044		CONTRACT NO. KH-3038-0000
DESIGNED BY PVC		COUNTY BALTIMORE COUNTY
DRAWN BY PVC		LOGMILE
CHECKED BY JJM /MRG		HORIZONTAL SCALE N/A
F.A.P. NO. SEE TITLE SHEET		VERTICAL SCALE N/A

DRAWING NO. **EX. FP - 1** OF **2** SHEET NO. xx OF xx

DESIGN PROFESSIONAL  
JEREMY KOSER  
JOHNSON, MIRIRAN & THOMPSON, INC.  
40 WIGHT AVENUE, HUNT VALLEY, MD 21030  
TEL: 410-329-3100  
EMAIL: JKoser@jmt.com

PROFESSIONAL CERTIFICATION  
I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 31183, EXPIRATION DATE: 1/13/2021.

**ECCLESTON MITIGATION SITE**

REVISIONS



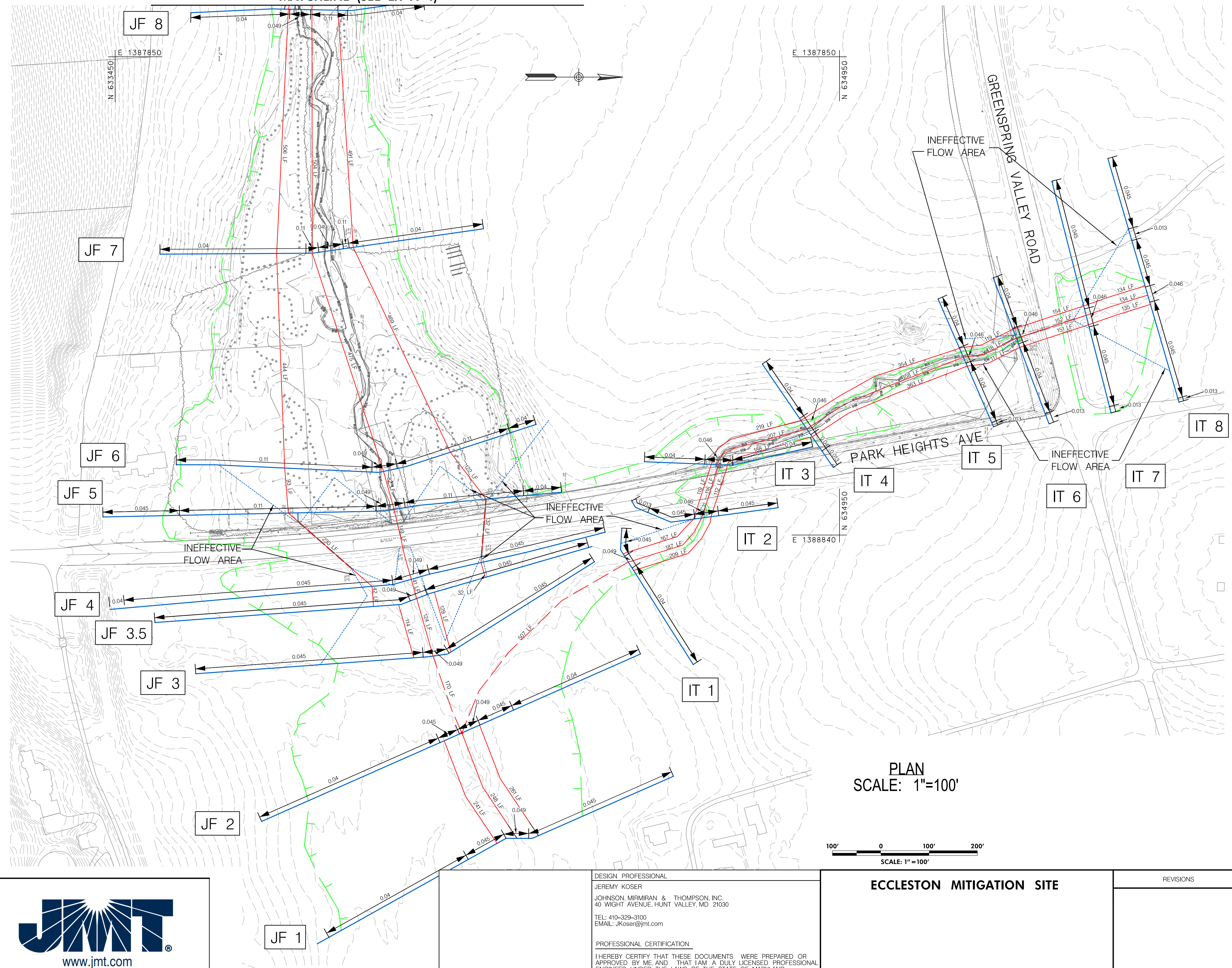
BY: MOCCA

C:\2017\170977\_002\_Eccleston\_Plan\_Turnin\CAD\DWG\FP\_E001\_ECCLESTON\_2D.dwg Thursday, October 08, 2020 11:07:57 AM

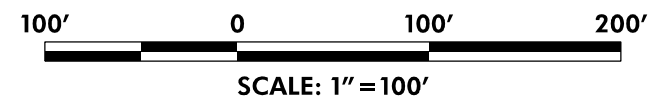
MATCHLINE (SEE EX FP-1)

LEGEND

- REACH LENGTHS —
- EXISTING HEC-RAS CROSS SECTIONS —
- EXISTING FLOODPLAIN —
- INEFFECTIVE FLOW AREA - - -



PLAN SCALE: 1"=100'



ECCLESTON MITIGATION SITE

DESIGN PROFESSIONAL  
 JEREMY KOSEK  
 JOHNSON, MIRMIRAN & THOMPSON, INC.  
 40 WIGHT AVENUE, HUNT VALLEY, MD 21030  
 TEL: 410-329-3100  
 EMAIL: JKoser@jmt.com

PROFESSIONAL CERTIFICATION  
 I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 31183, EXPIRATION DATE: 1/13/2021.

OWNER / DEVELOPER INFORMATION  
 MARYLAND TRANSPORTATION AUTHORITY  
 2310 BRIDGING HWY  
 BALTIMORE, MD 21224

MARYLAND COORDINATE SYSTEM - HOR. NAD 83/91 MD STATE PLANE VERT. NAVD 88

GREENSPRING VALLEY ROAD  
 SW CORNER PARK HEIGHTS AVE  
 OWINGS MILLS, MD 21117

FLOODPLAIN MAP - EXISTING CONDITIONS

SCALE AS SHOWN	DATE SEPTEMBER 2020	PROJECT NO. 17-10977-002
MDE PROJECT NO. 21-SF-0044	CONTRACT NO. KH-3038-0000	
DESIGNED BY PVC	COUNTY BALTIMORE COUNTY	
DRAWN BY PVC	LOGMILE	
CHECKED BY JJM /MRG	HORIZONTAL SCALE N/A	
F.A.P. NO. SEE TITLE SHEET	VERTICAL SCALE N/A	

DRAWING NO. EX. FP - 2 OF 2 SHEET NO. xx OF xx

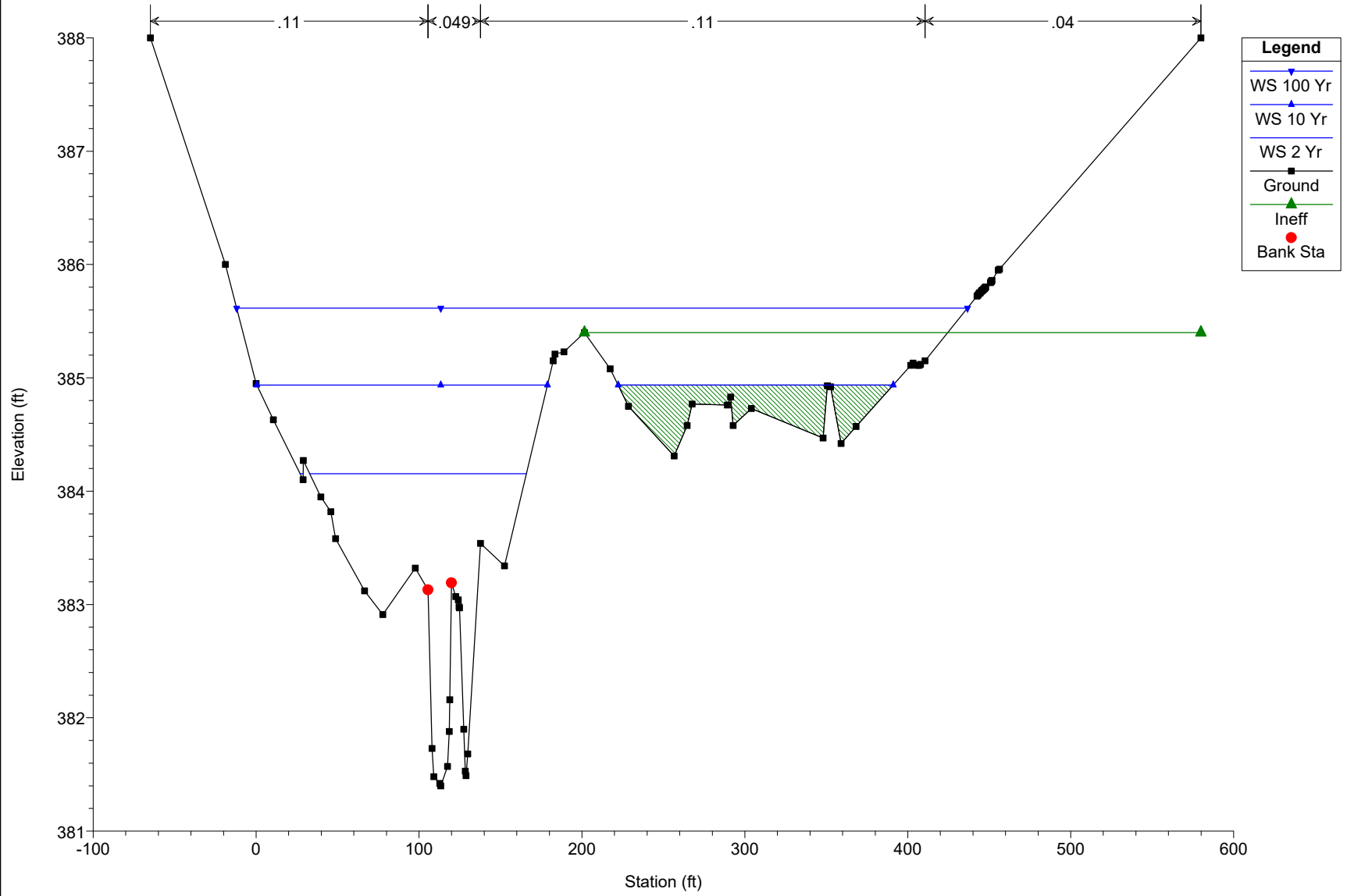


BY: MOCCA



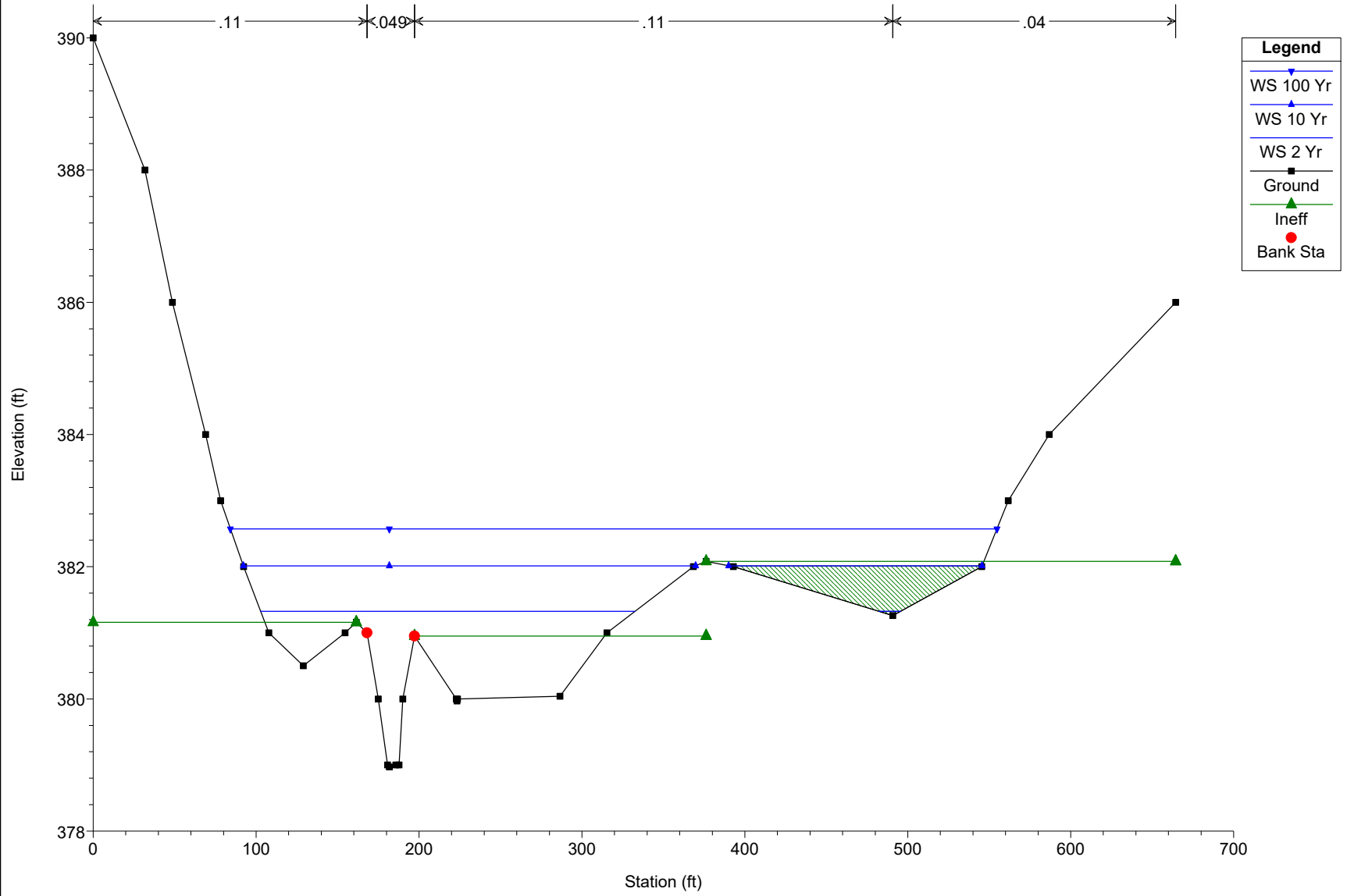
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = Jones Falls RS = 15



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

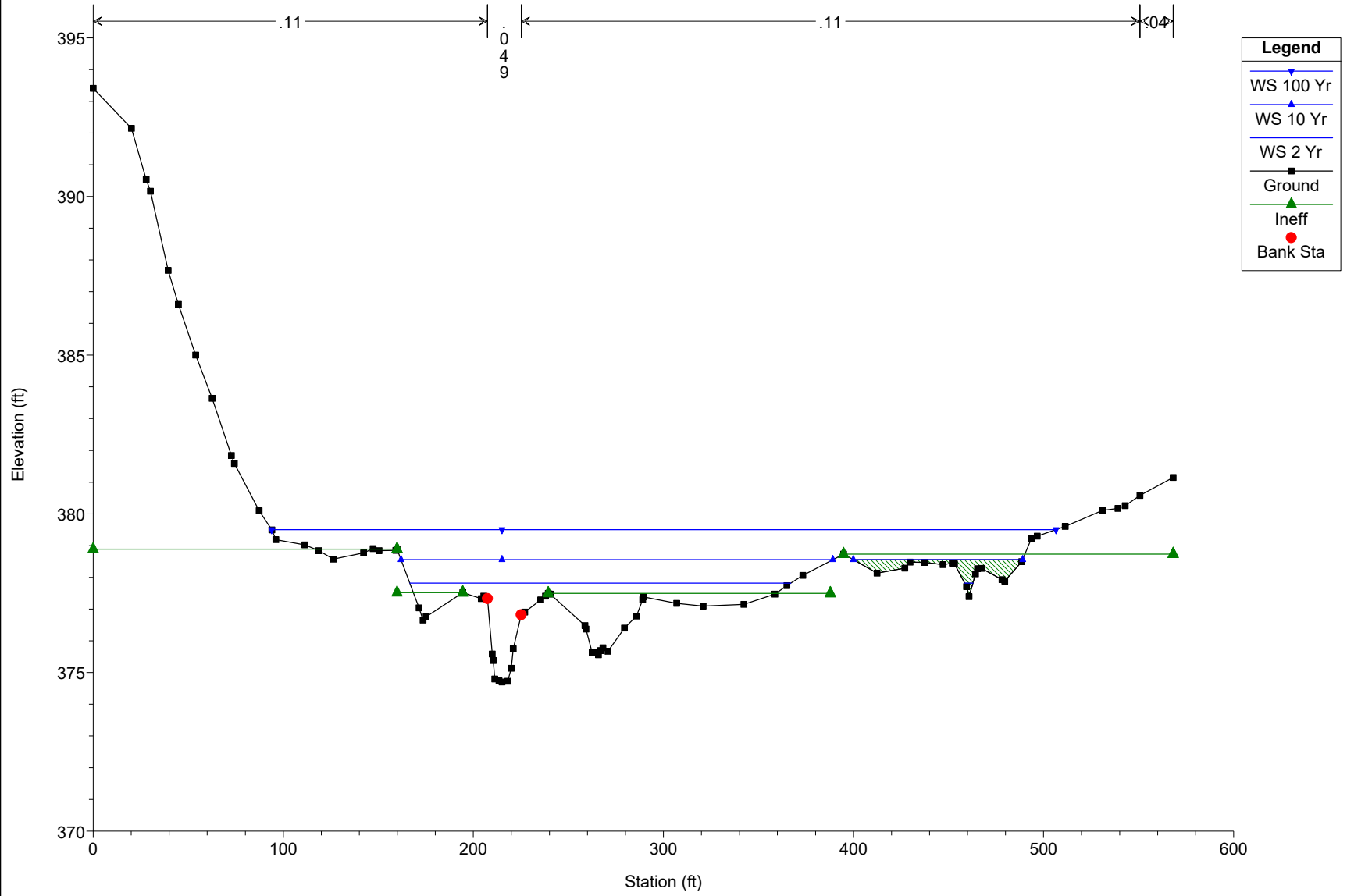
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EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

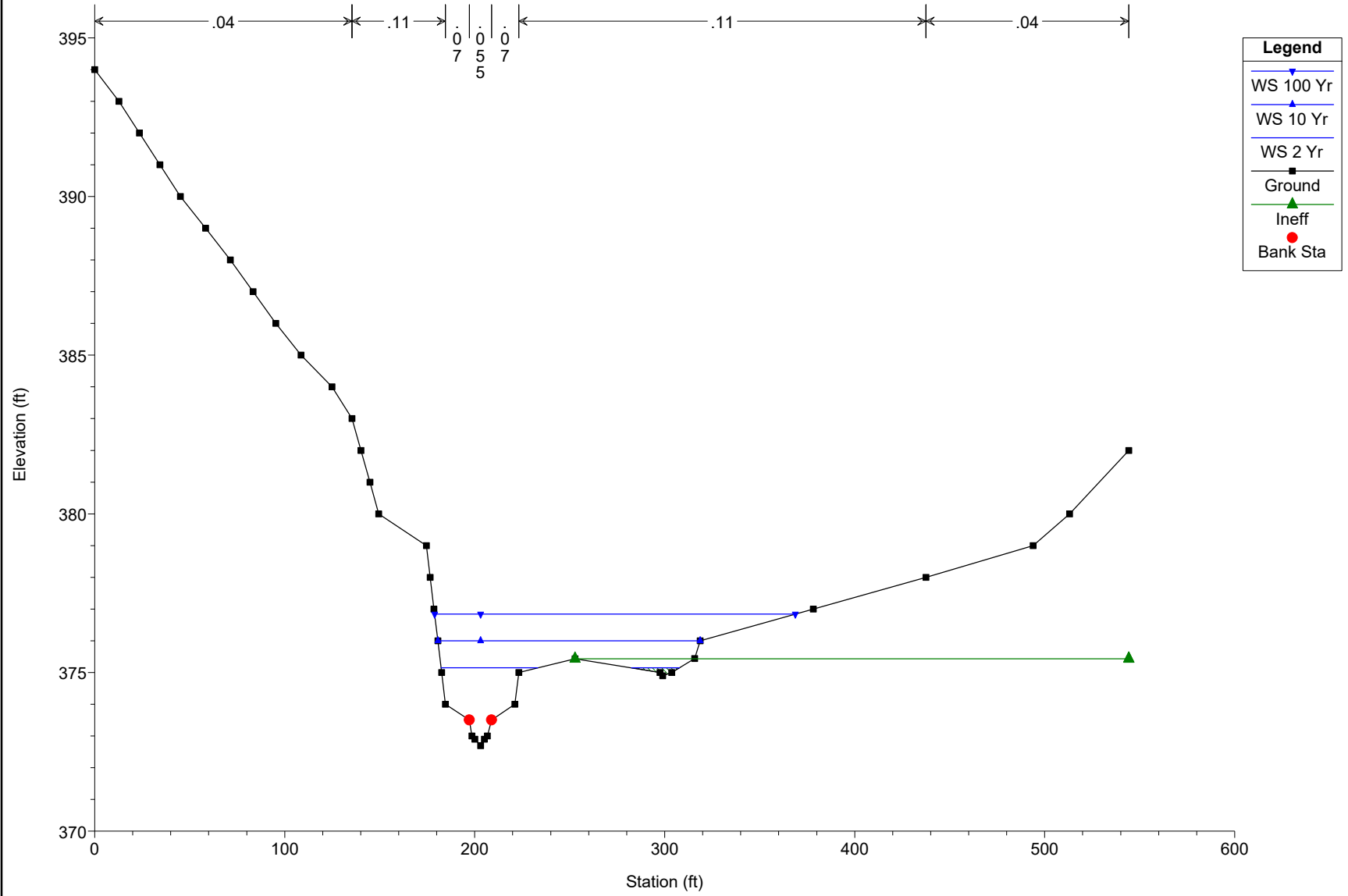
River = Jones Falls Reach = Jones Falls RS = 14



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

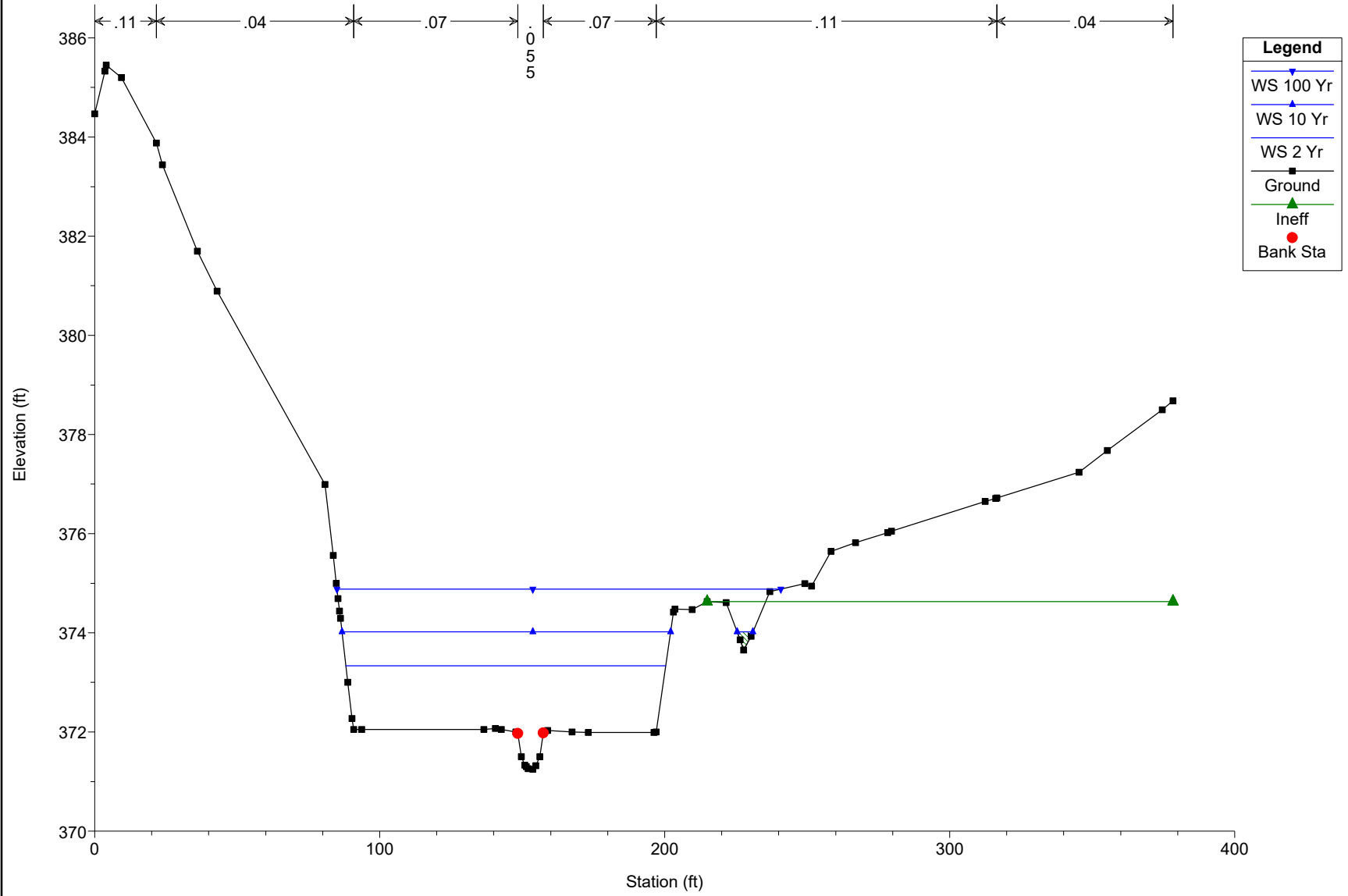
River = Jones Falls Reach = Jones Falls RS = 13.6



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

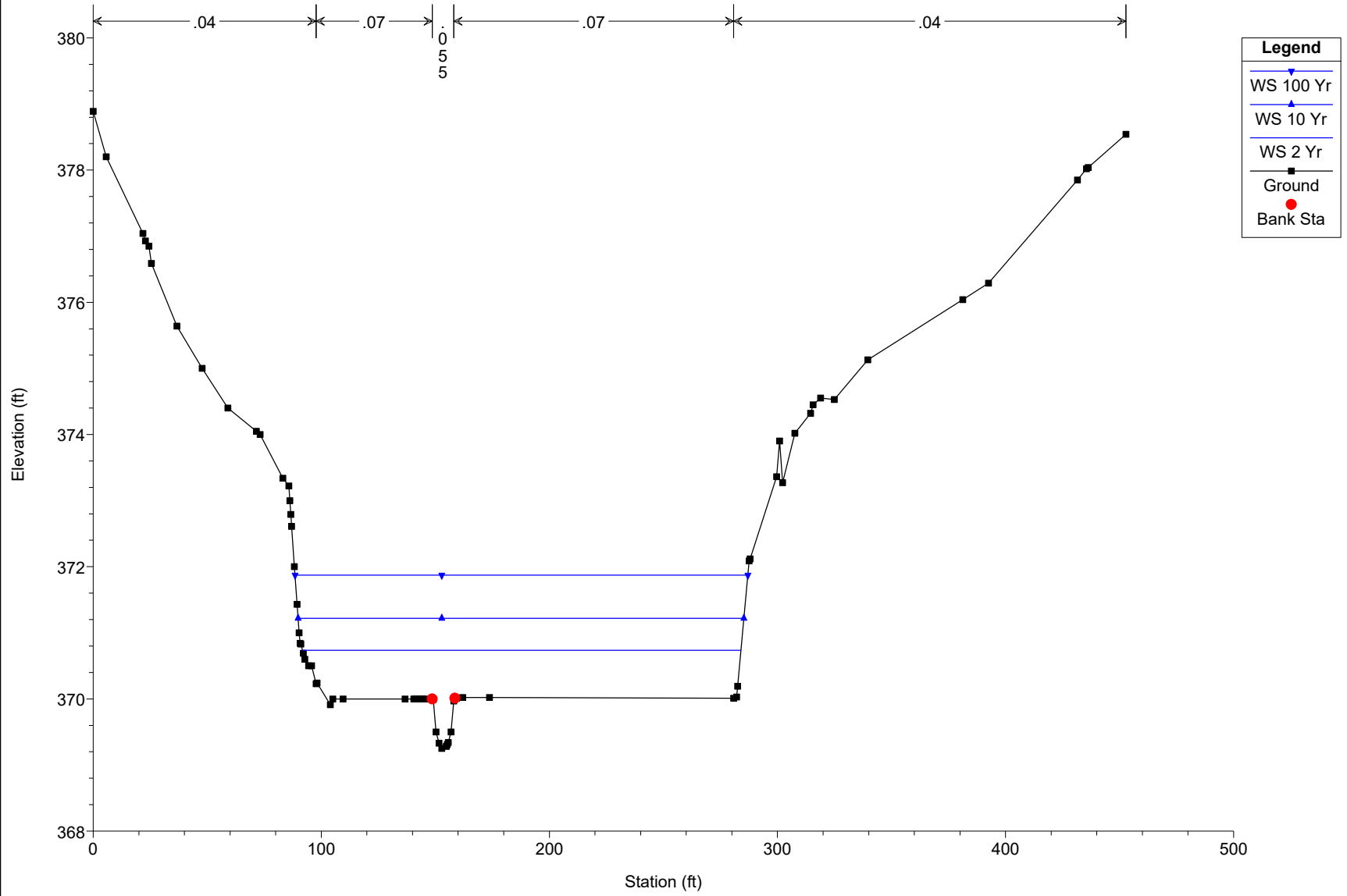
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls RS = 13.5



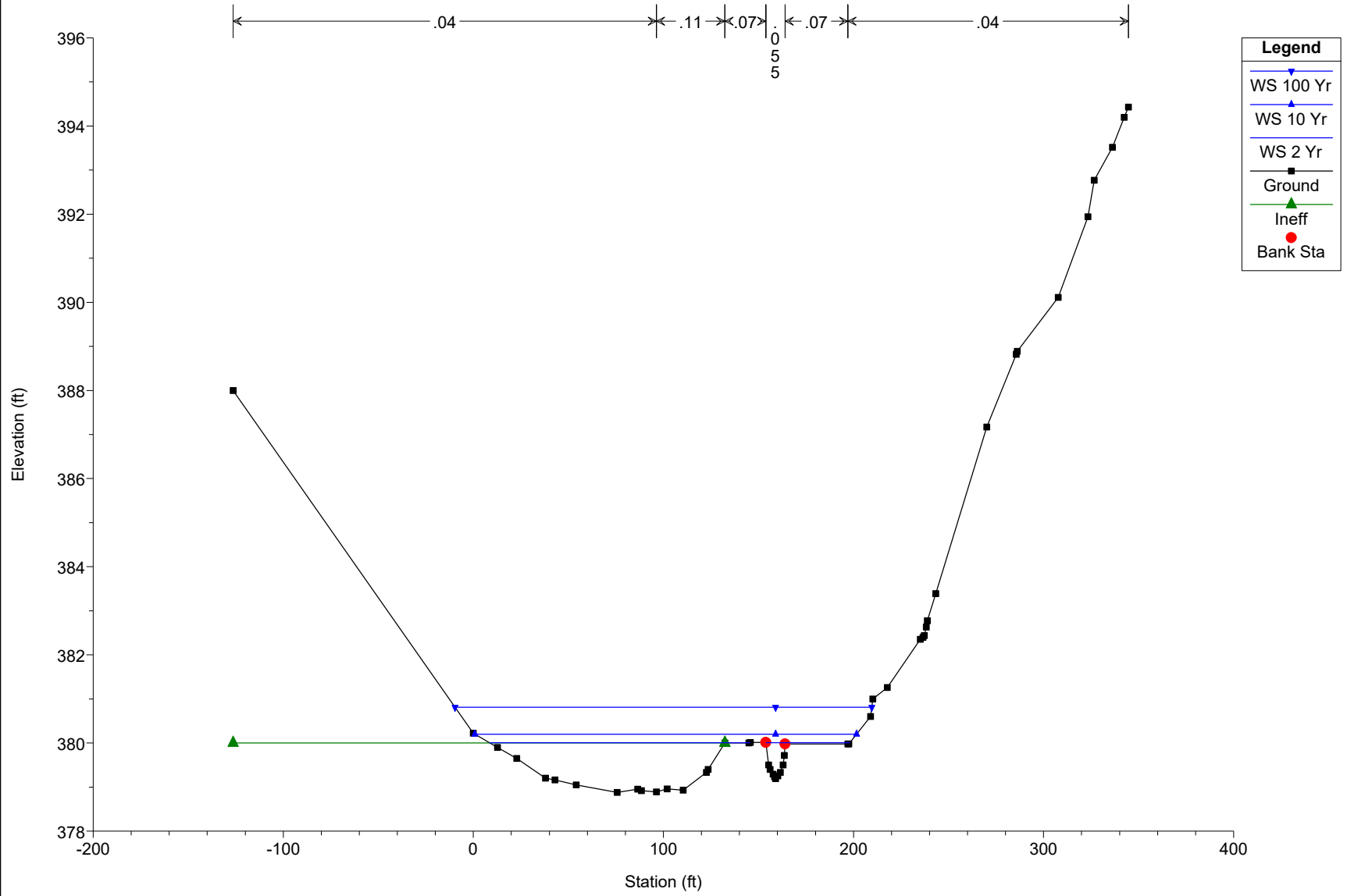
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls RS = 13



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

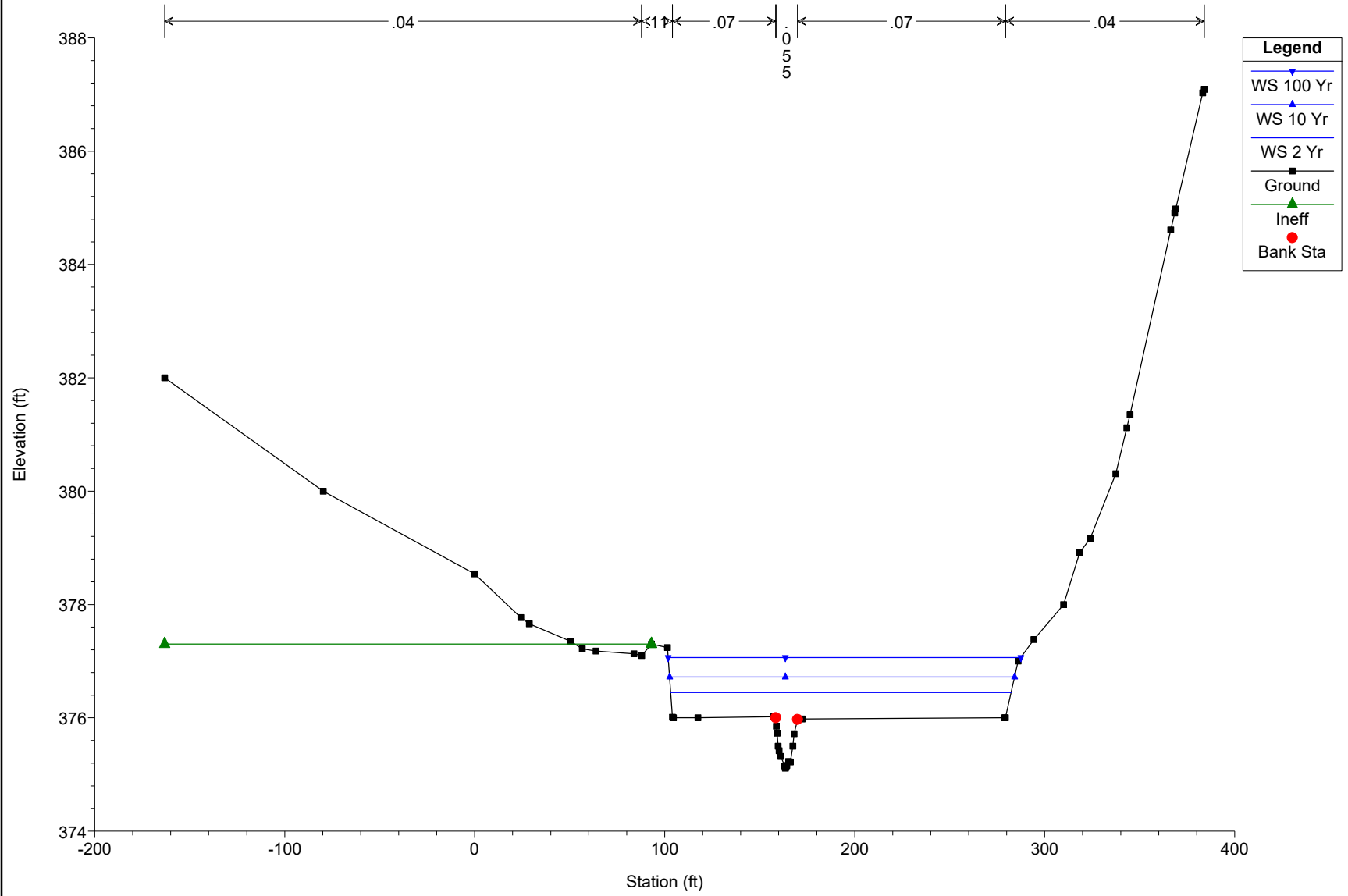
Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = North Tributary RS = 2



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = North Tributary RS = 1.5

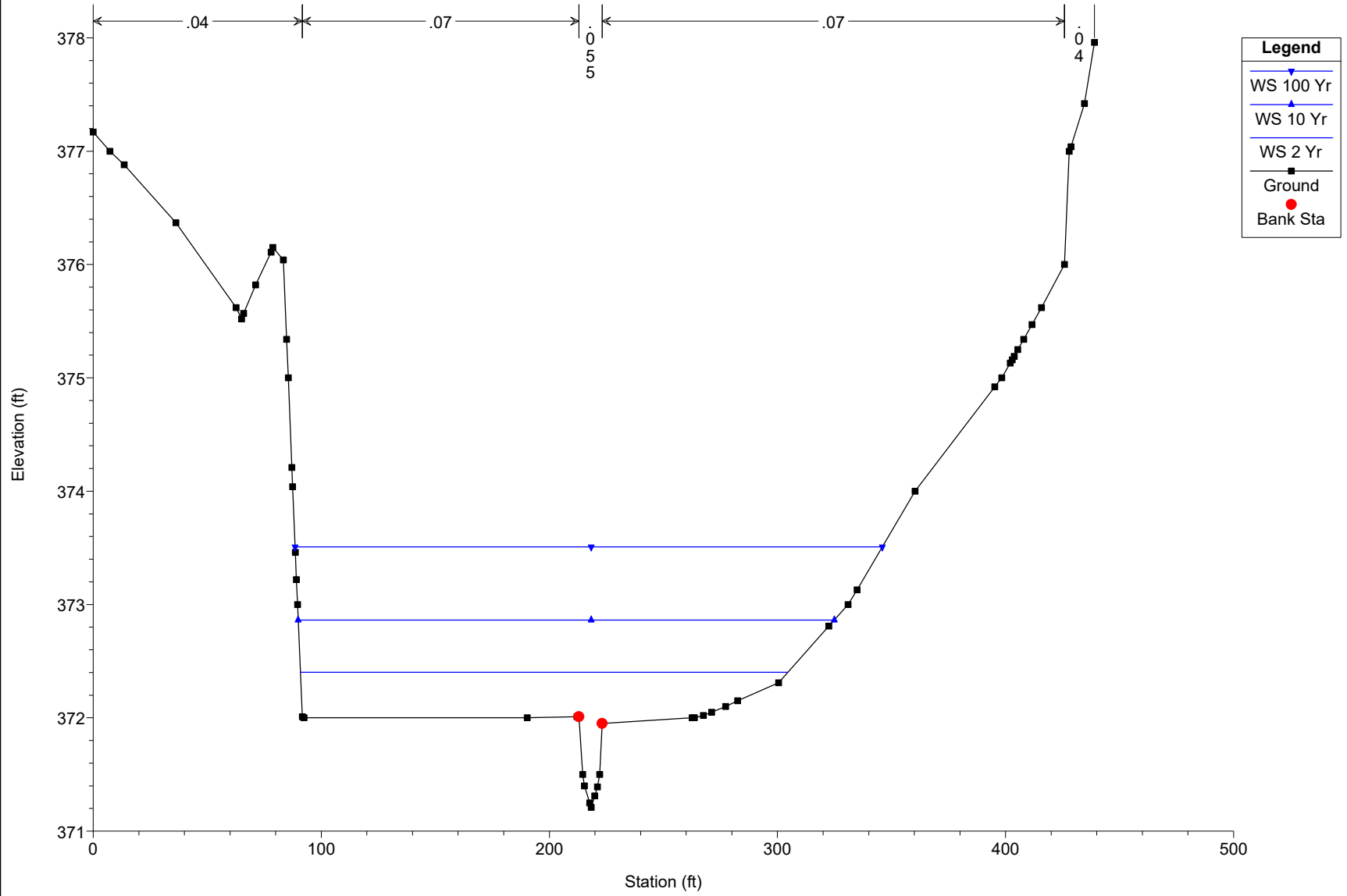




EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = North Tributary RS = 1



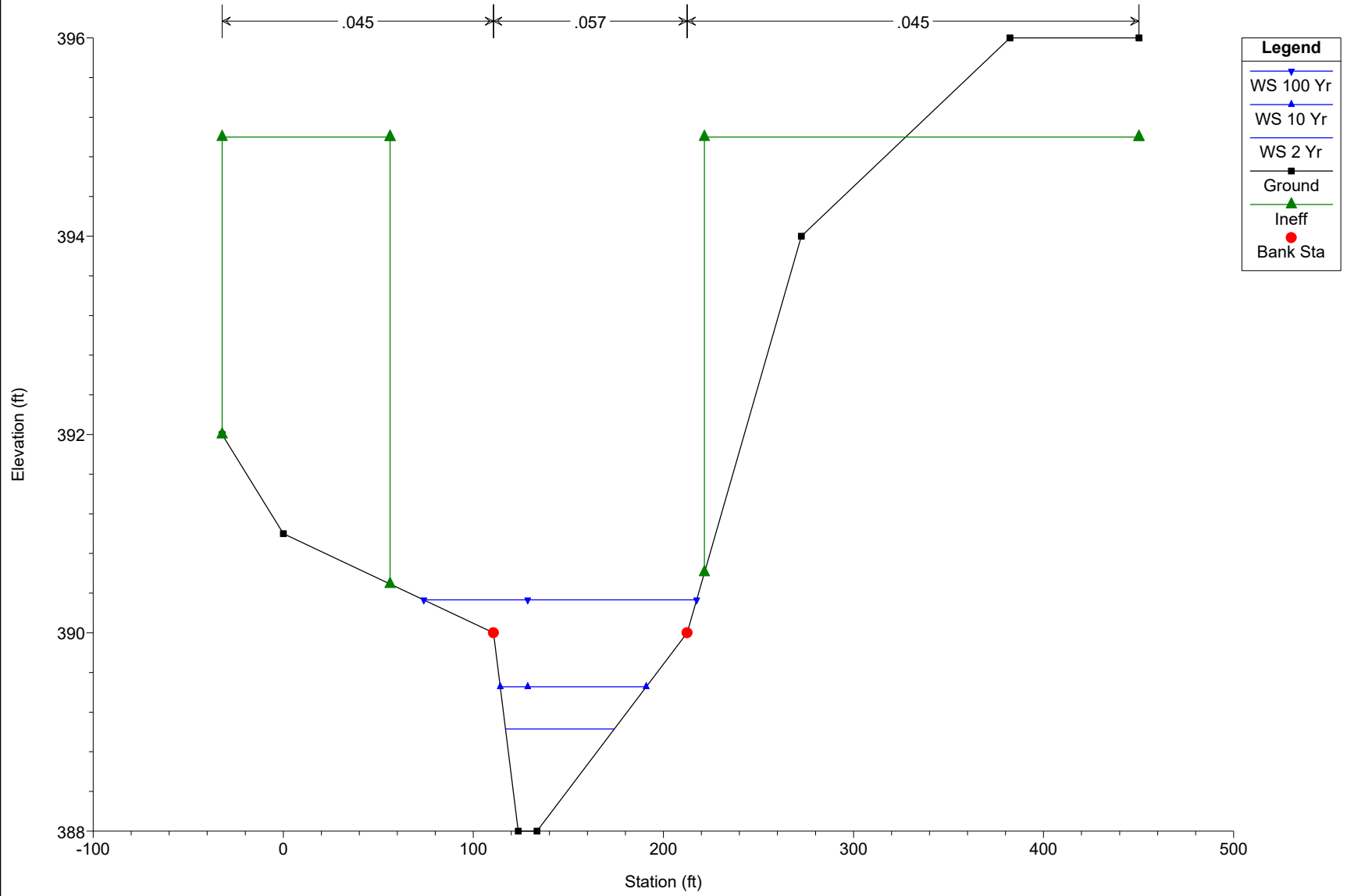
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

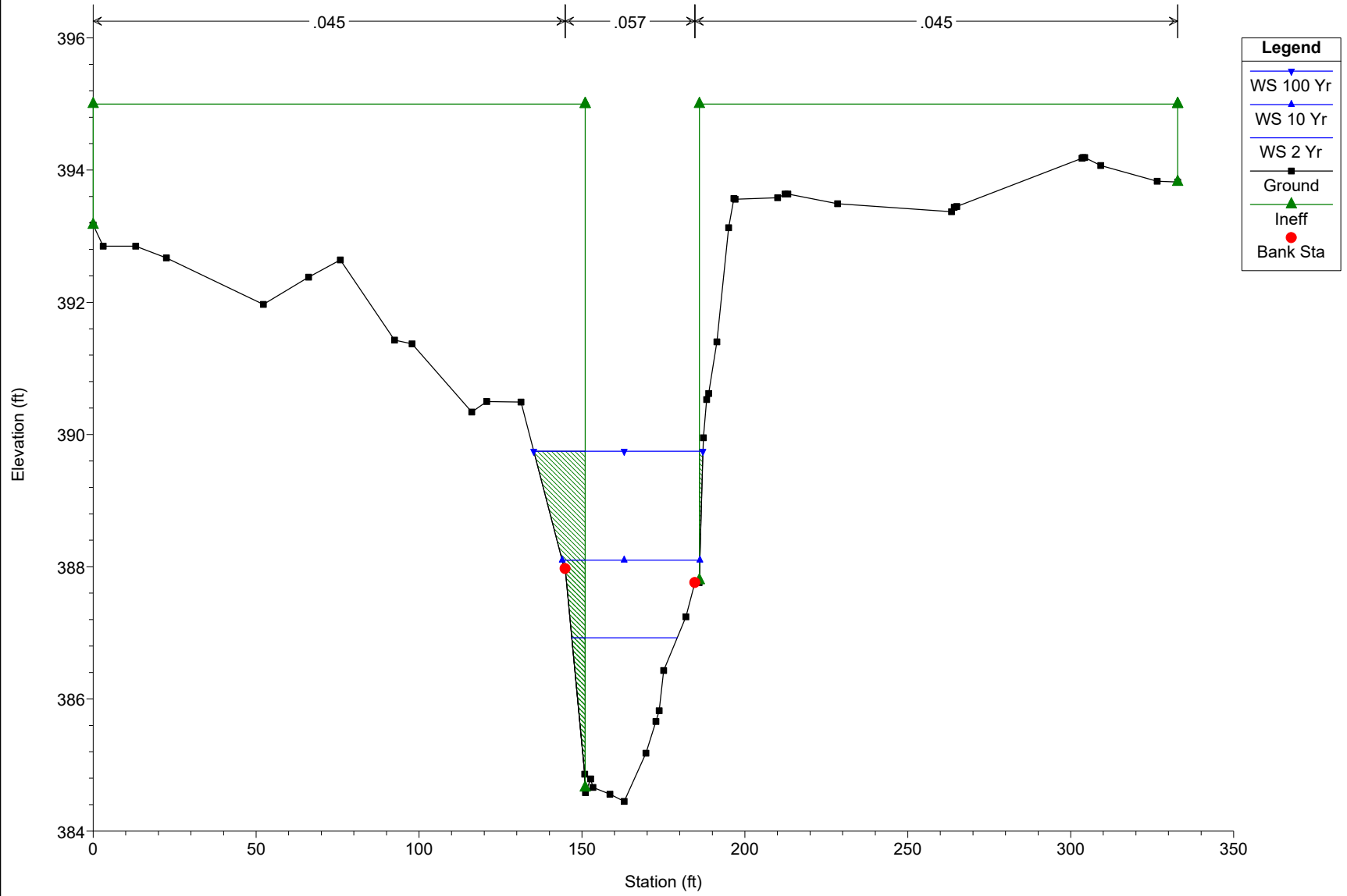
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = South Tributary RS = 8



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

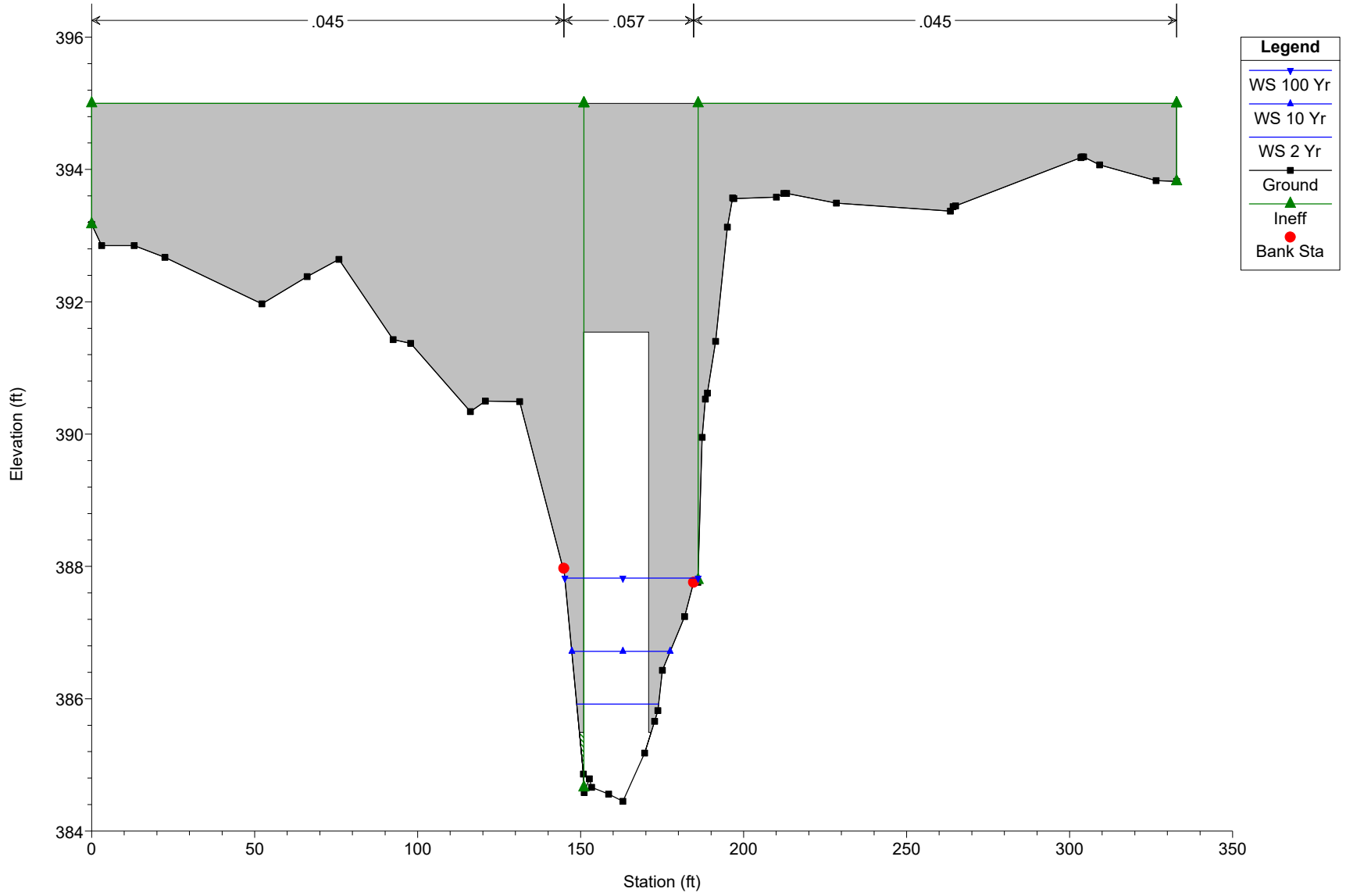
Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = South Tributary RS = 7



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

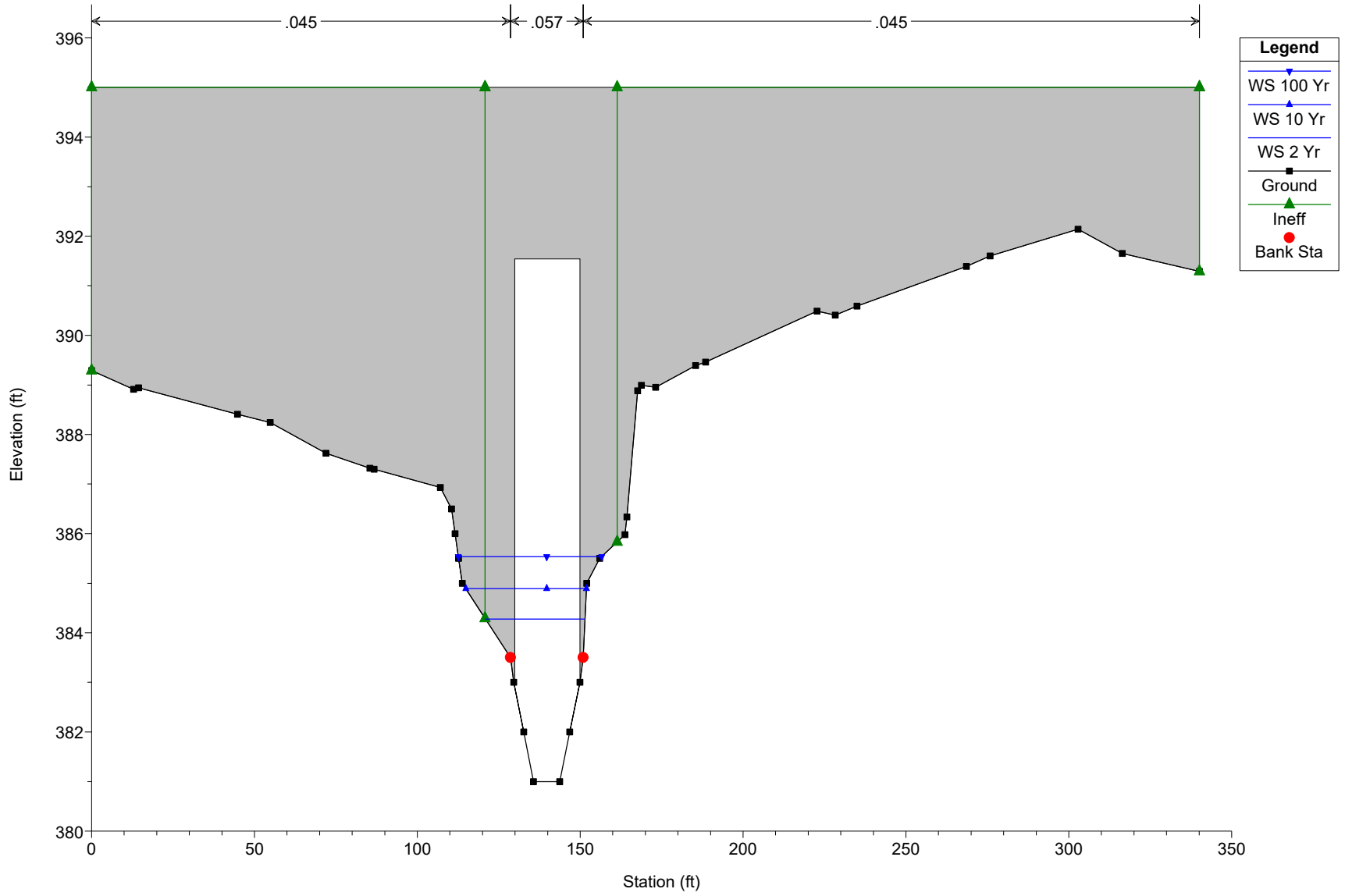
River = Jones Falls Reach = South Tributary RS = 6.5 BR Cliffholme Rd Bridge



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

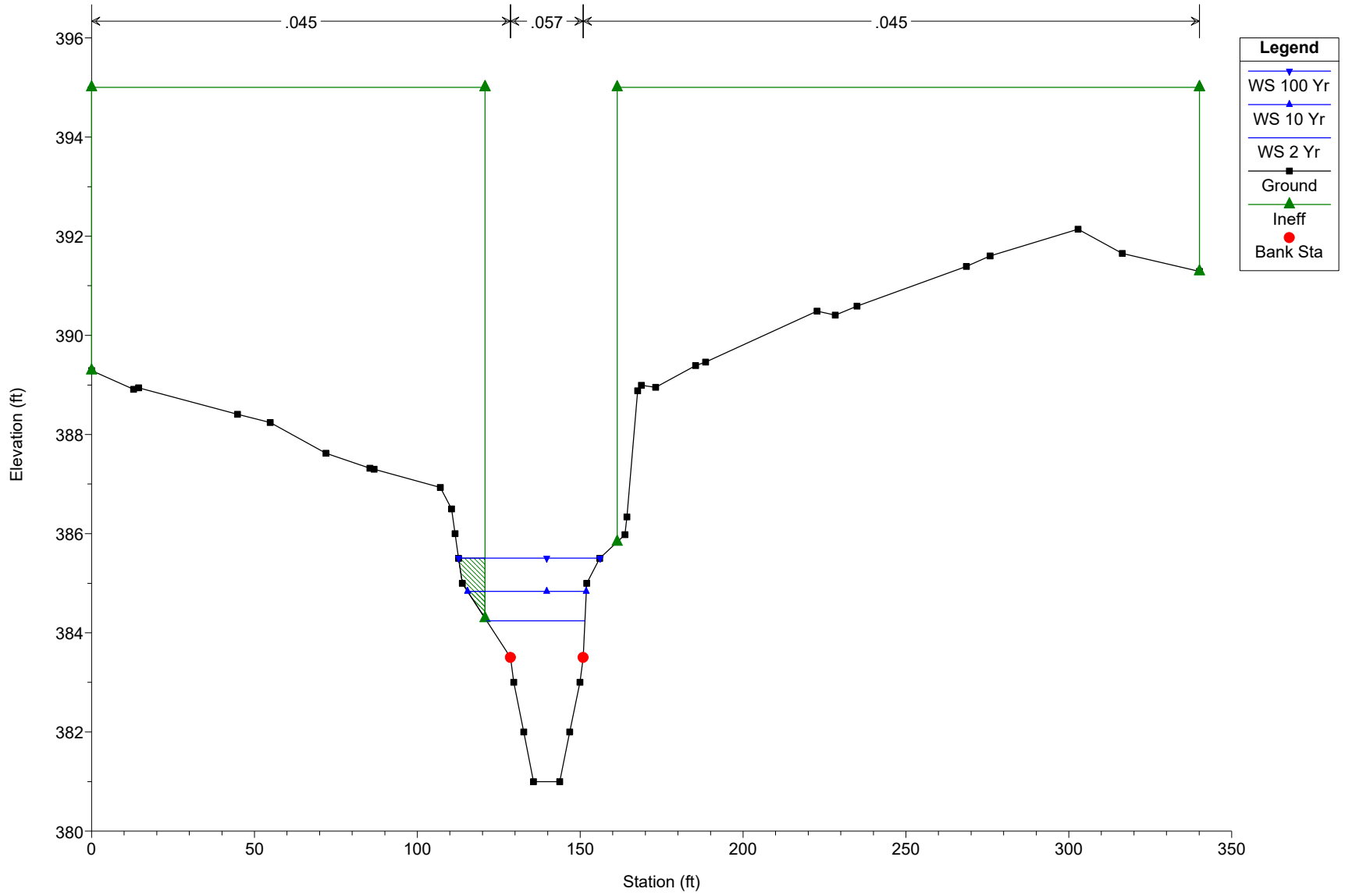
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = South Tributary RS = 6.5 BR Cliffholme Rd Bridge



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

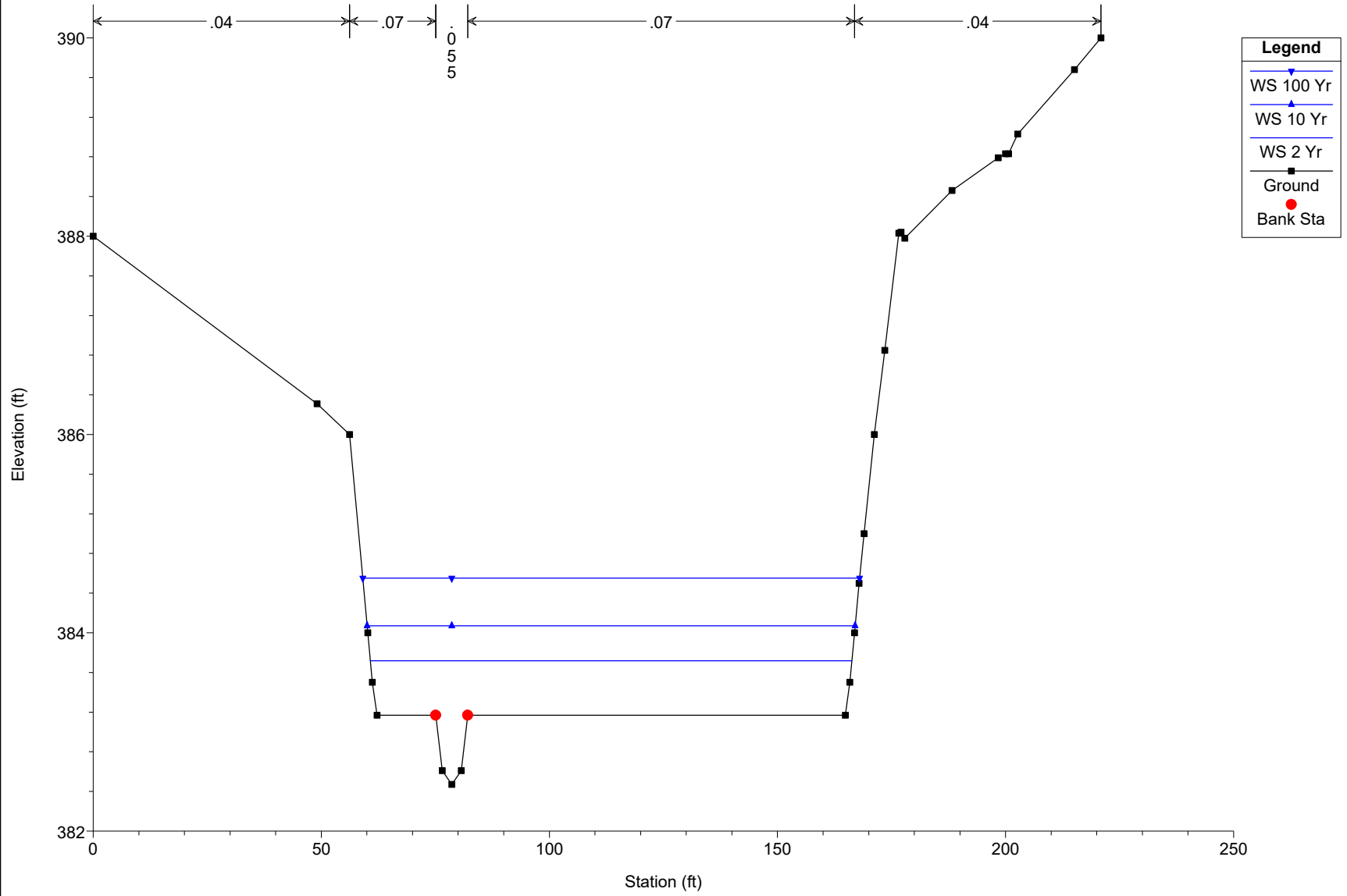
Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = South Tributary RS = 6



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = South Tributary RS = 5



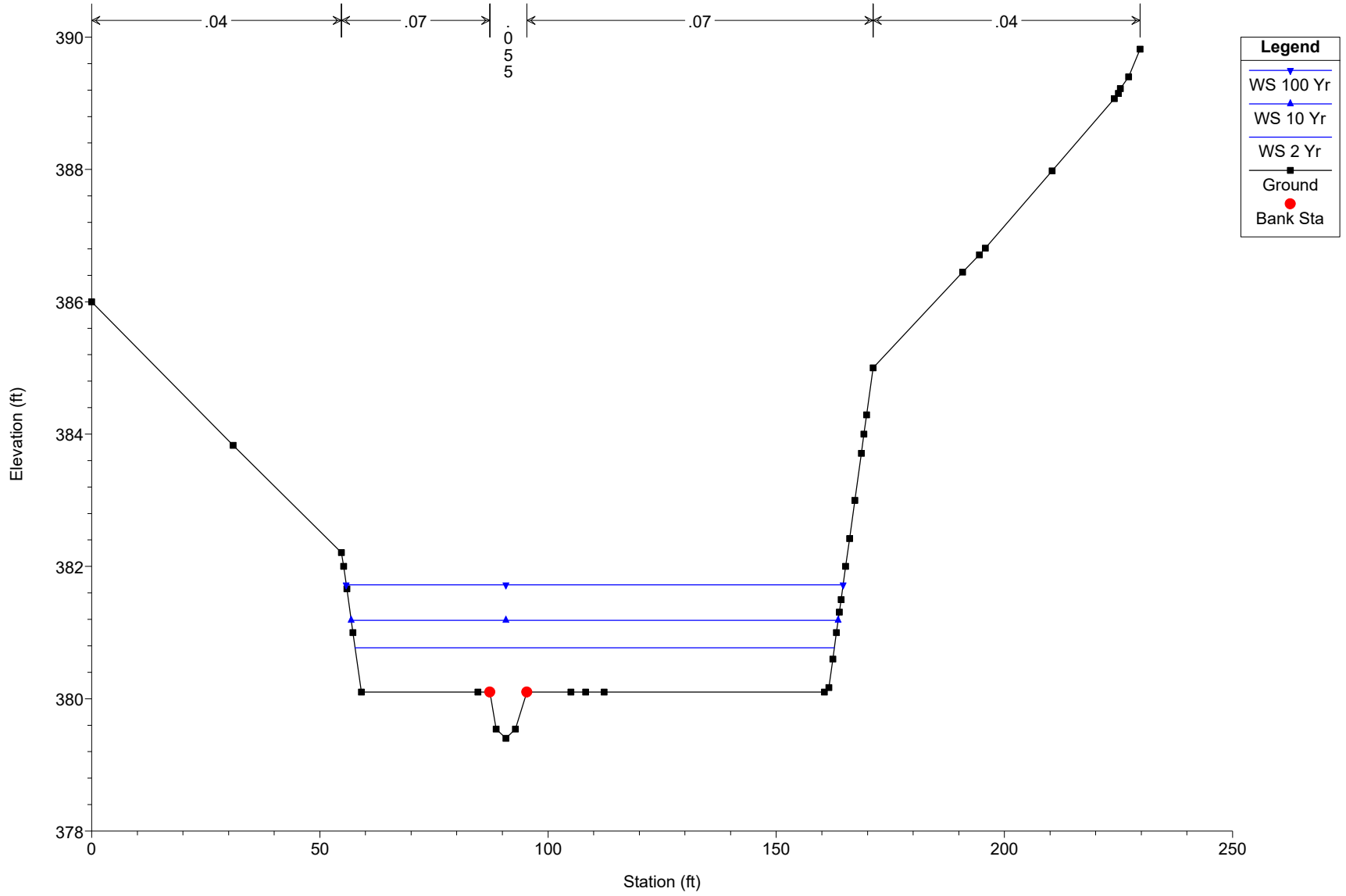
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

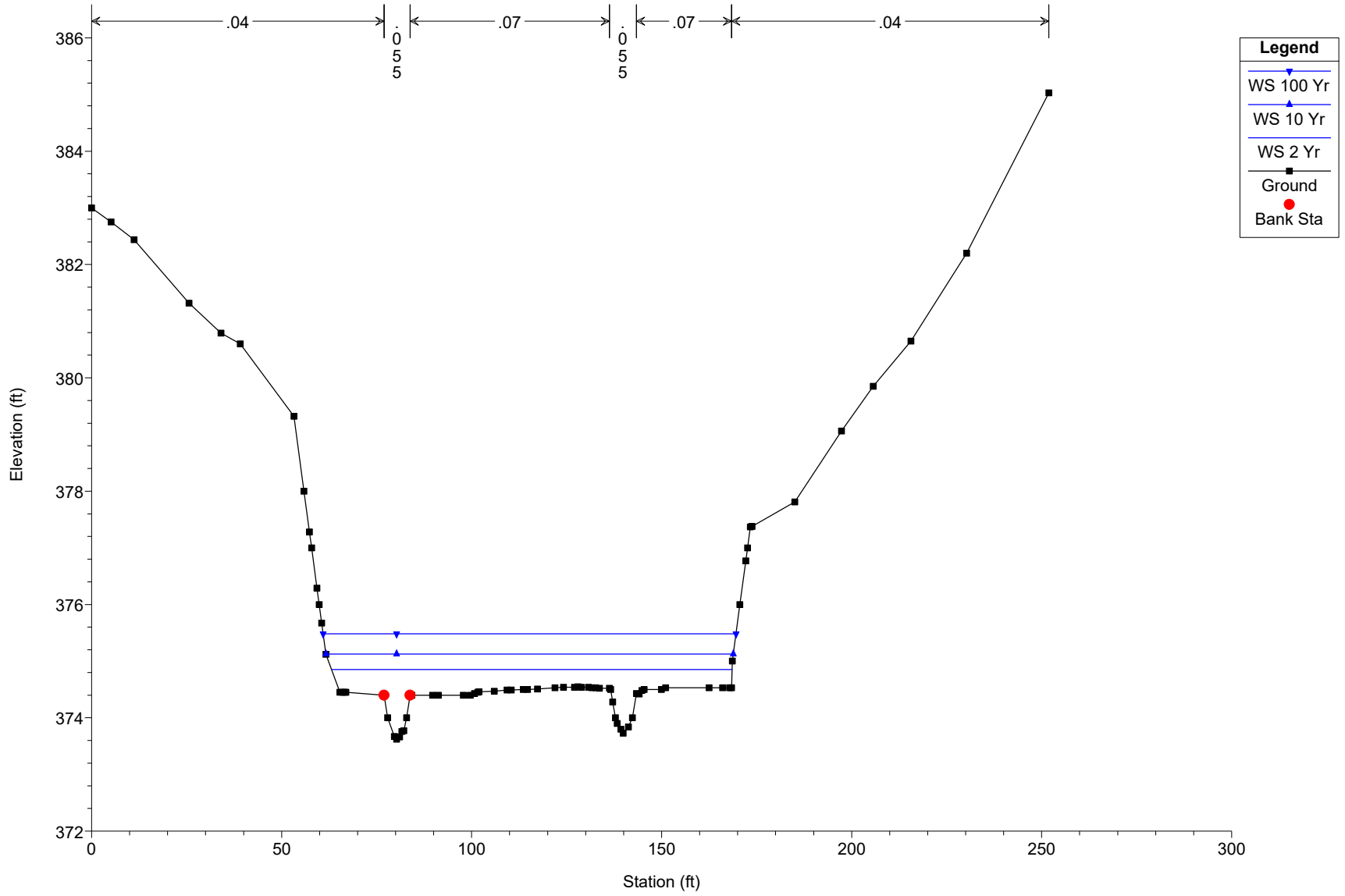
River = Jones Falls Reach = South Tributary RS = 4





EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = South Tributary RS = 3



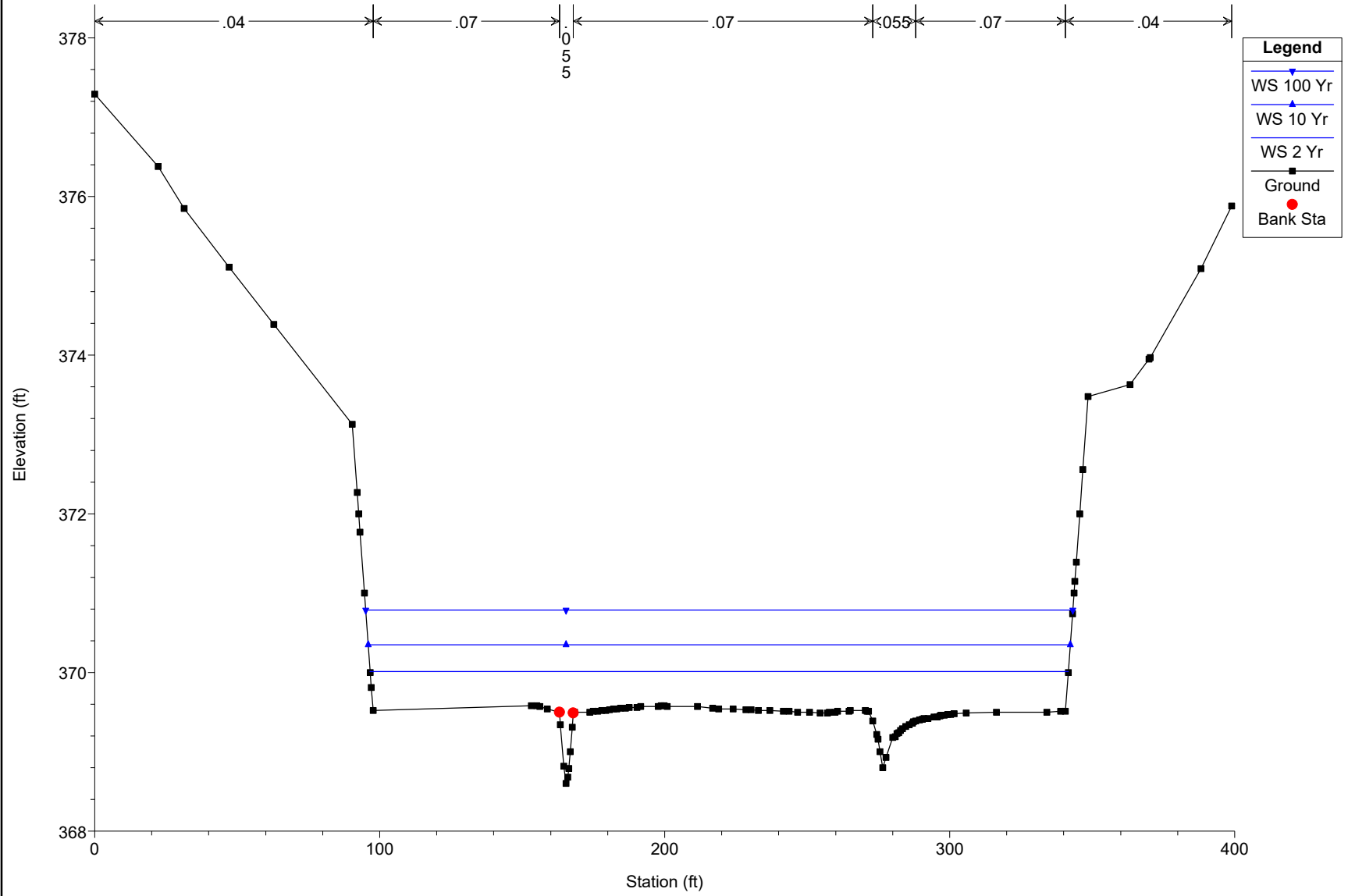
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

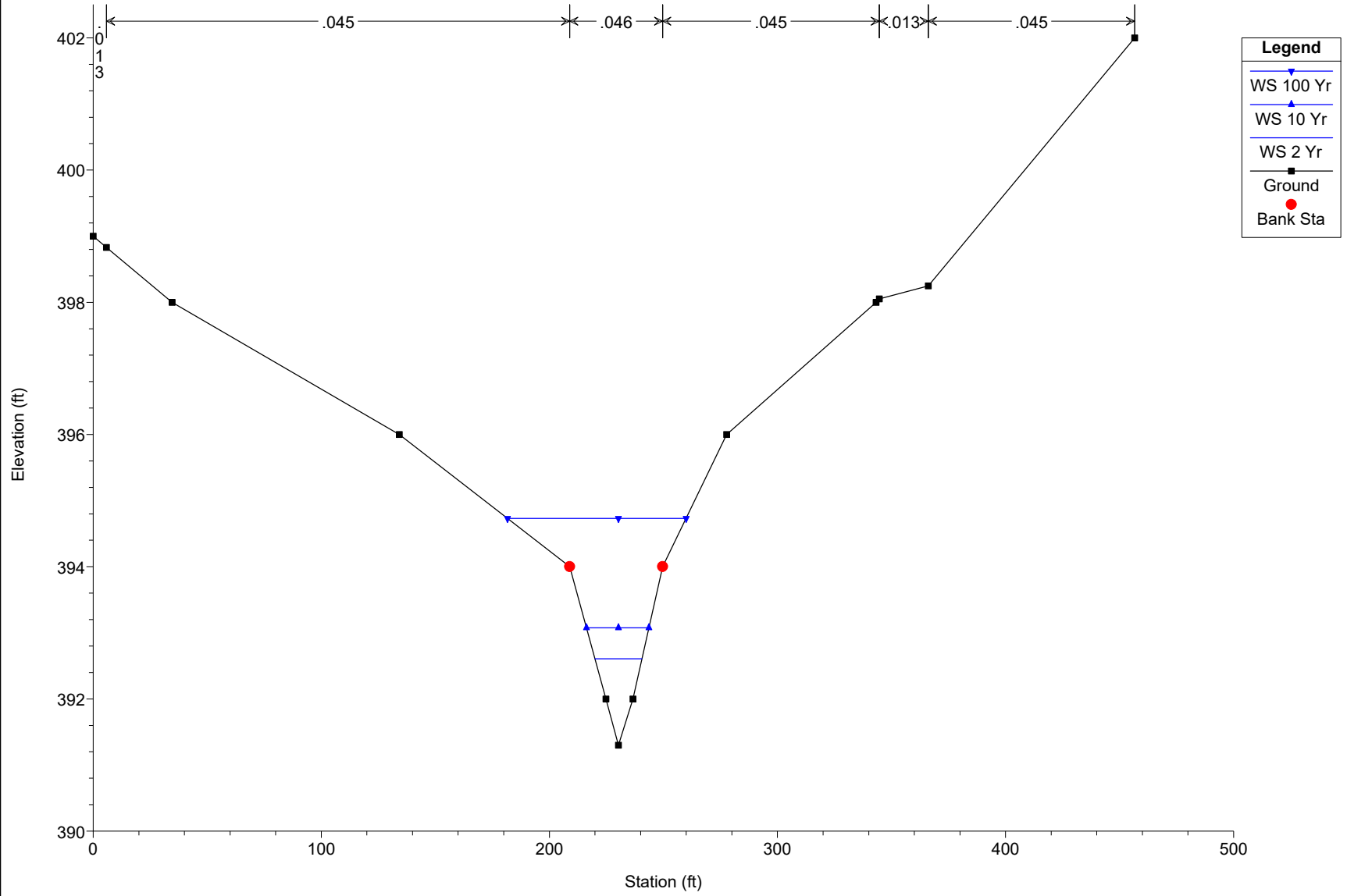
River = Jones Falls Reach = South Tributary RS = 2



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 8



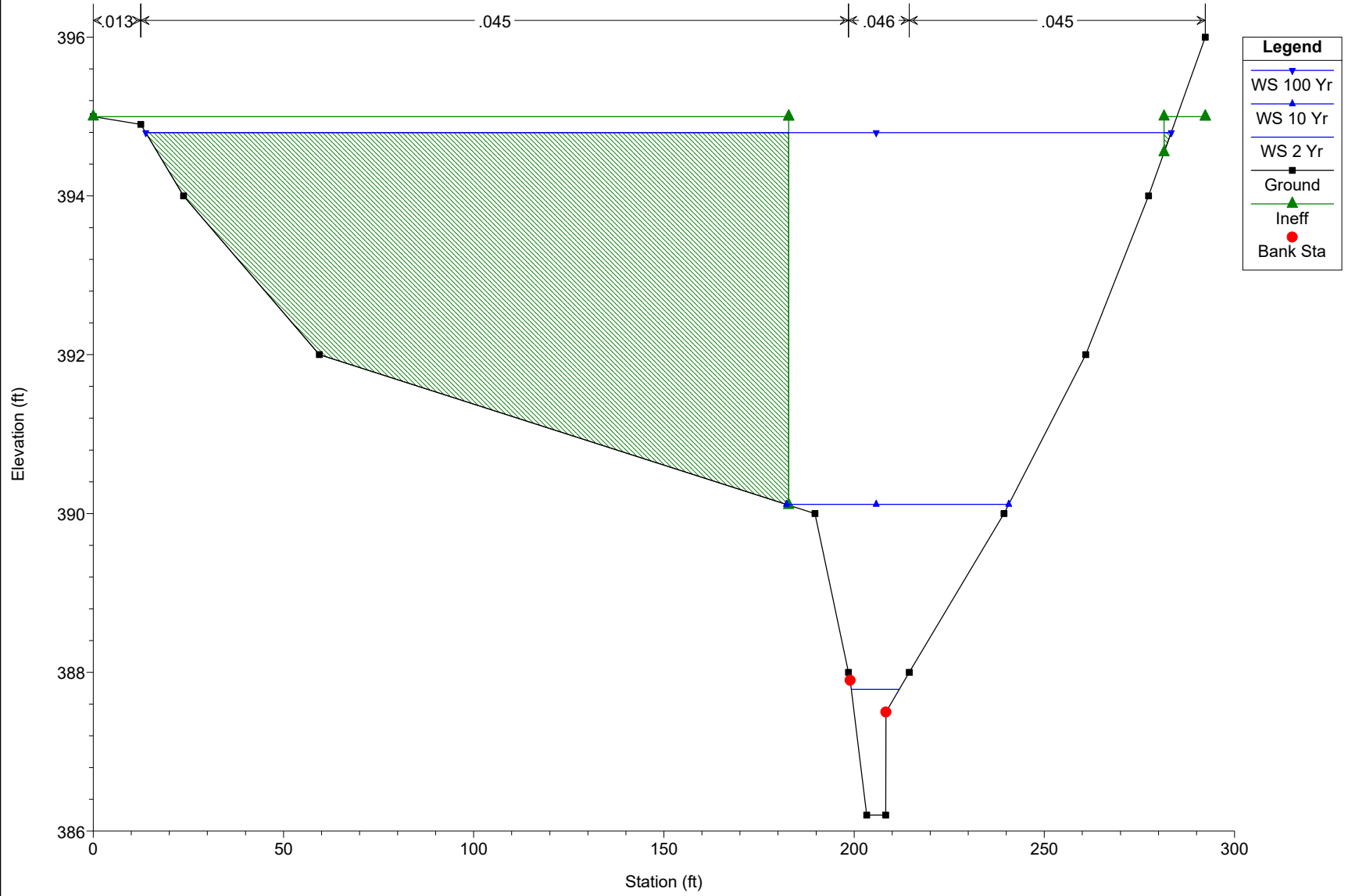
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 7



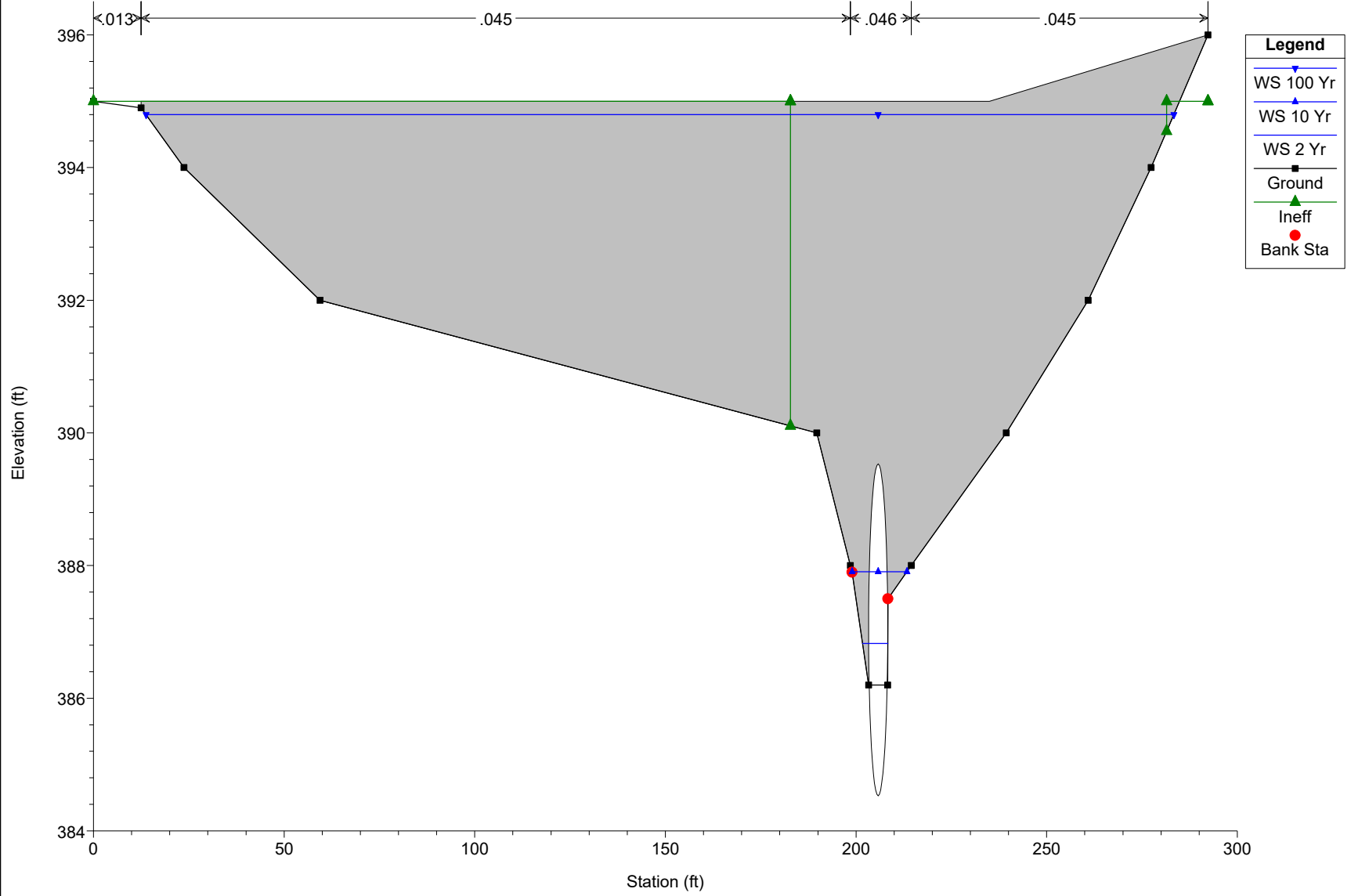
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Ineff
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

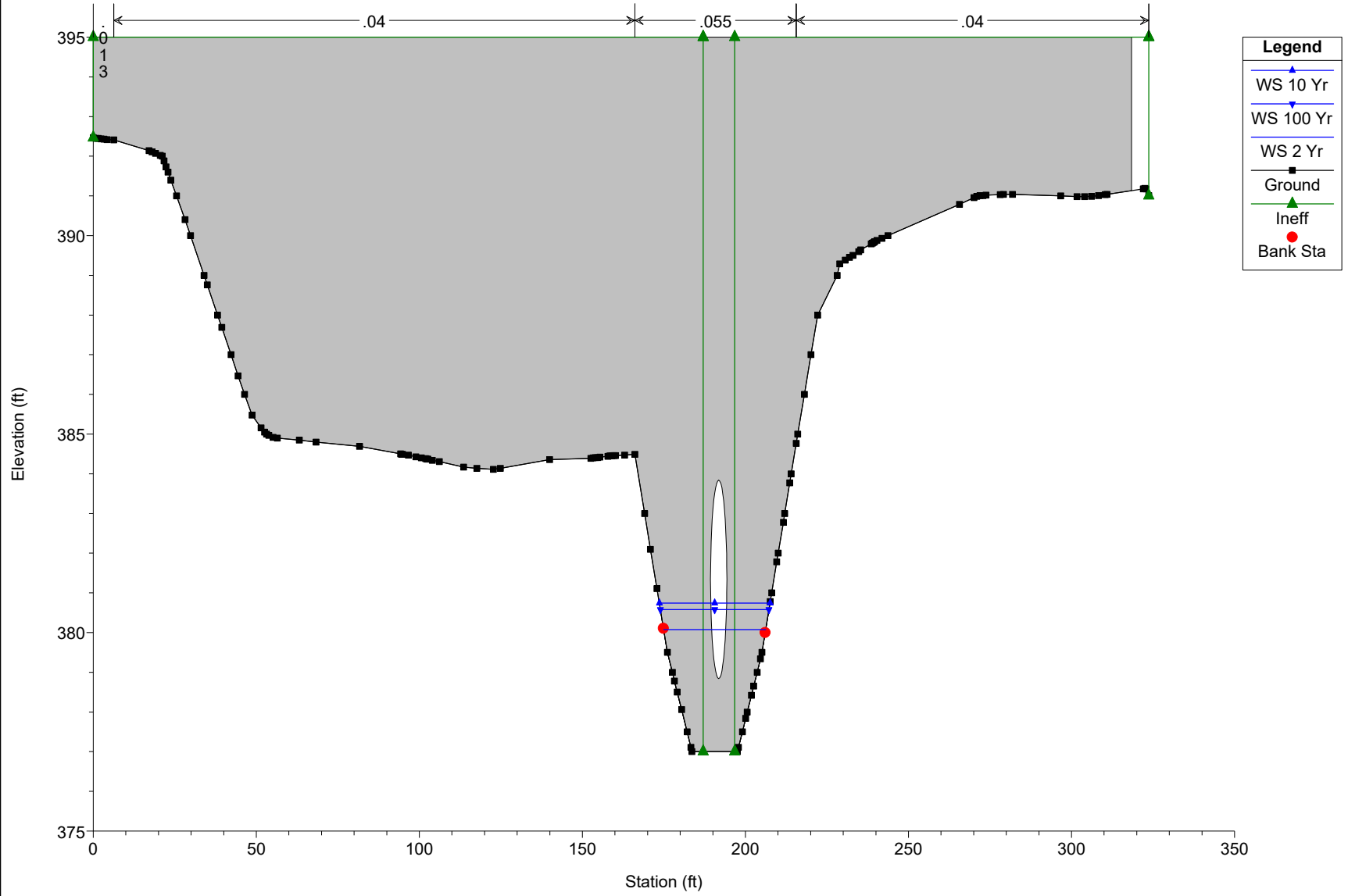
River = Jones Falls Reach = IntsTrib RS = 6.5 Culv Culvert under Greenspring Valley Rd.



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

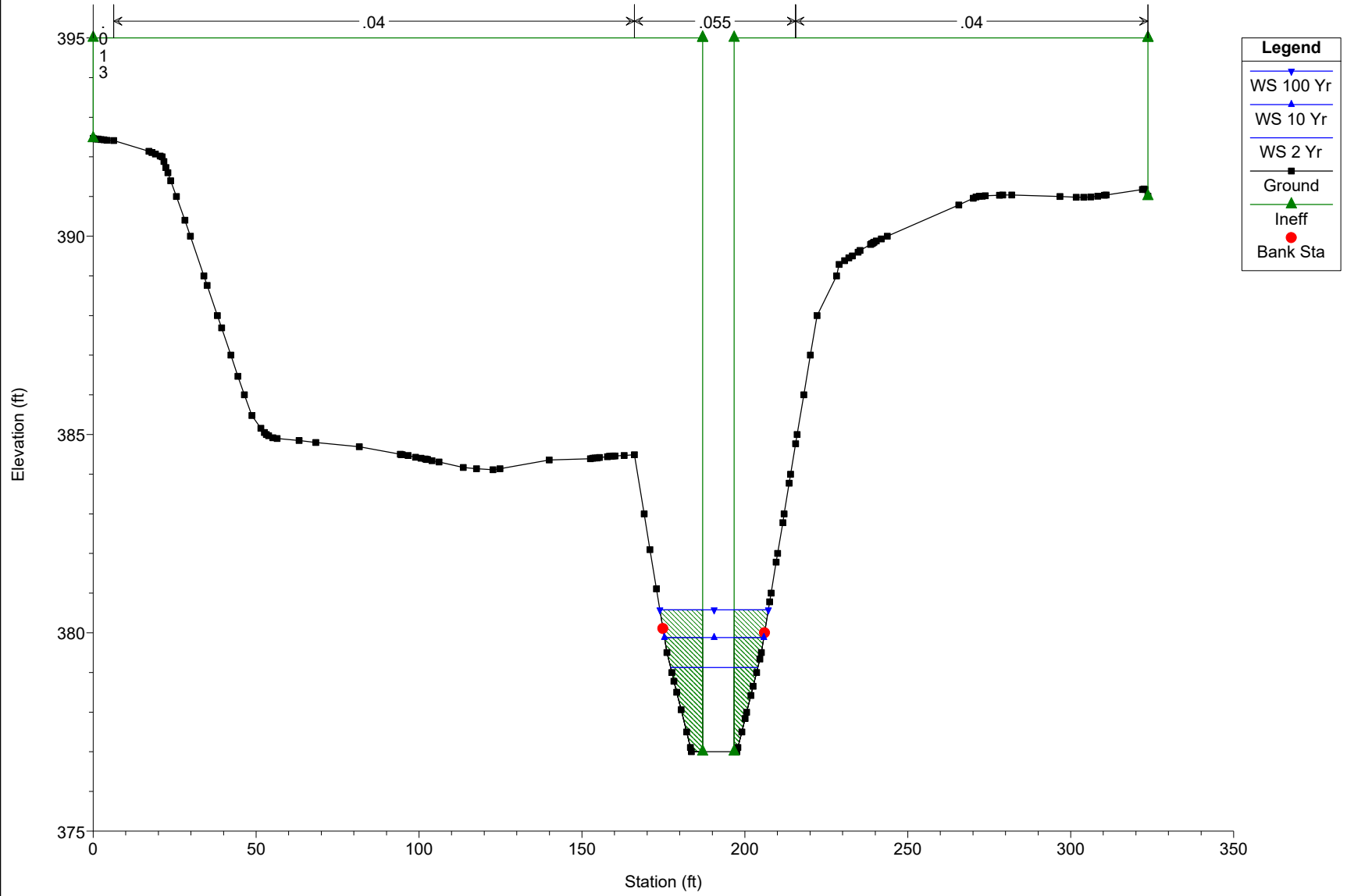
River = Jones Falls Reach = IntsTrib RS = 6.5 Culv Culvert under Greenspring Valley Rd.



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

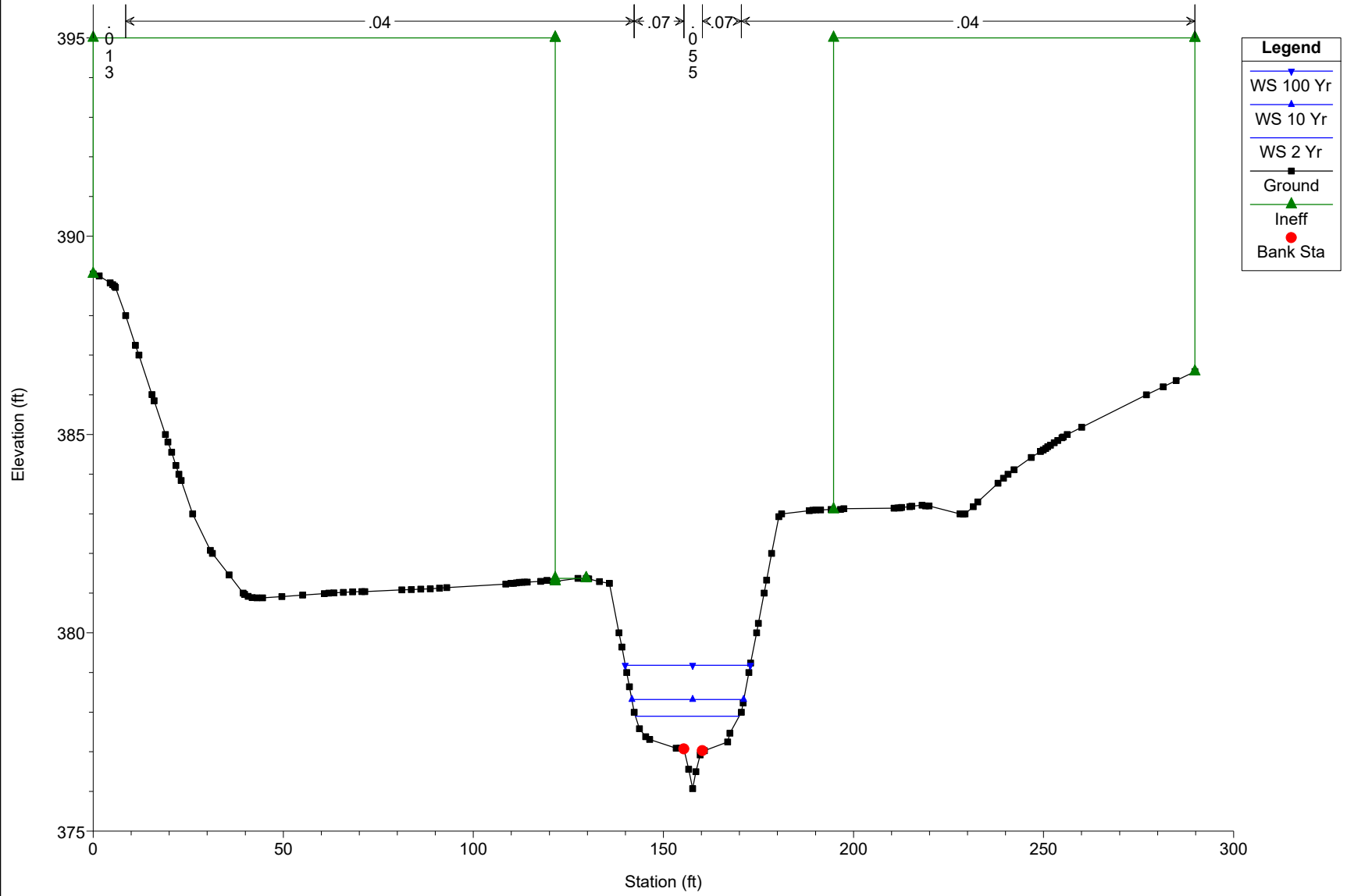
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 6



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

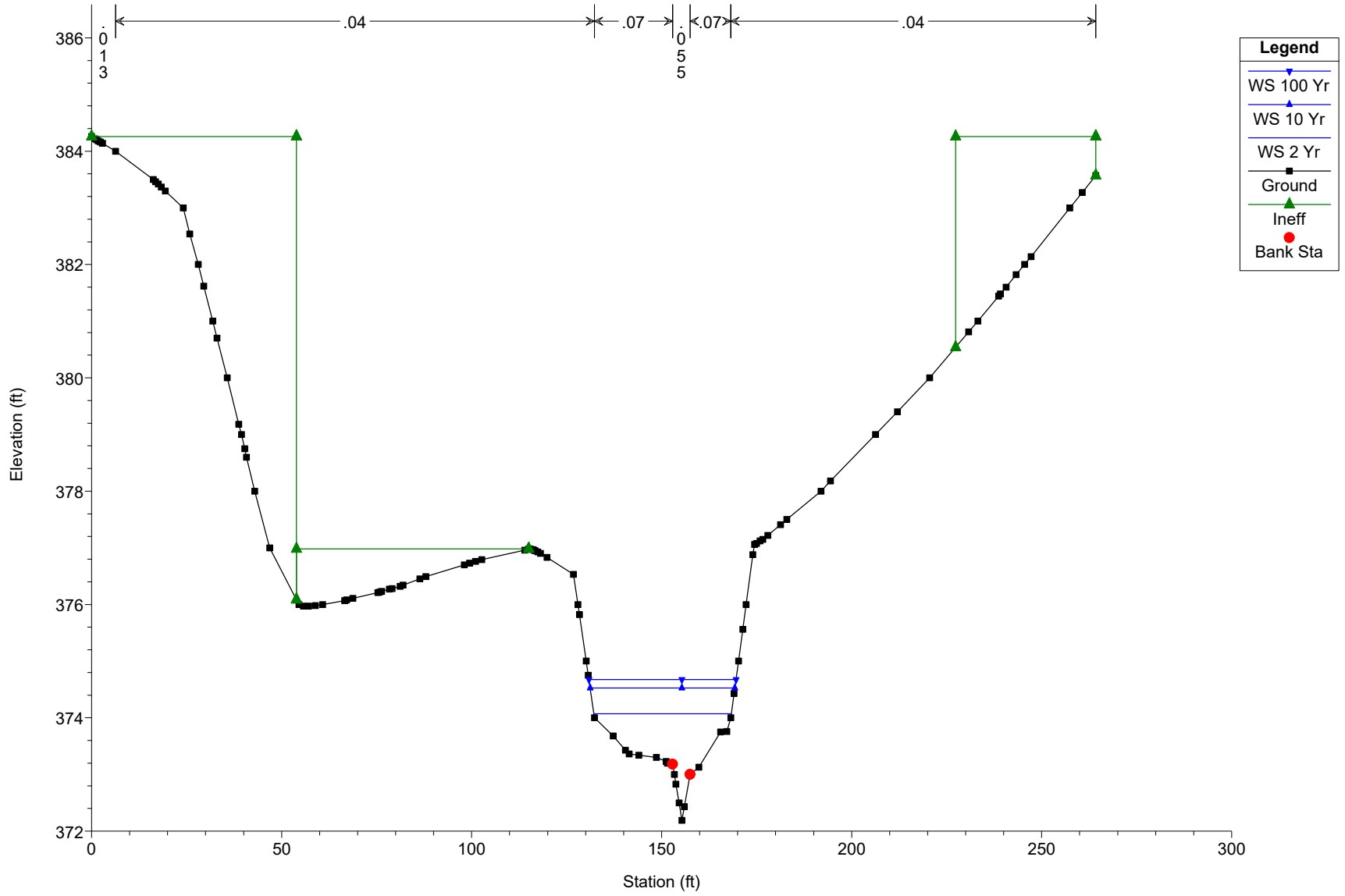
Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = IntsTrib RS = 5





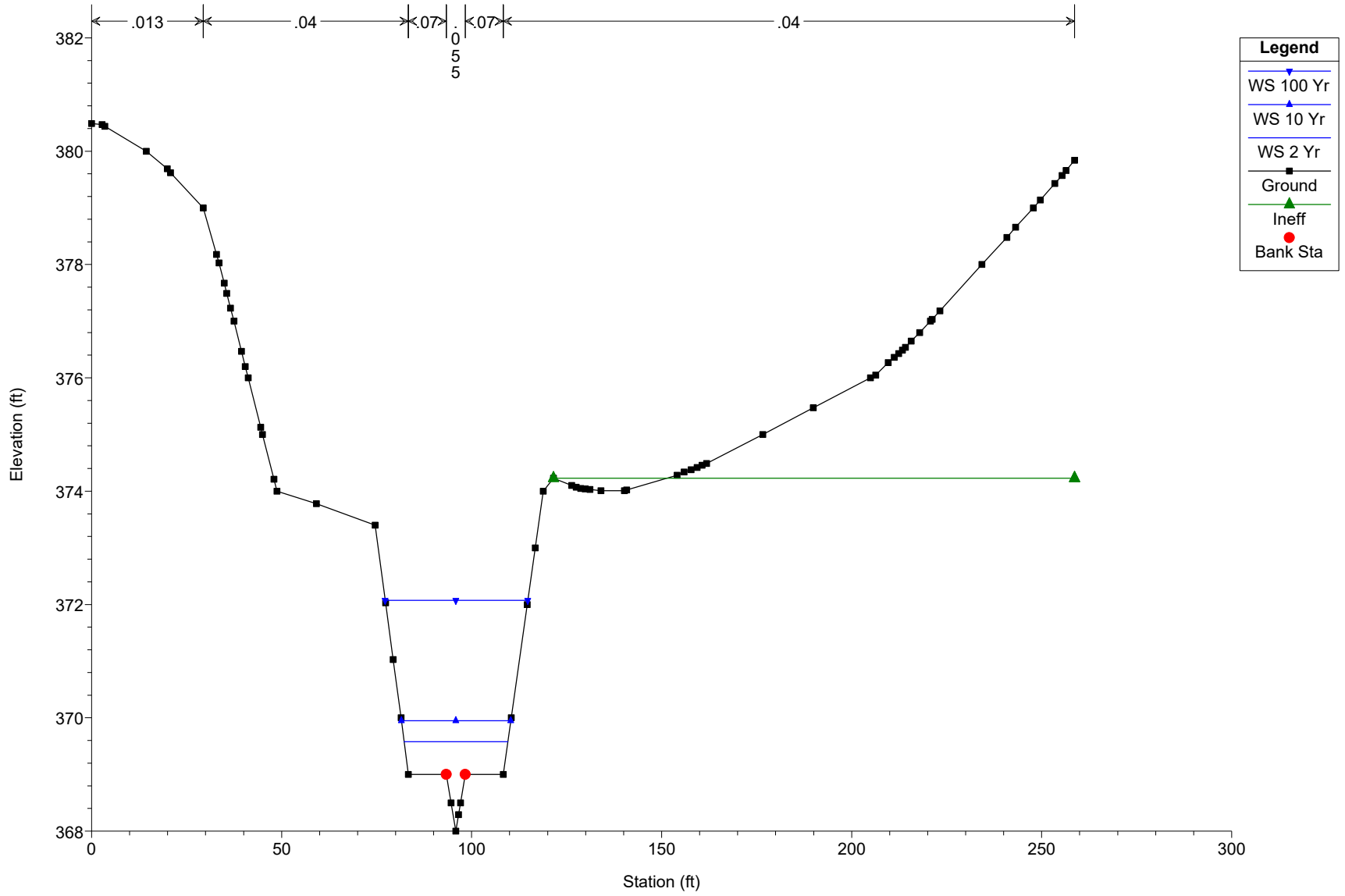
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = IntsTrib RS = 4.5



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

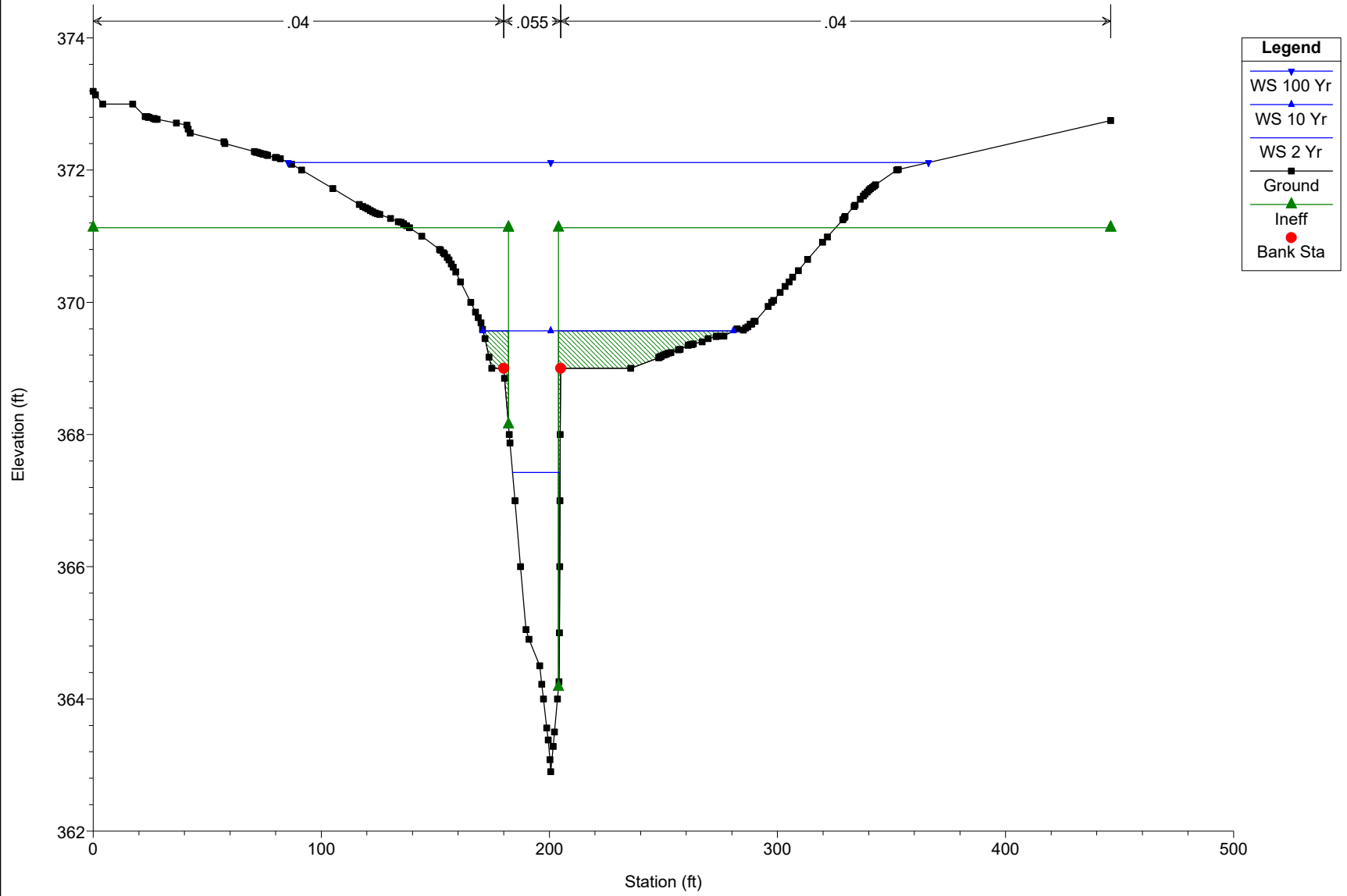
Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = IntsTrib RS = 4



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 3



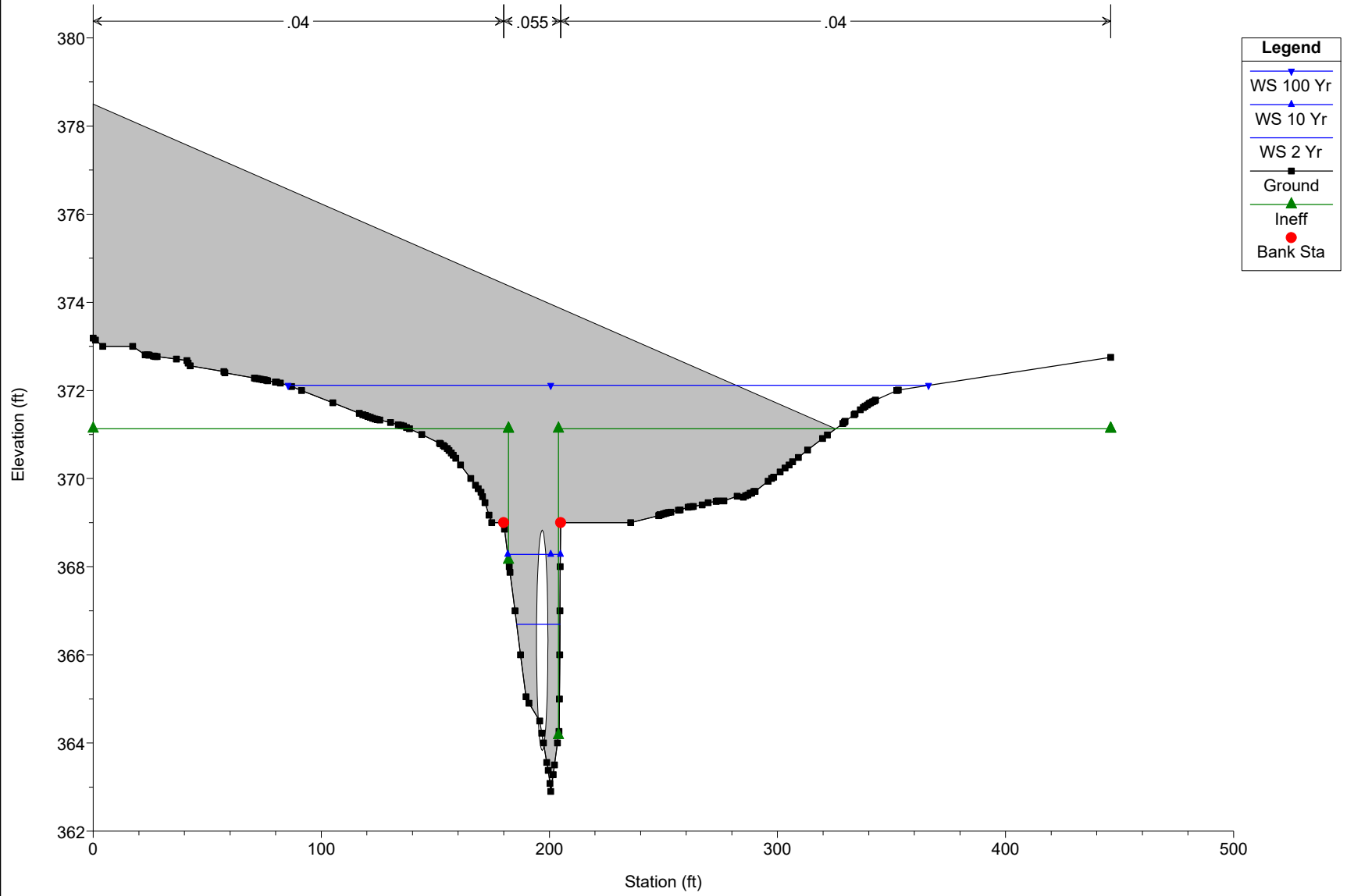
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Ineff
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 2.5 Culv Intersection Trib Culvert under Park Heights Ave.

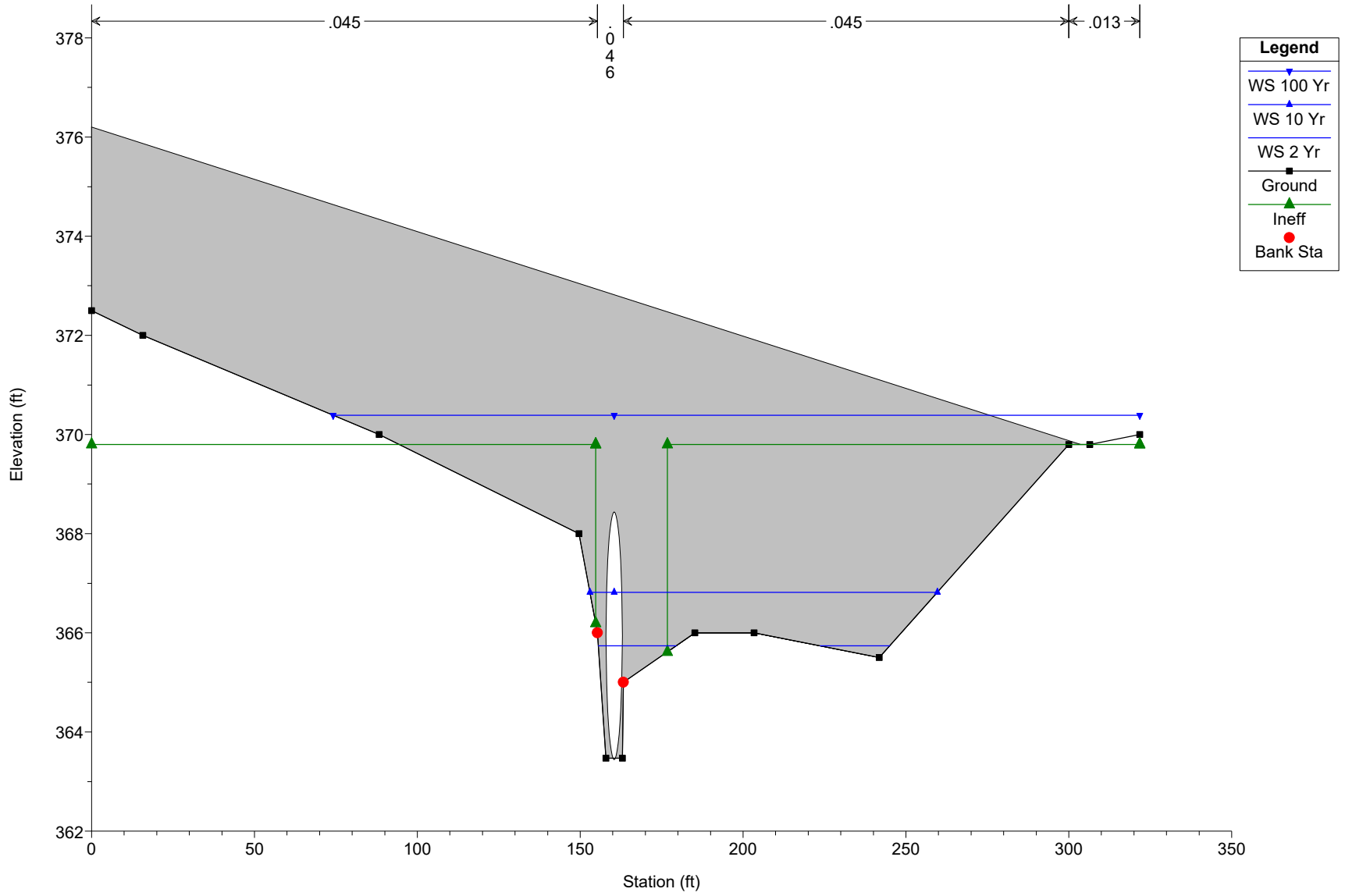


Legend	
WS 100 Yr	Blue line with inverted triangle
WS 10 Yr	Blue line with triangle
WS 2 Yr	Blue line with triangle
Ground	Black line with square
Ineff	Green line with triangle
Bank Sta	Red circle

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

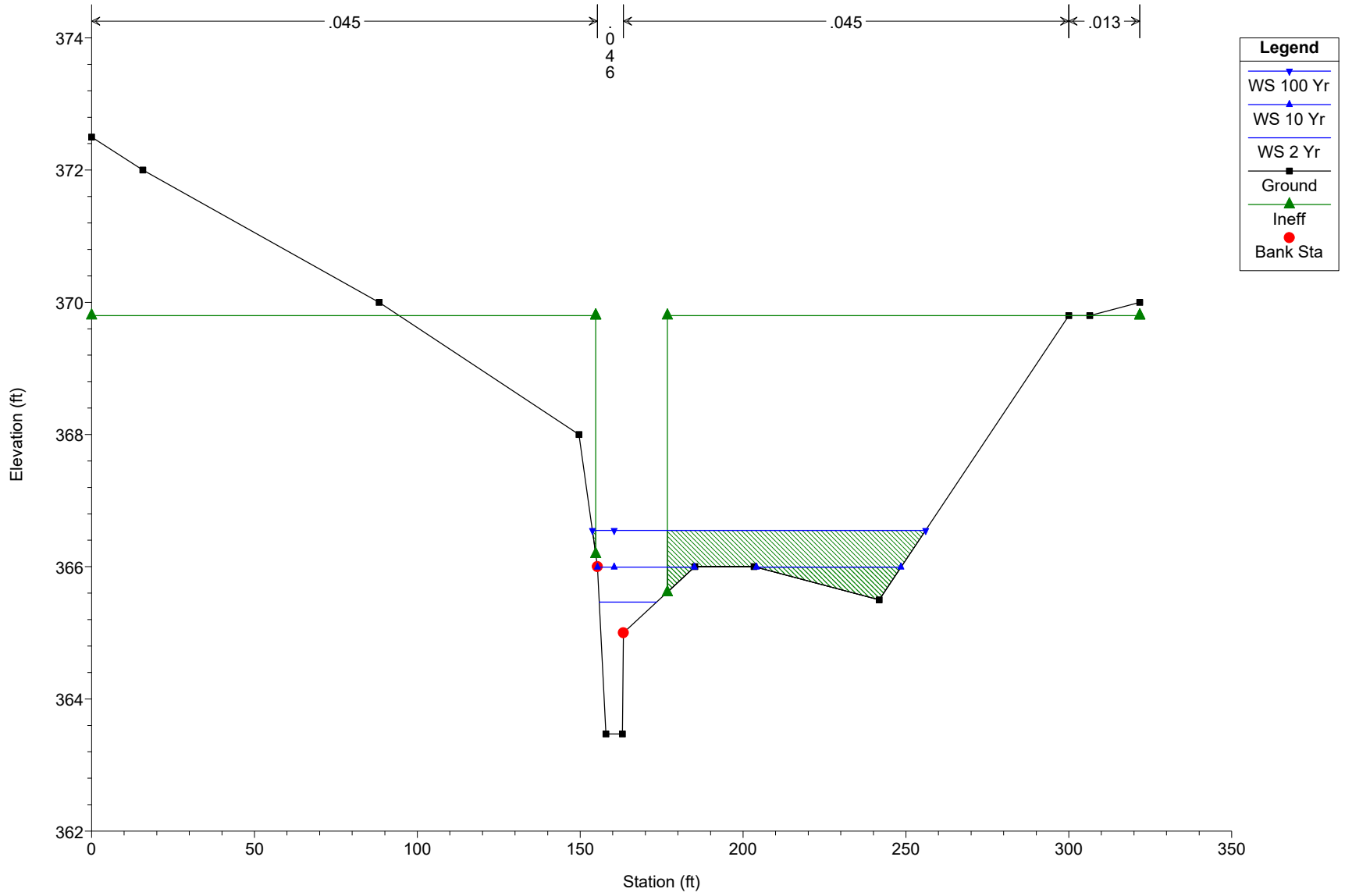
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 2.5 Culv Intersection Trib Culvert under Park Heights Ave.



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = IntsTrib RS = 2

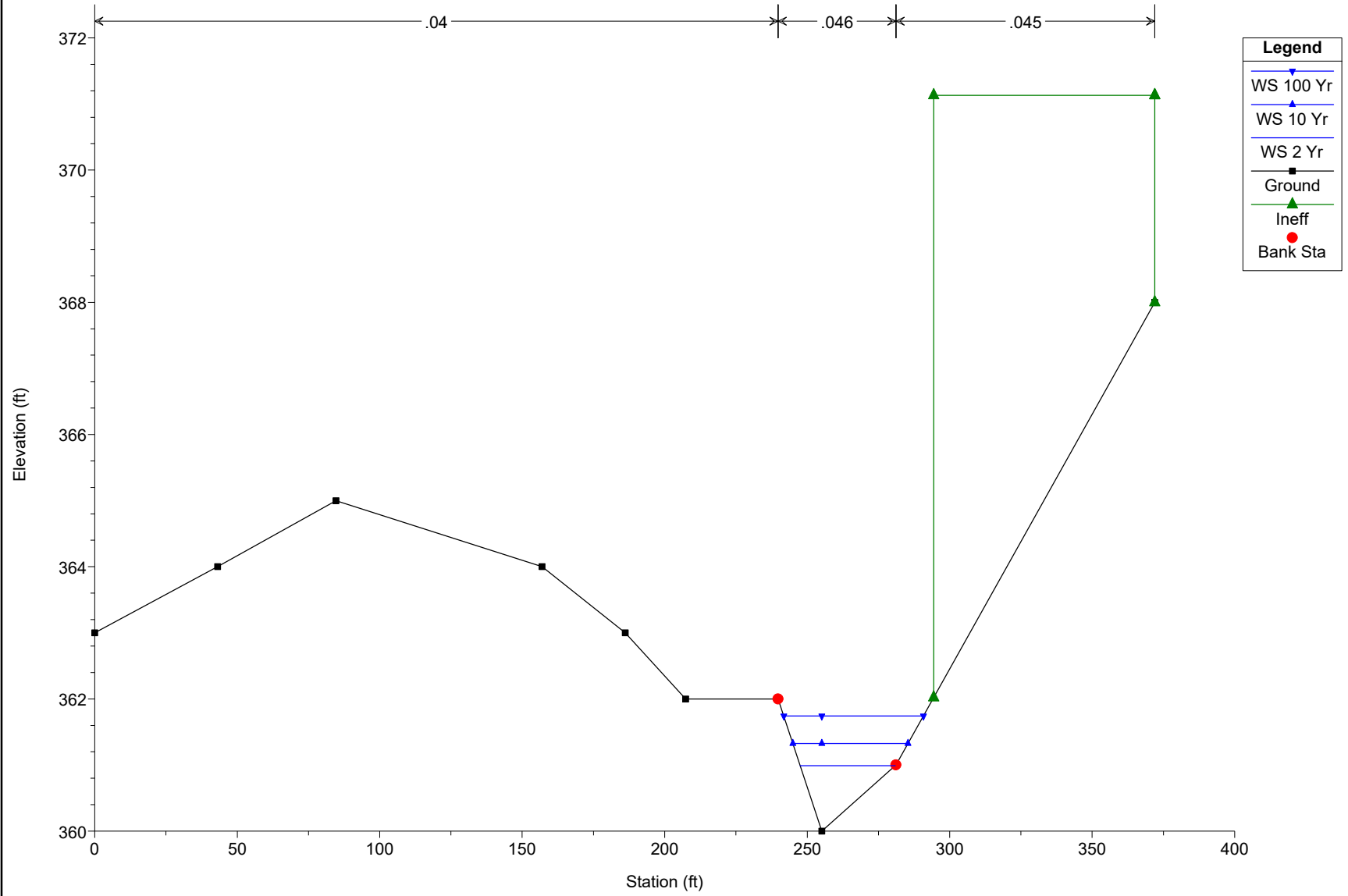


Legend	
WS 100 Yr	Blue line with downward triangle
WS 10 Yr	Blue line with upward triangle
WS 2 Yr	Blue line with square
Ground	Black line with square
Ineff	Green line with upward triangle
Bank Sta	Red dot

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = IntsTrib RS = 1

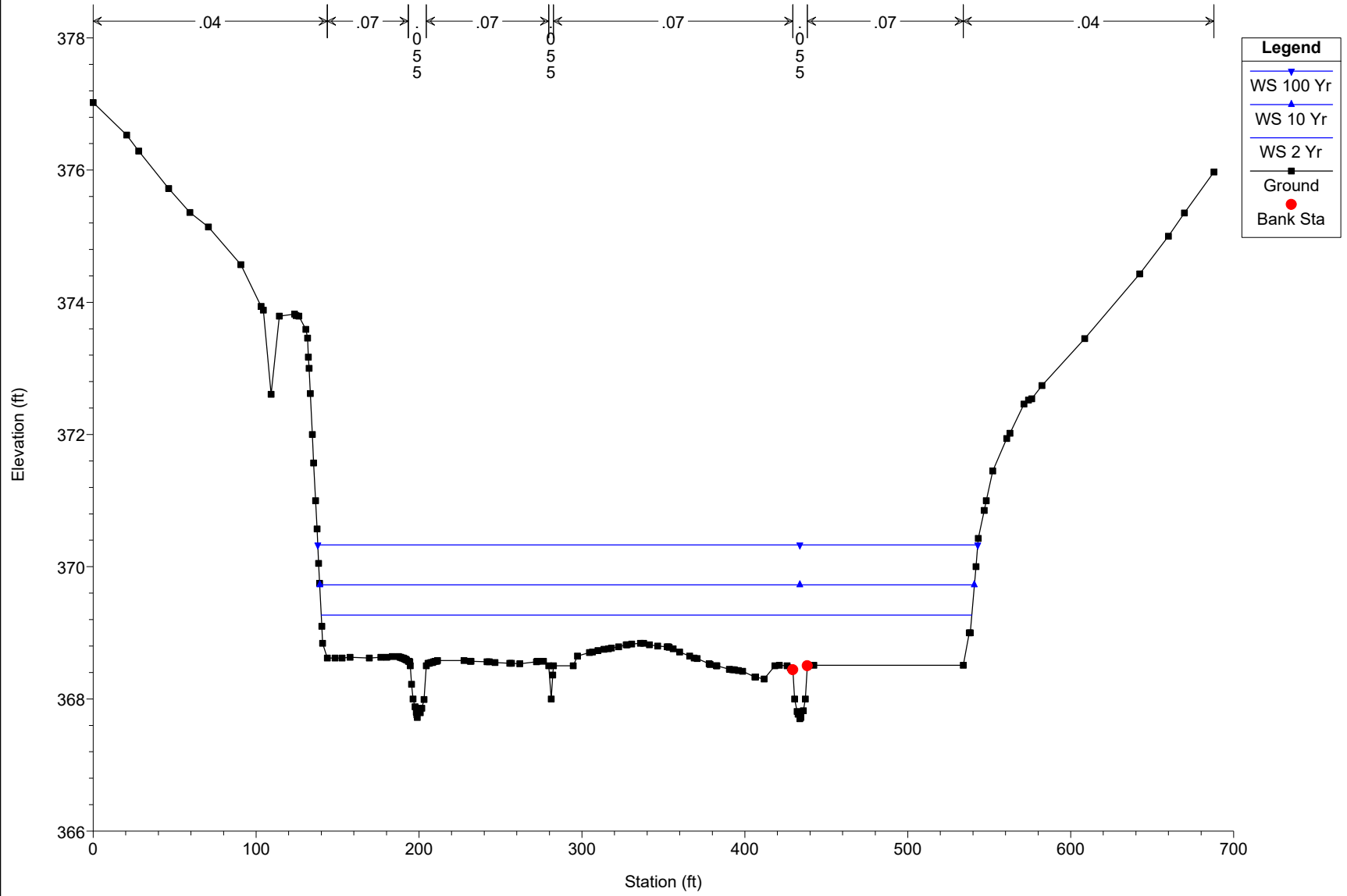


Legend	
WS 100 Yr	Blue line with downward triangles
WS 10 Yr	Blue line with upward triangles
WS 2 Yr	Blue line with horizontal dashes
Ground	Black line with square markers
Ineff	Green line with upward triangles
Bank Sta	Red dot

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls 2 RS = 12

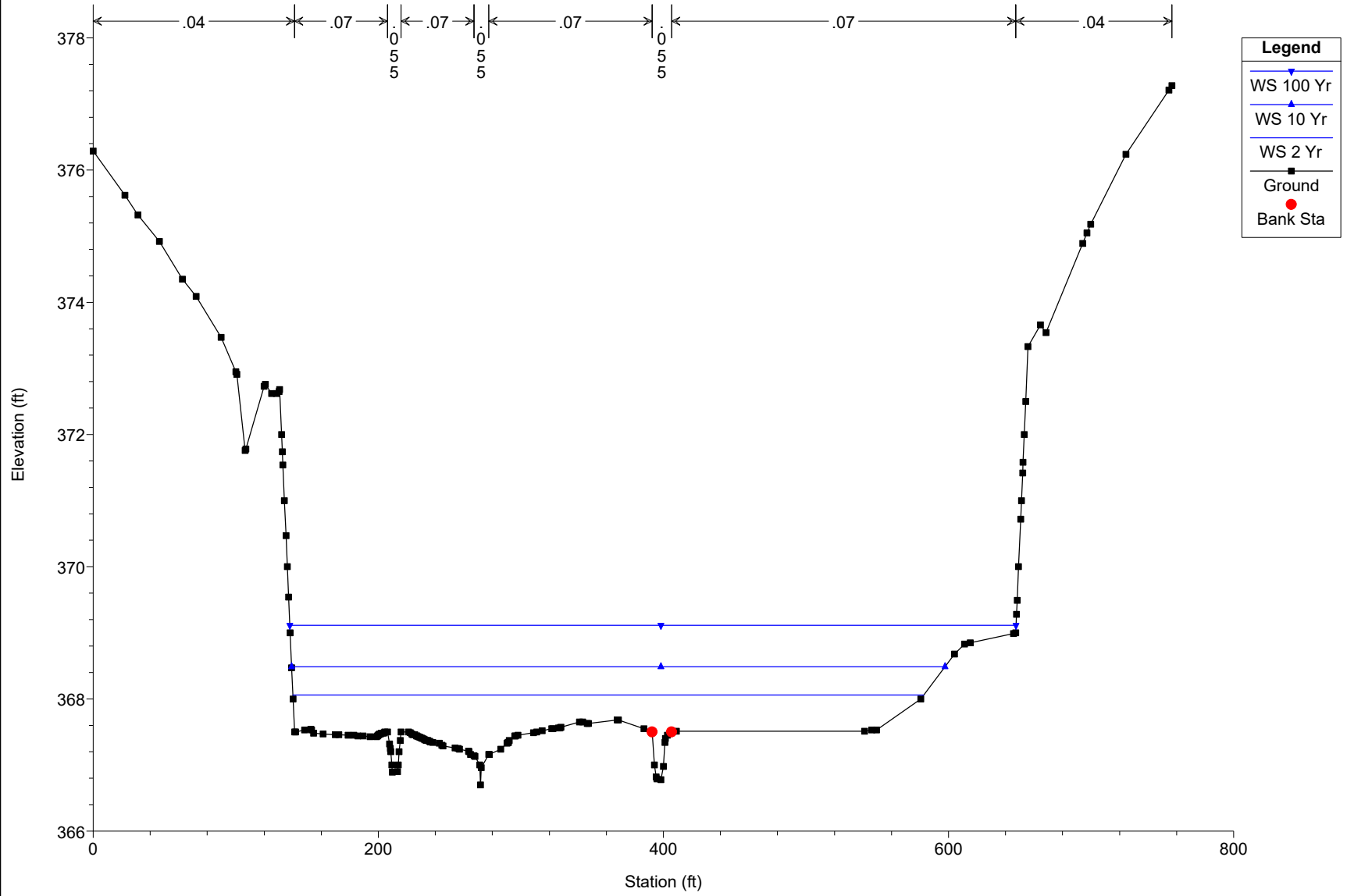




EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

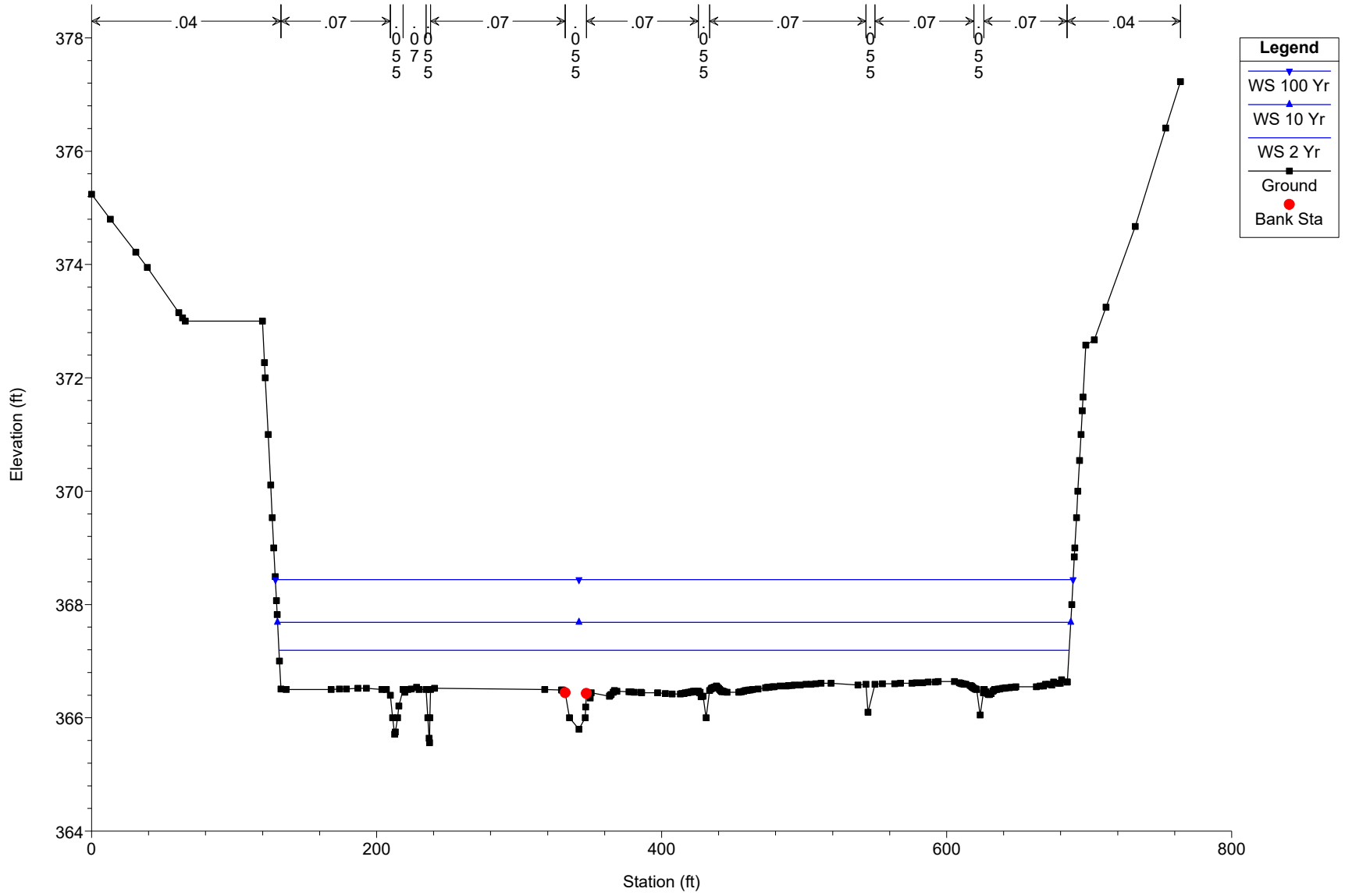
River = Jones Falls Reach = Jones Falls 2 RS = 11.2



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

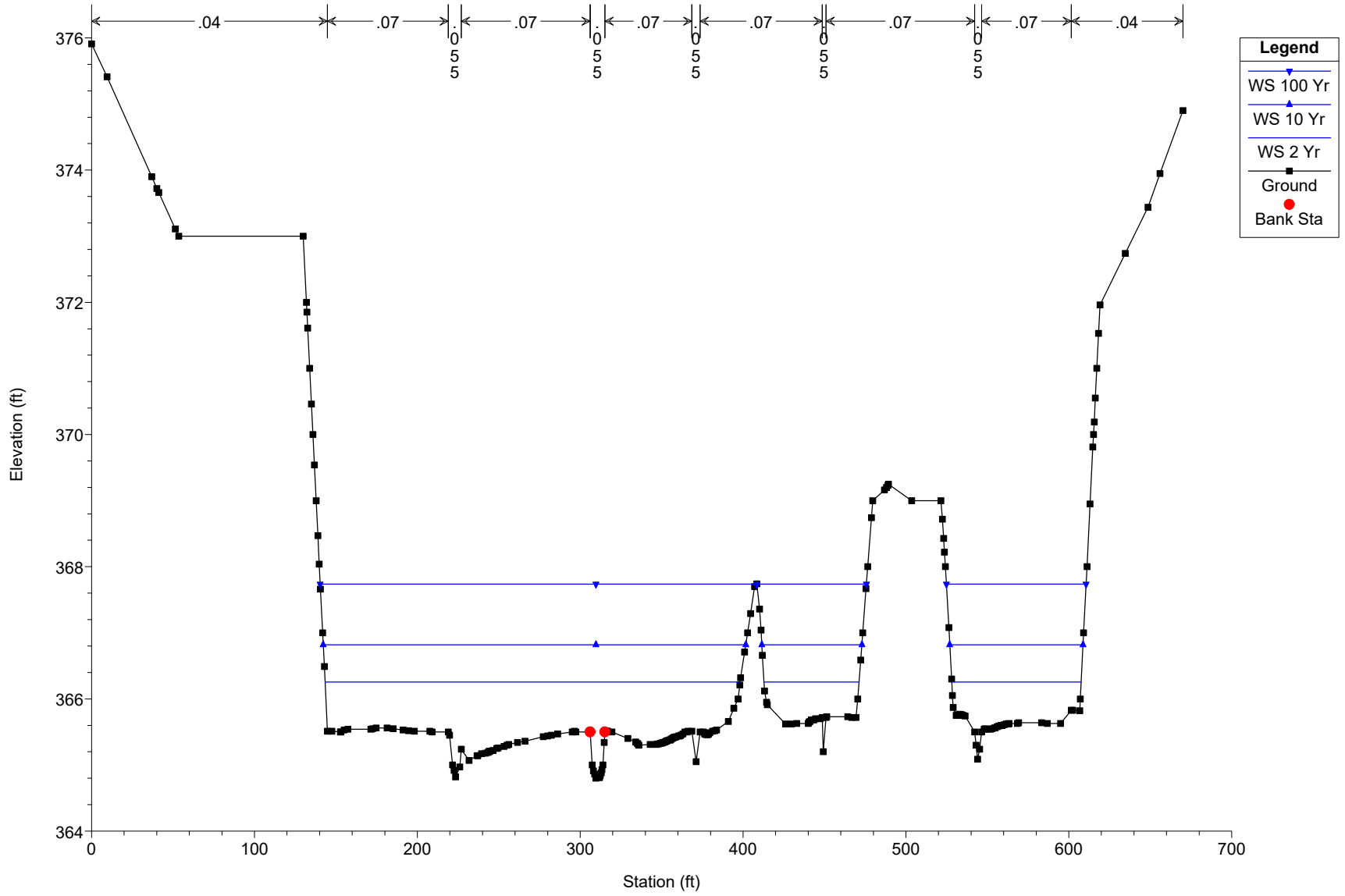
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EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

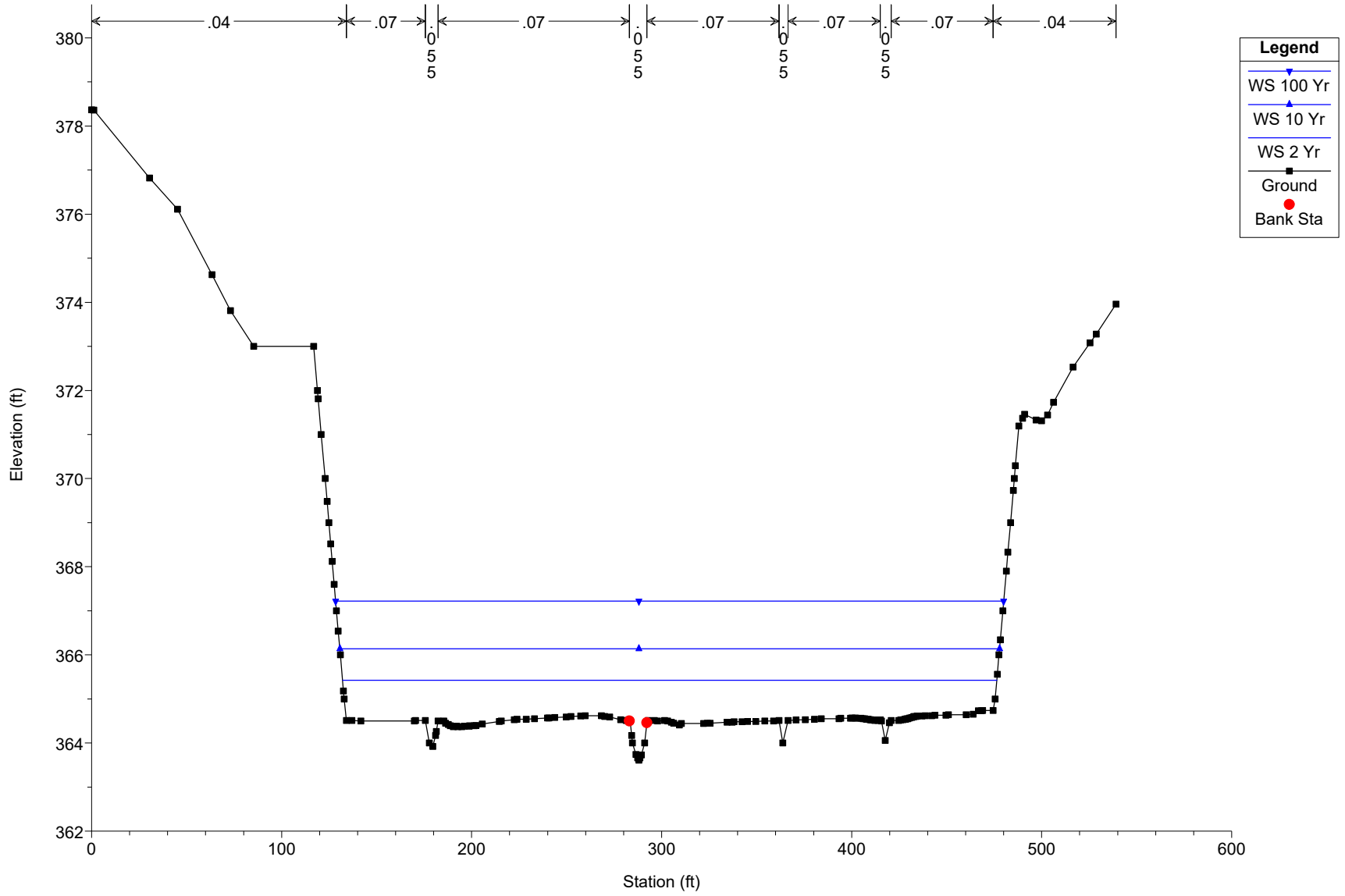
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EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

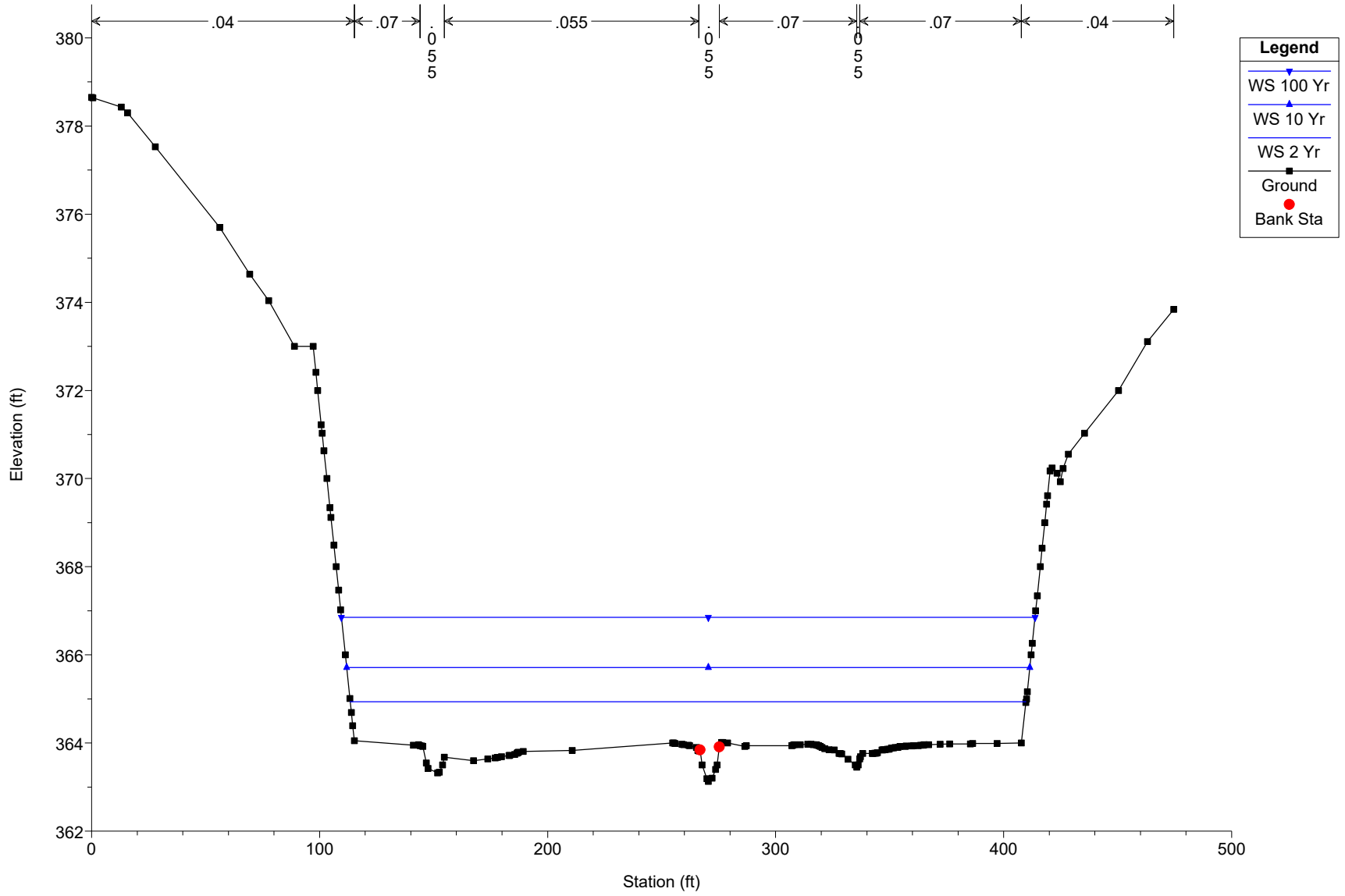
River = Jones Falls Reach = Jones Falls 3 RS = 10.5



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

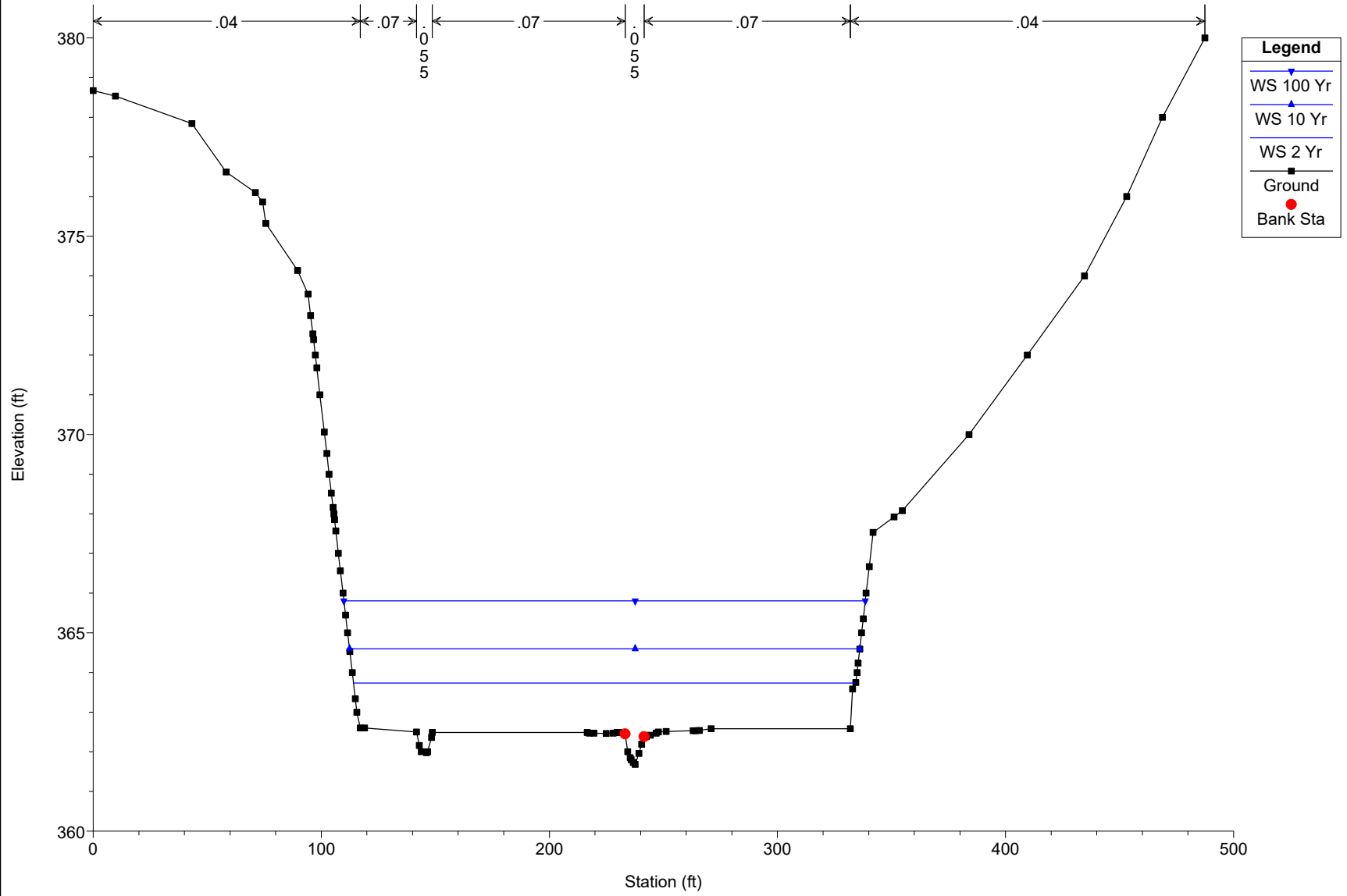
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls 3 RS = 10



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls 3 RS = 9



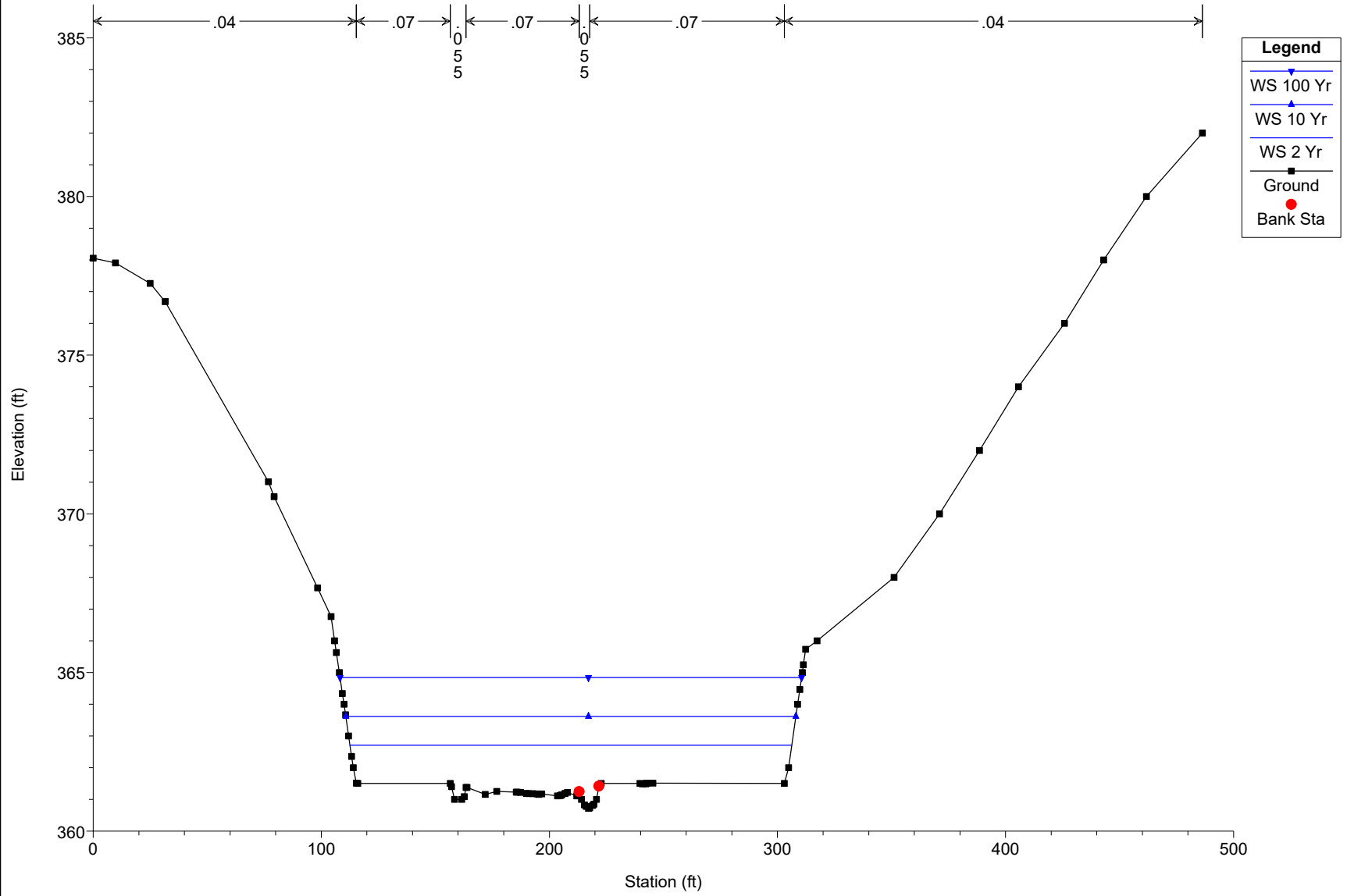
**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls 3 RS = 8

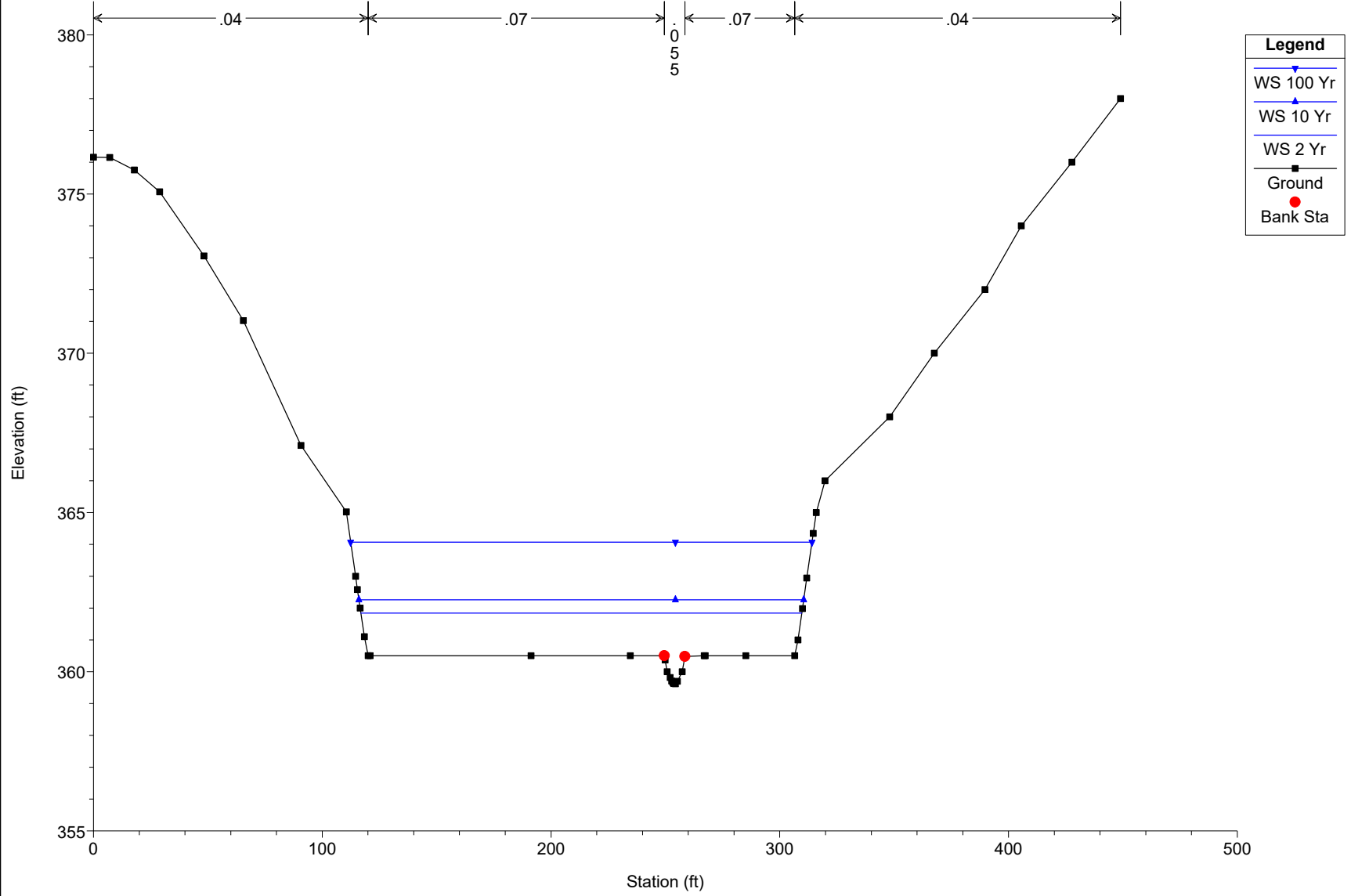


**Legend**

- WS 100 Yr
- WS 10 Yr
- WS 2 Yr
- Ground
- Bank Sta

EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

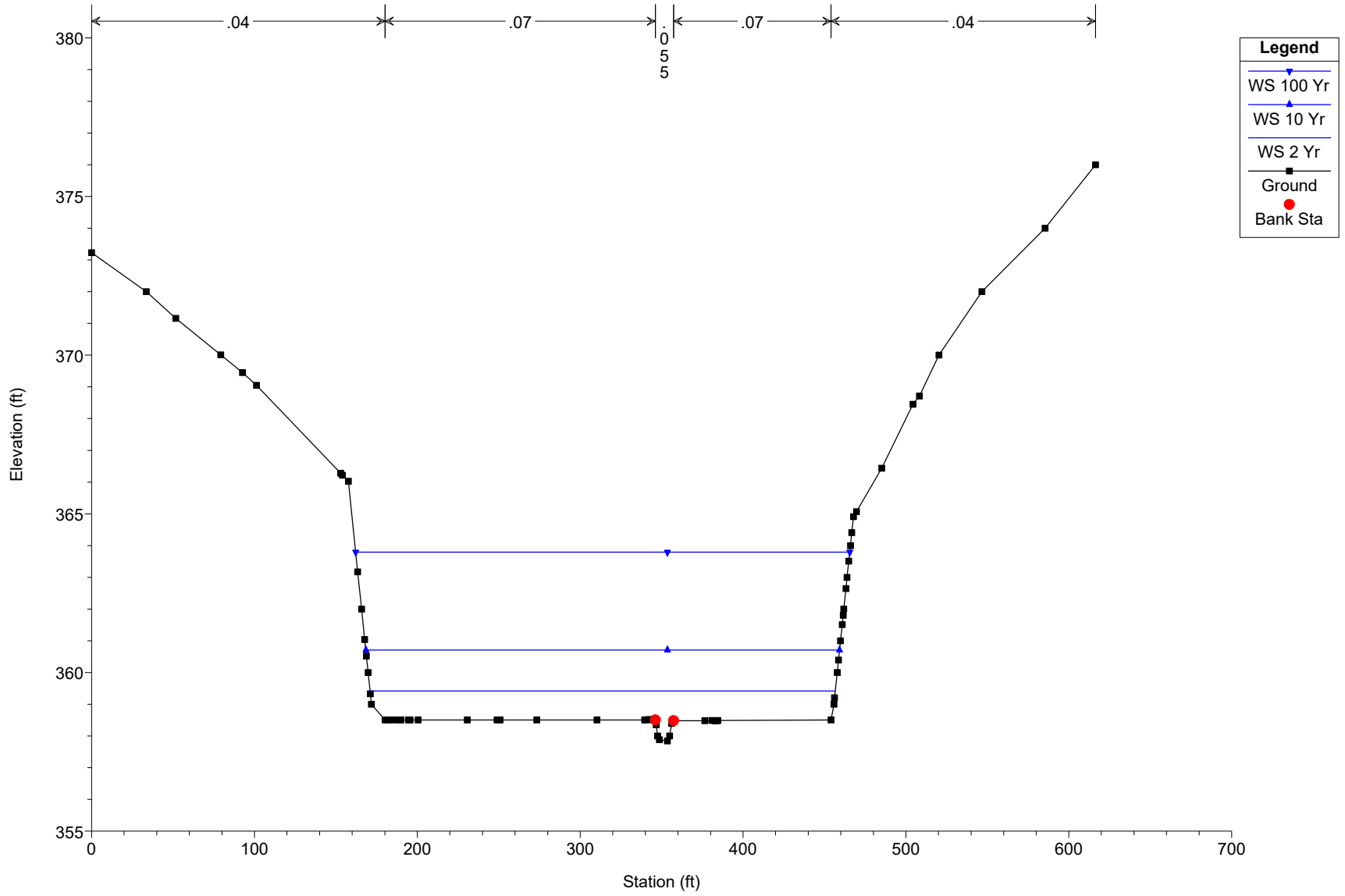
Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls 3 RS = 7.2





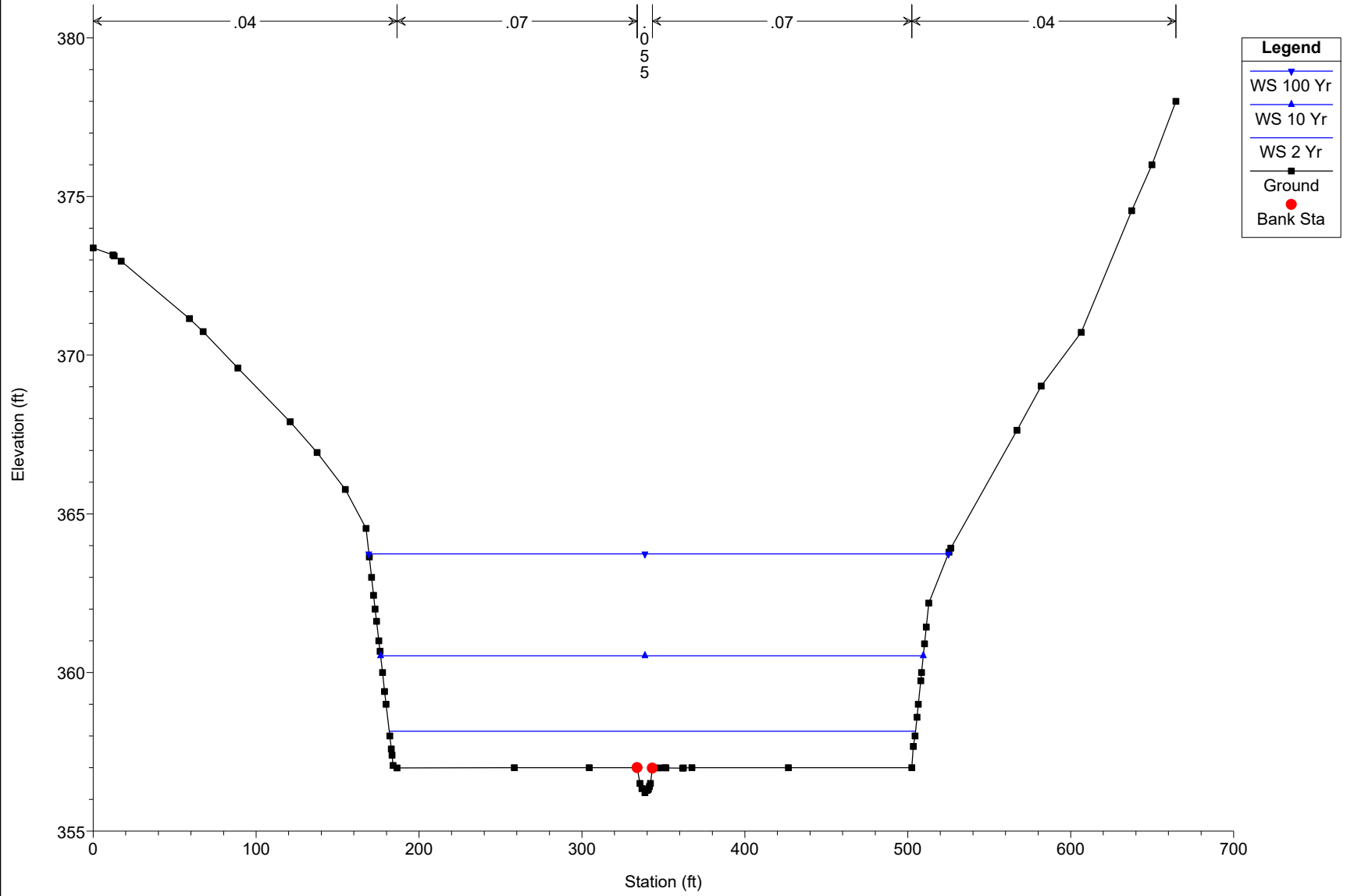
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls 3 RS = 7.1



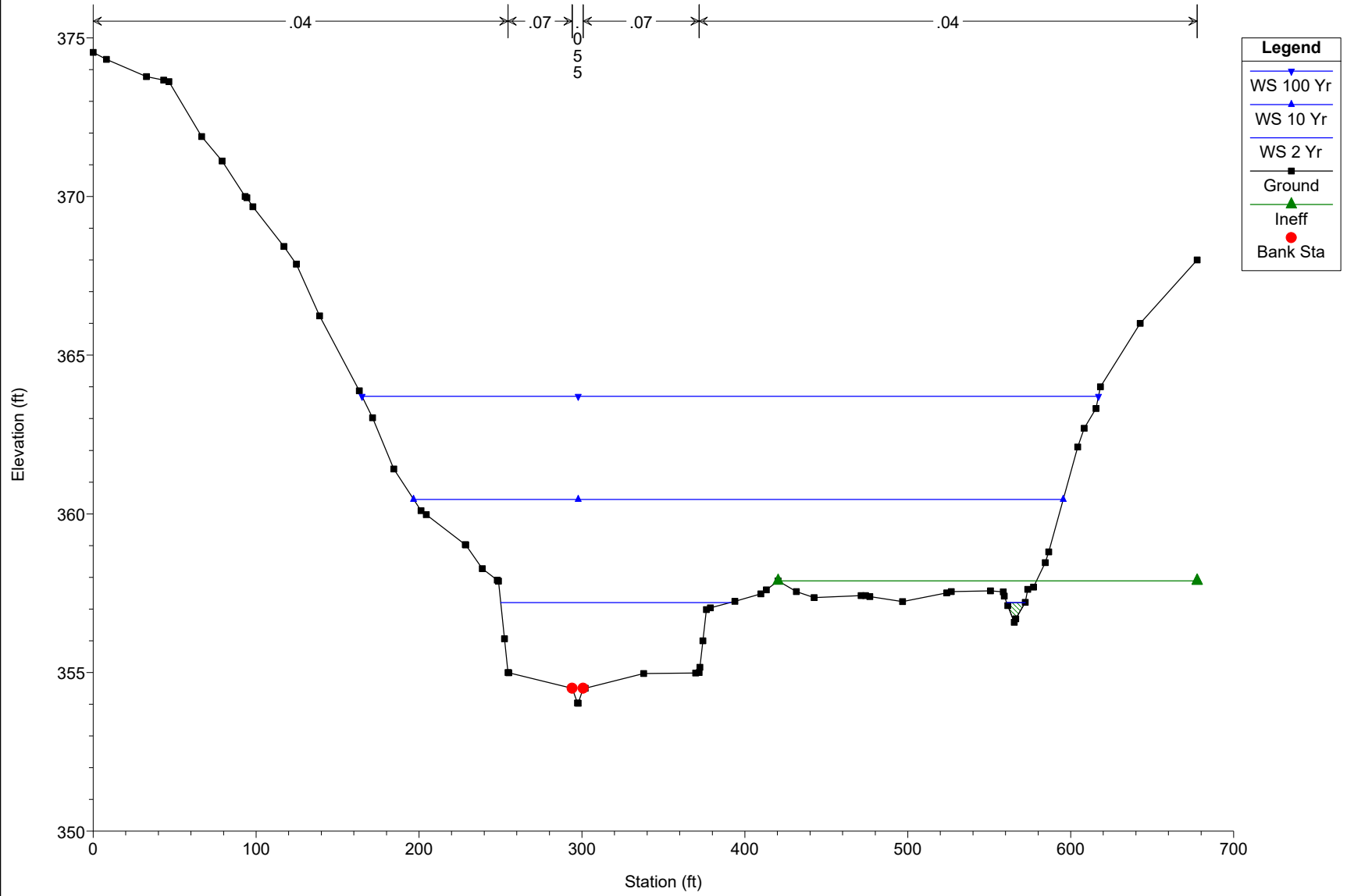
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = Jones Falls 3 RS = 7



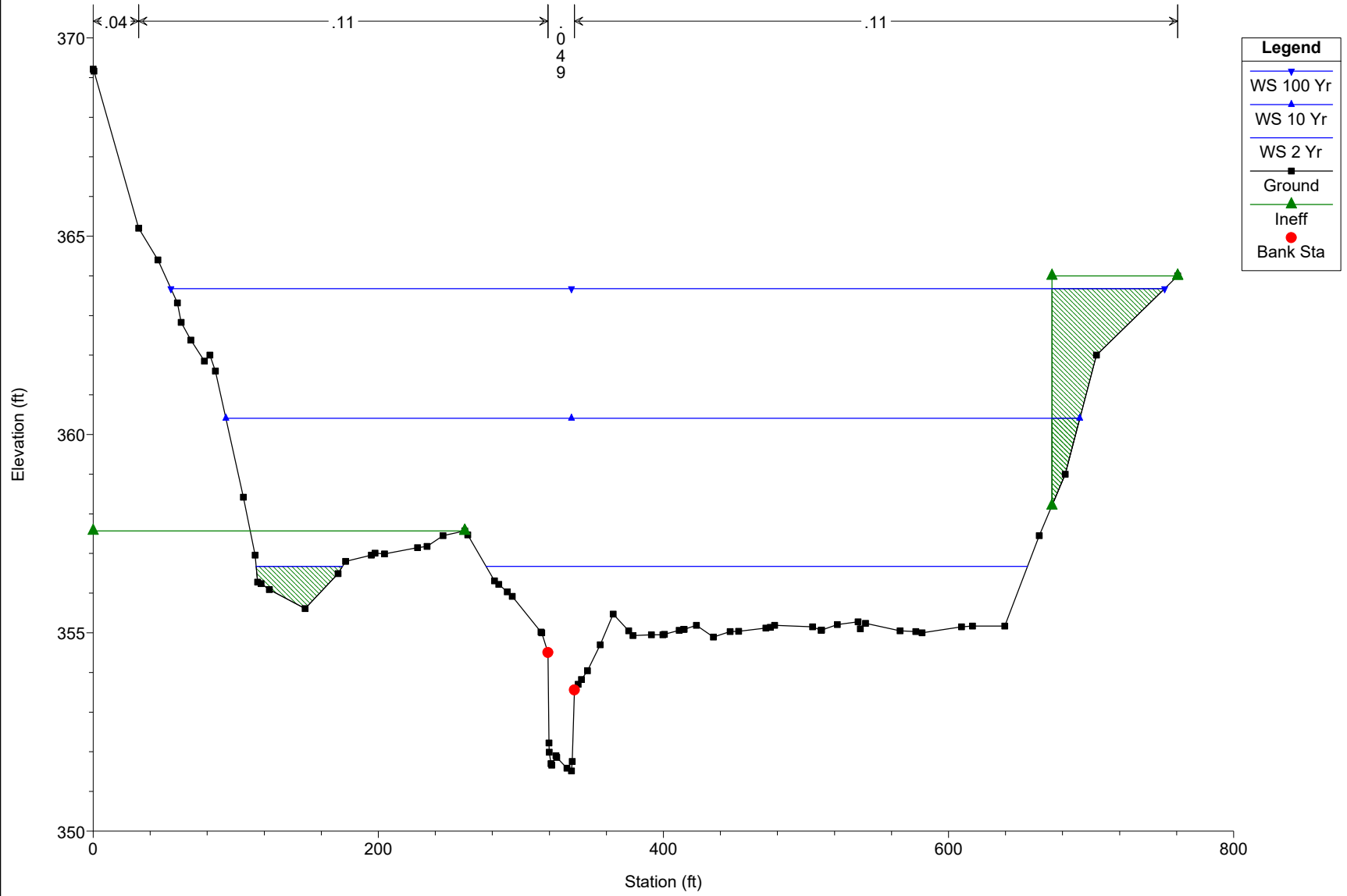
EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
 River = Jones Falls Reach = Jones Falls 3 RS = 6.5



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

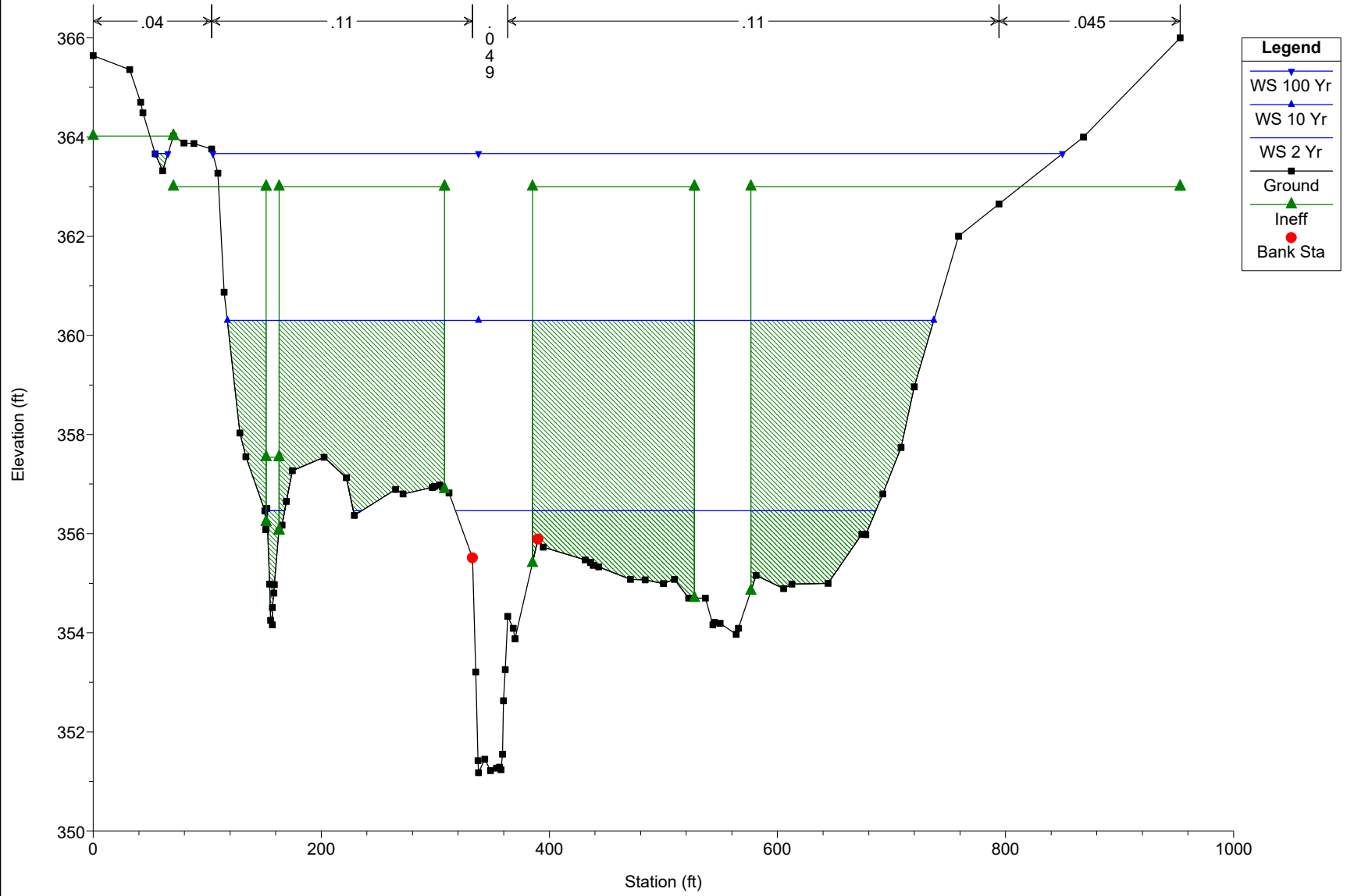
Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls 3 RS = 6



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

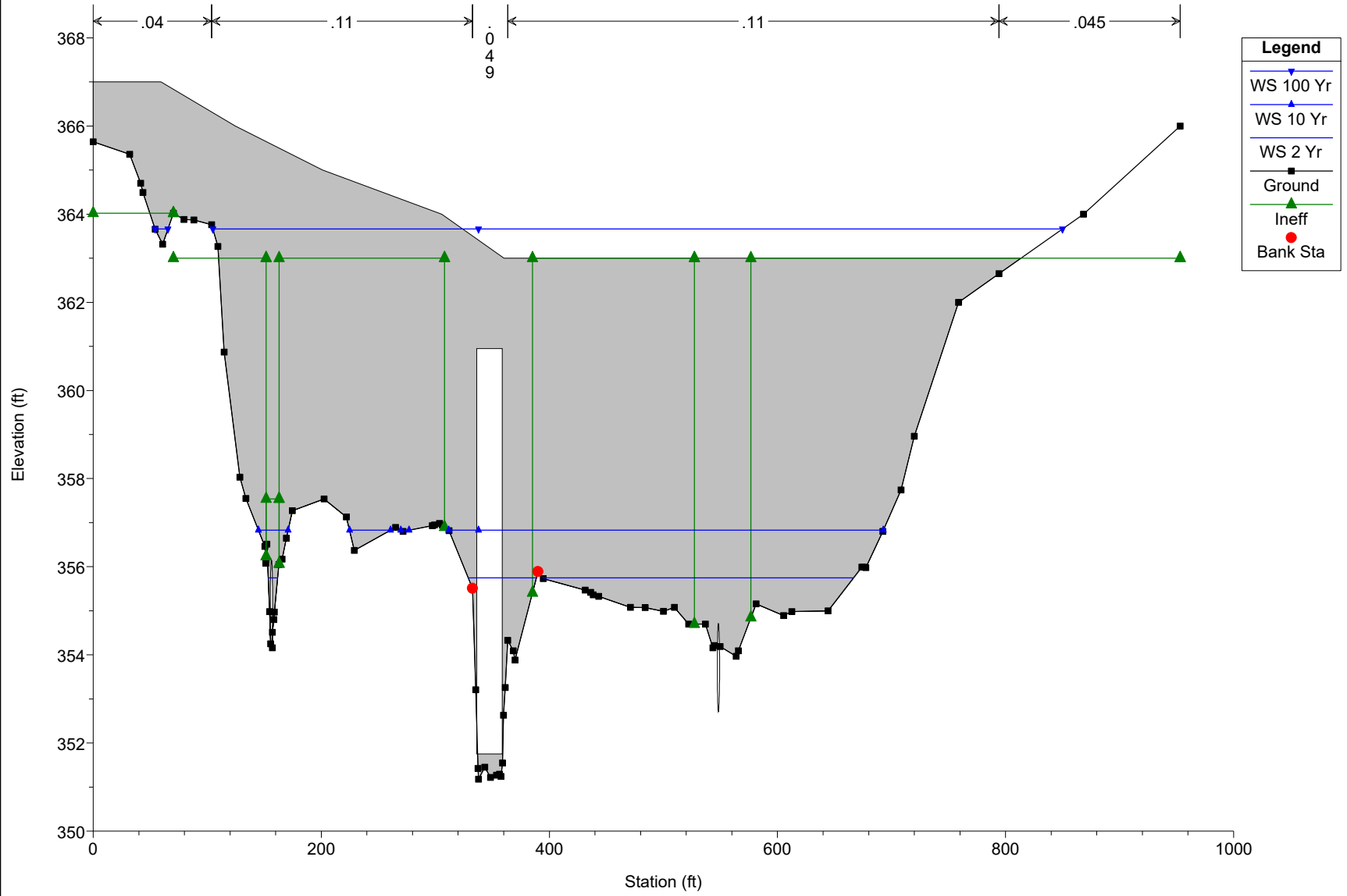
River = Jones Falls Reach = Jones Falls 3 RS = 5



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions

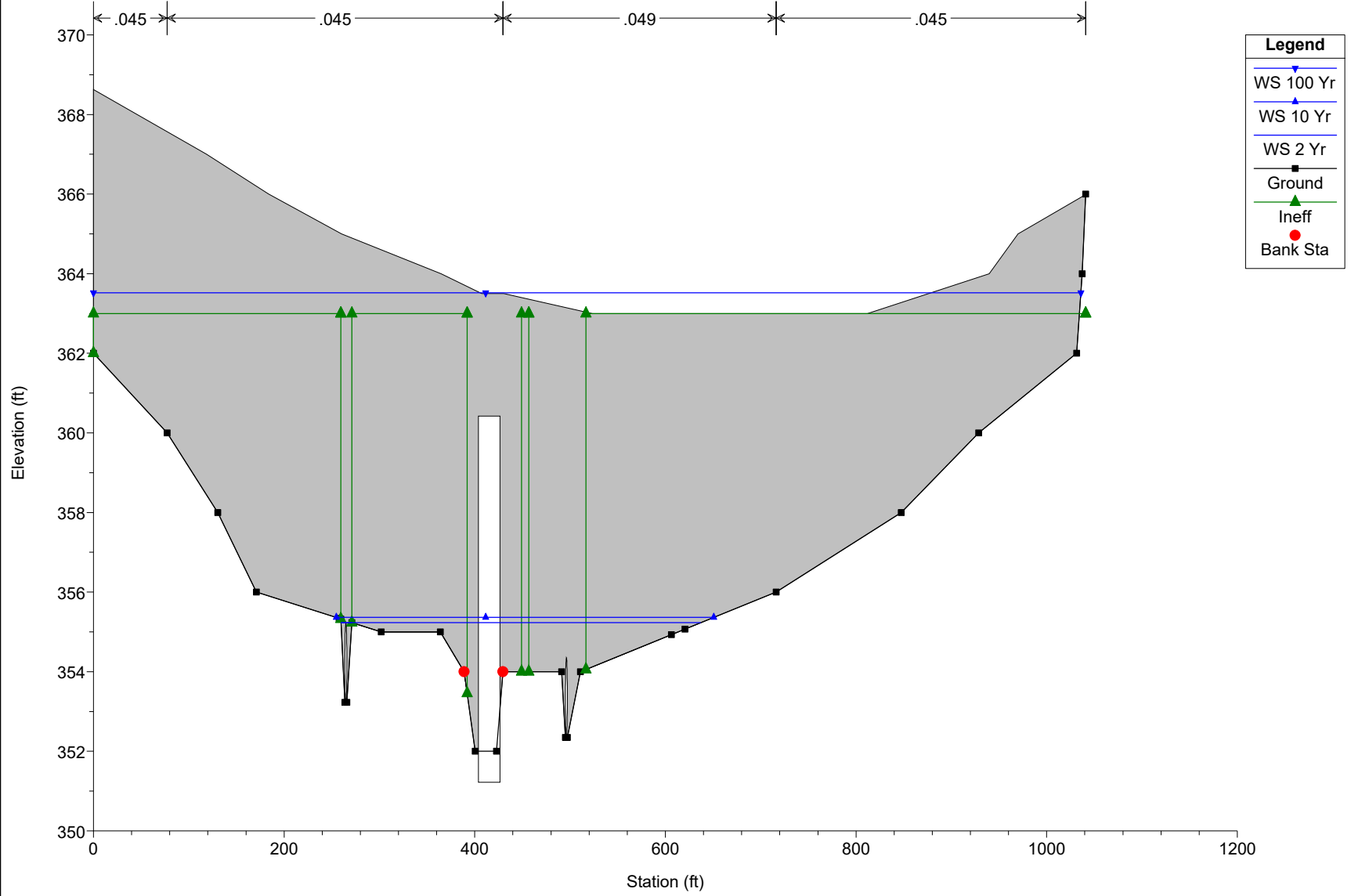
River = Jones Falls Reach = Jones Falls 3 RS = 4.5 Culv Bridge over Jones Falls on Park Heights Ave.



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

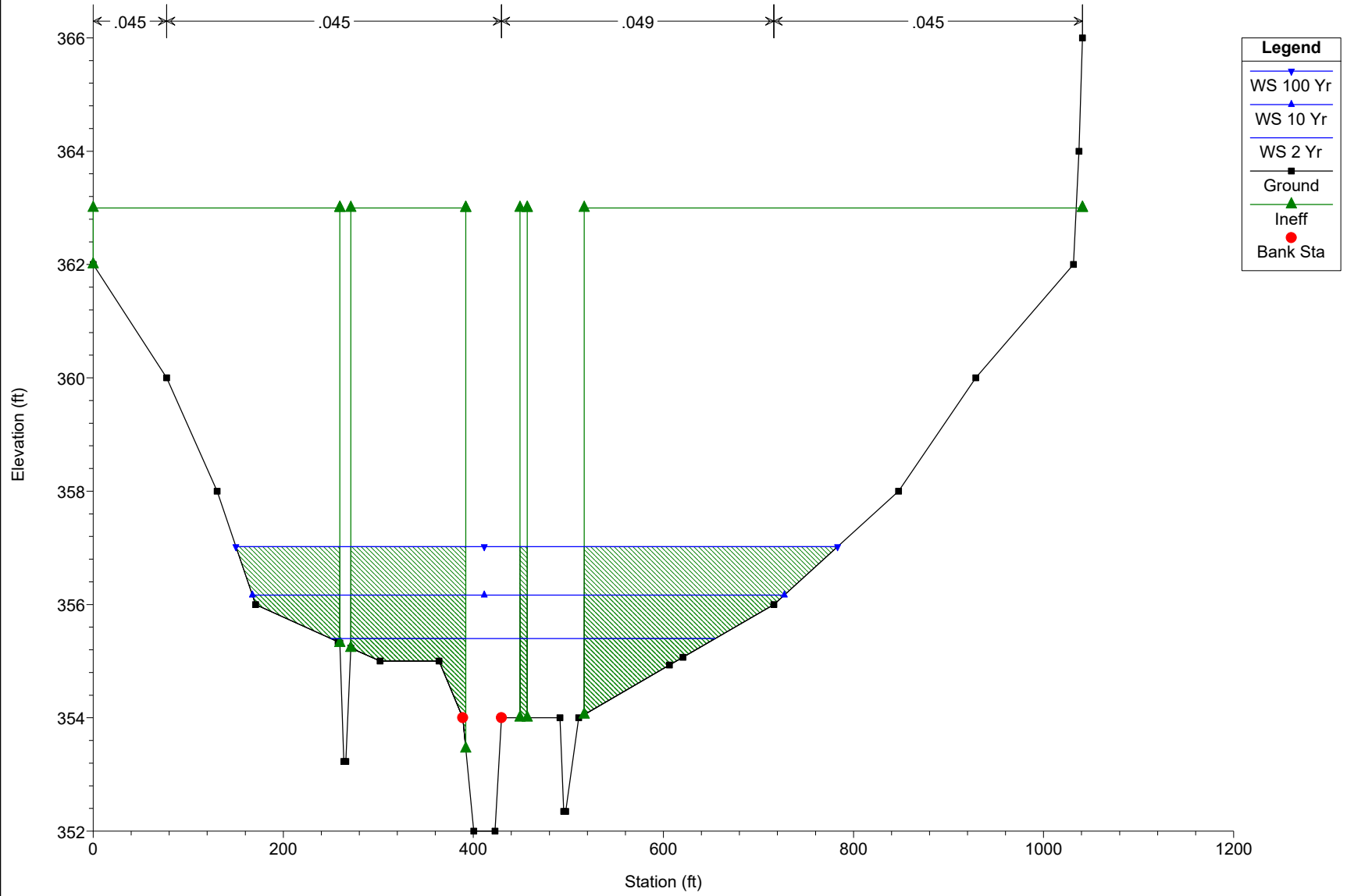
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls 3 RS = 4.5 Culv Bridge over Jones Falls on Park Heights Ave.



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

Geom: EcclestonMod\_Proposed with Junctions  
River = Jones Falls Reach = Jones Falls 3 RS = 4

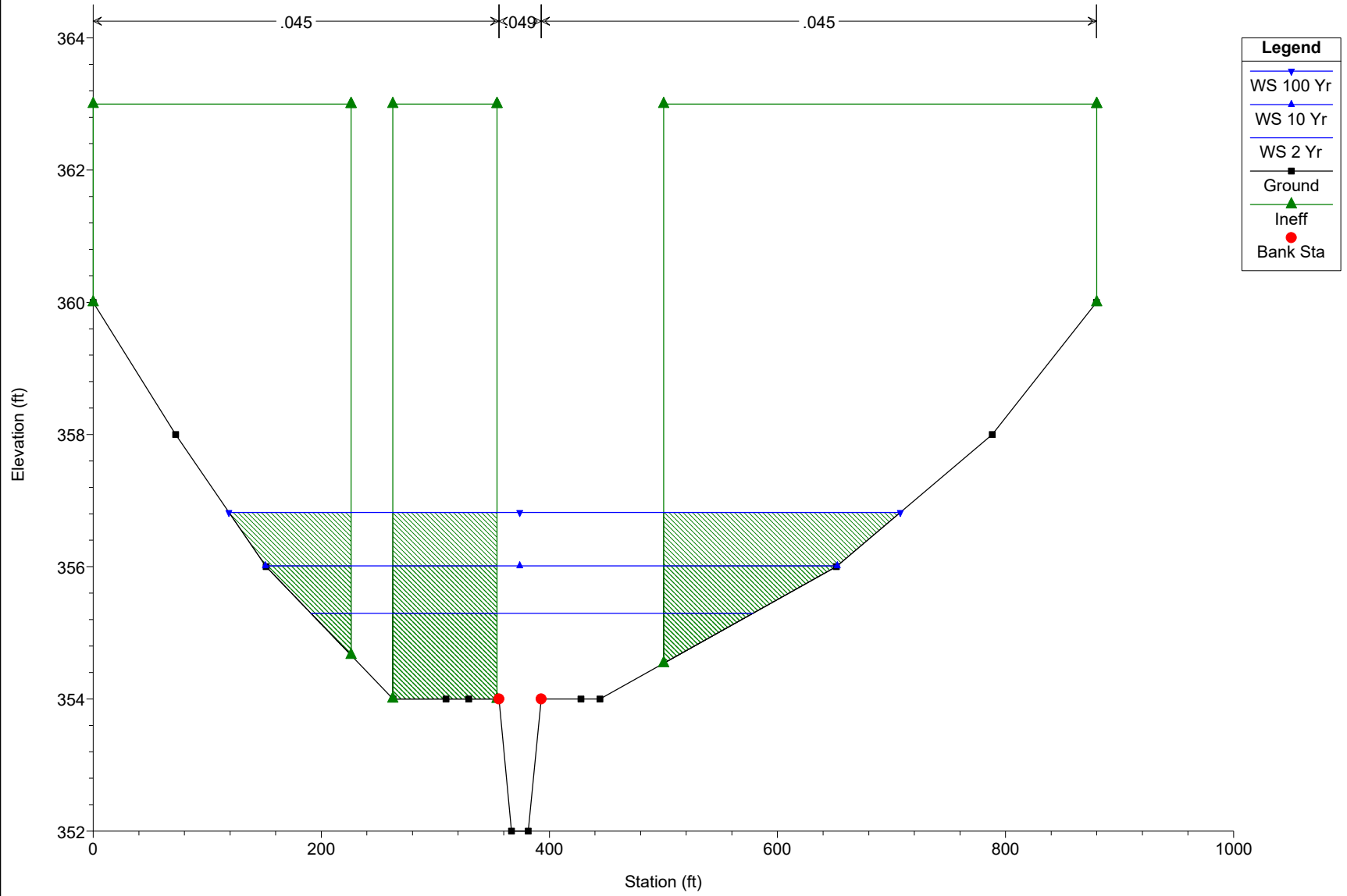




EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

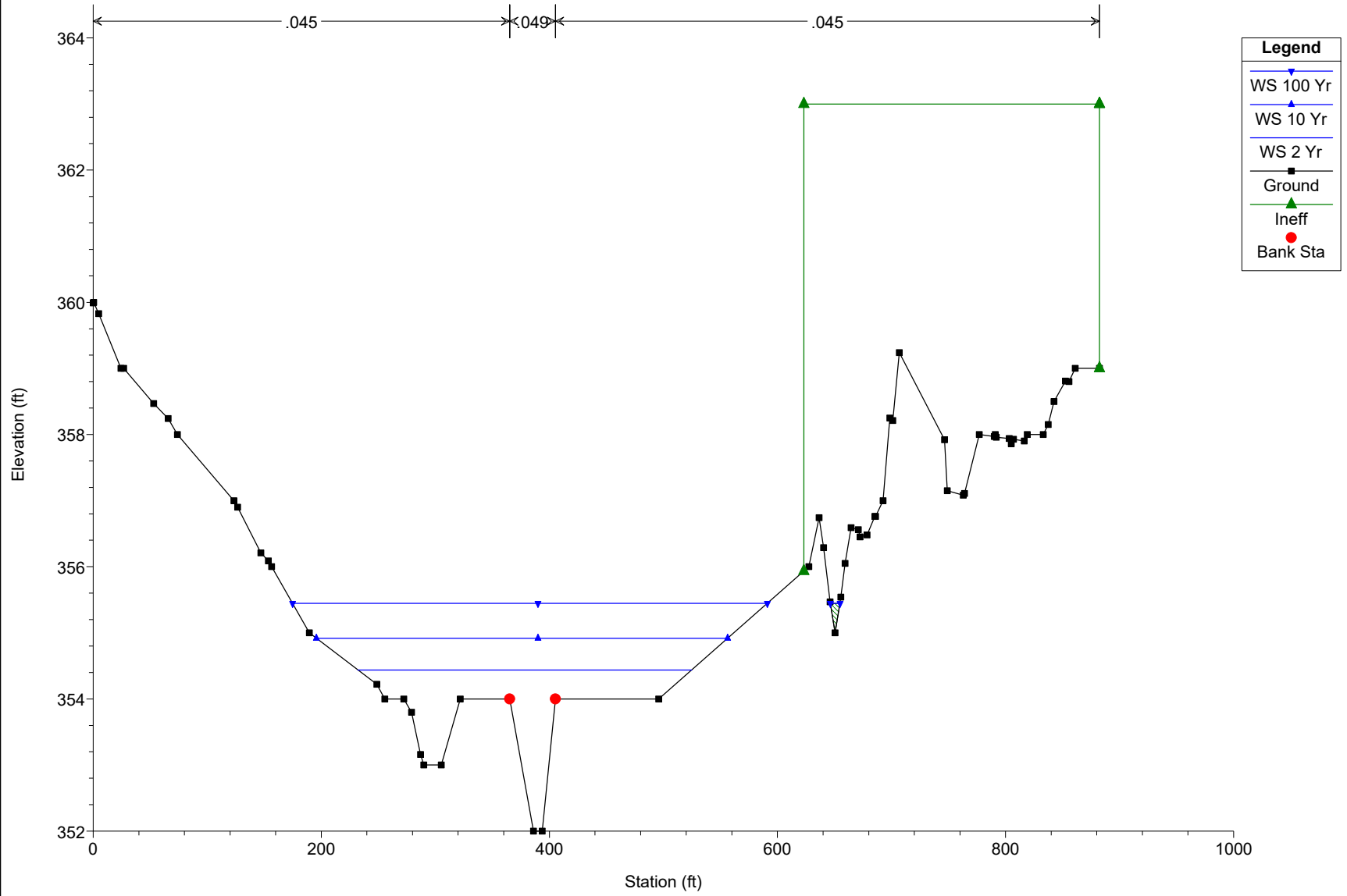
Geom: EcclestonMod\_Proposed with Junctions

River = Jones Falls Reach = Jones Falls 3 RS = 3.5



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

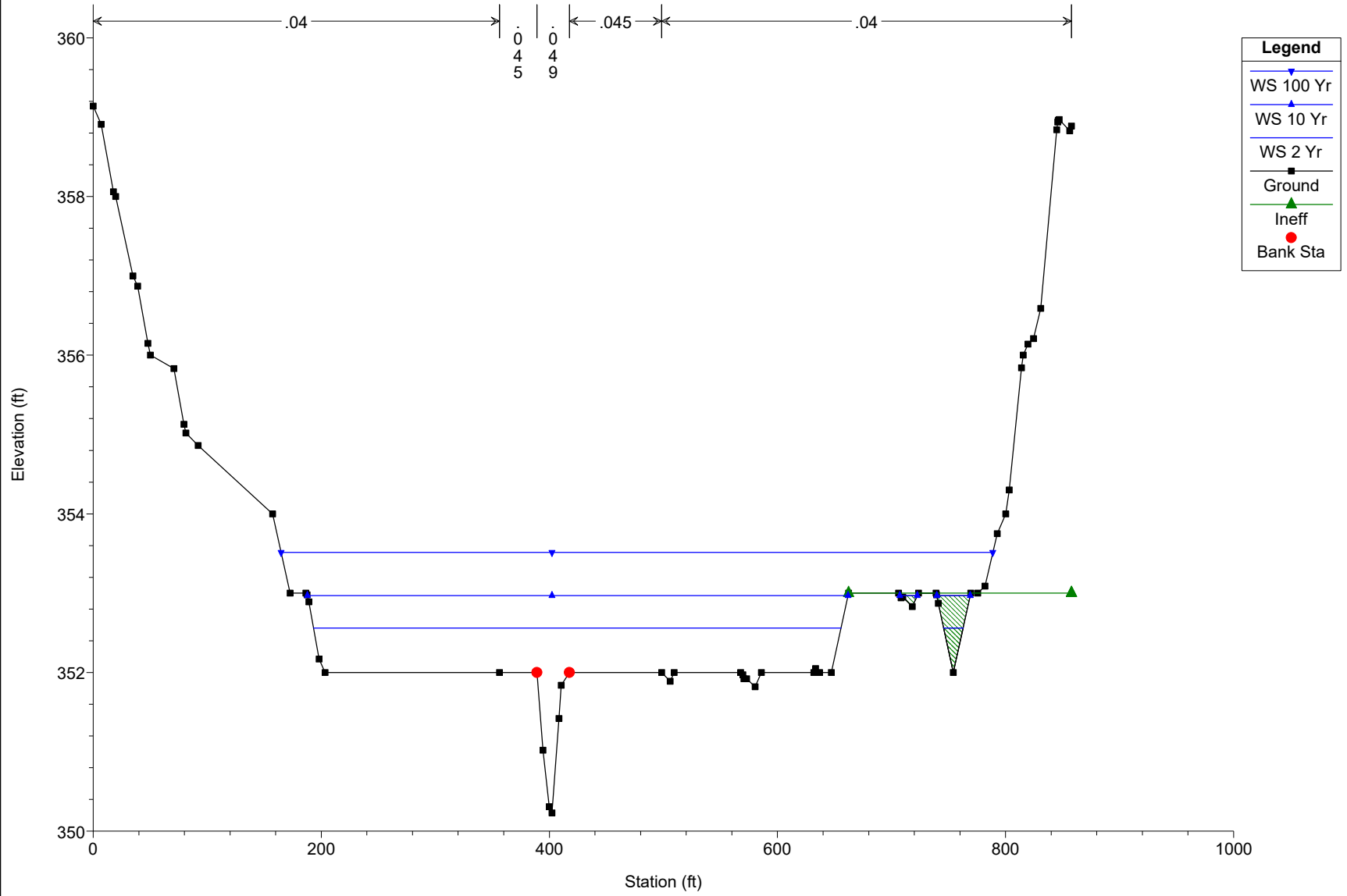
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River = Jones Falls Reach = Jones Falls 3 RS = 3



EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

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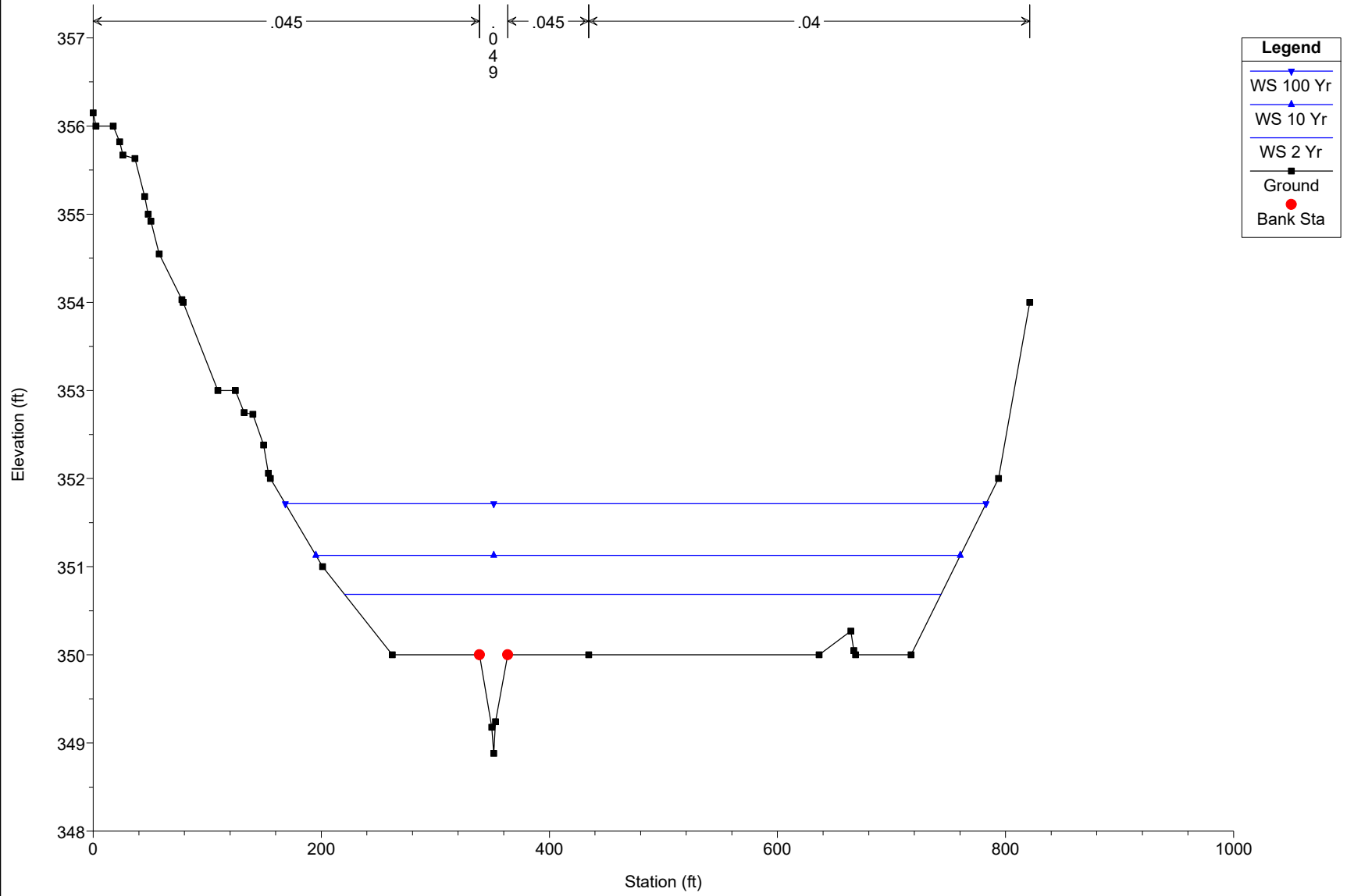
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EcclestonMod\_Proposed Plan: Proposed\_Eccleston 3/18/2021

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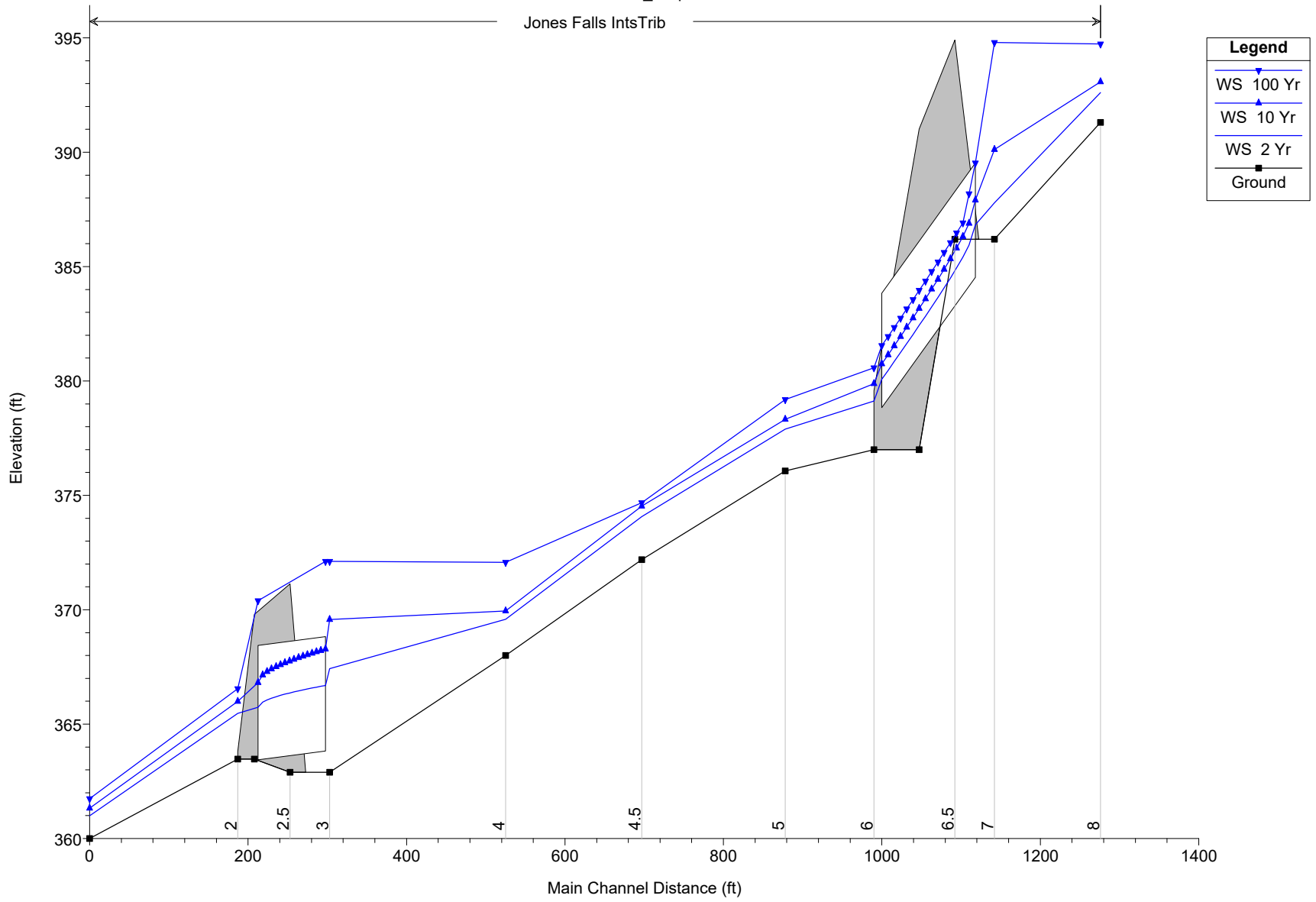
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EcclestonMod\_Proposed Plan: Proposed 1/10/2021

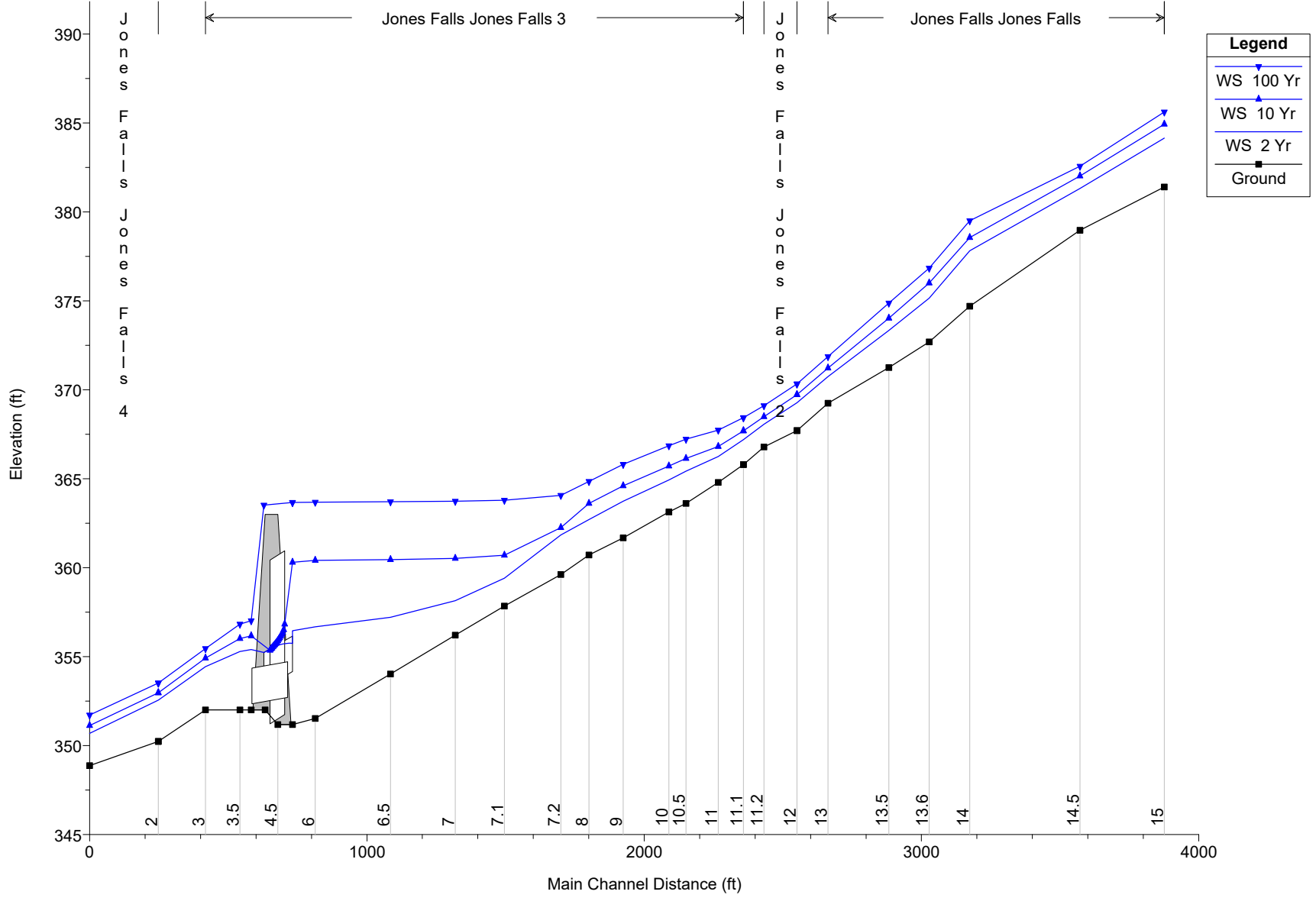
Geom: EcclestonMod\_Proposed with Junctions

Jones Falls IntsTrib



EcclestonMod\_Proposed Plan: Proposed 1/10/2021

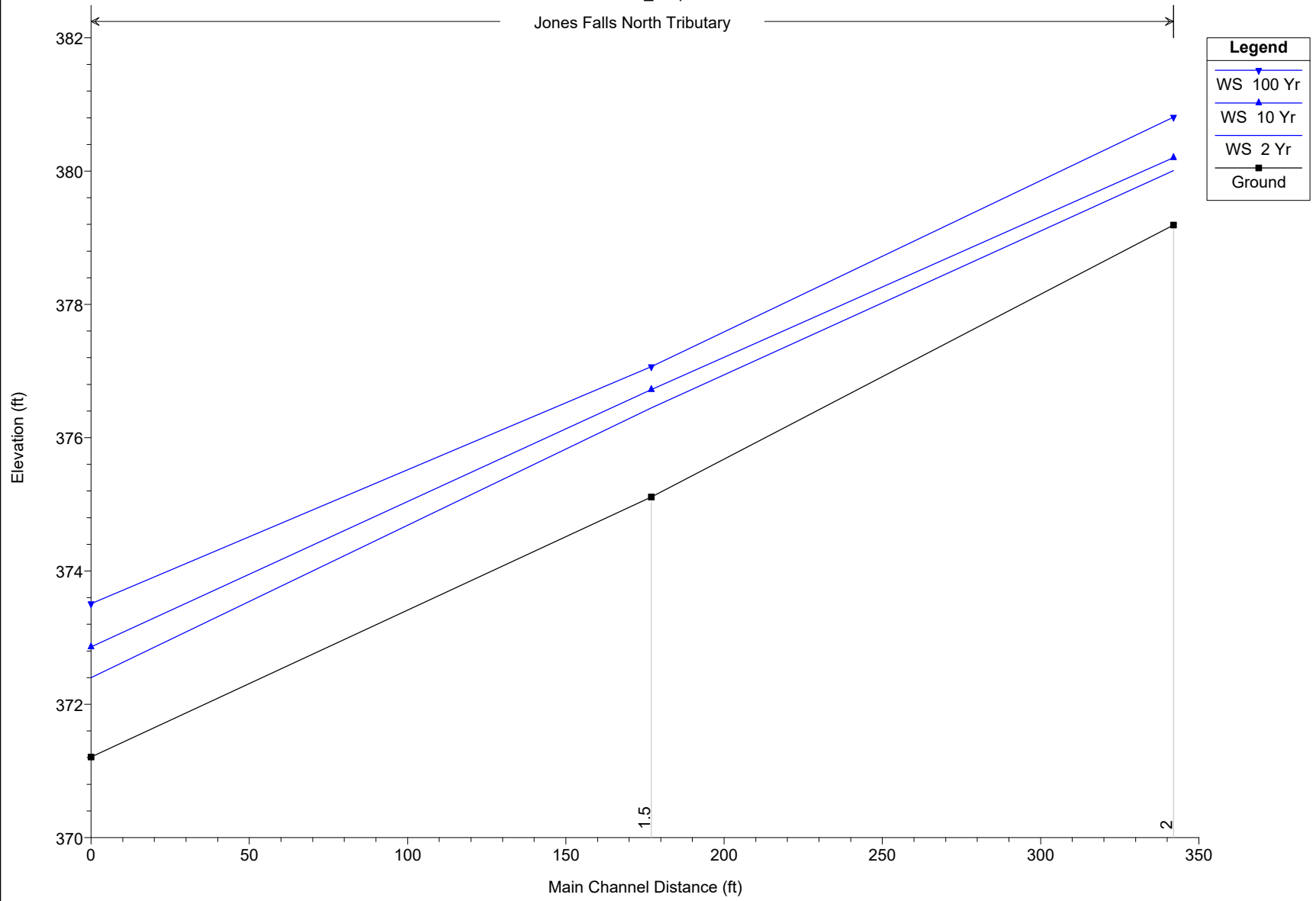
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EcclestonMod\_Proposed Plan: Proposed 1/10/2021

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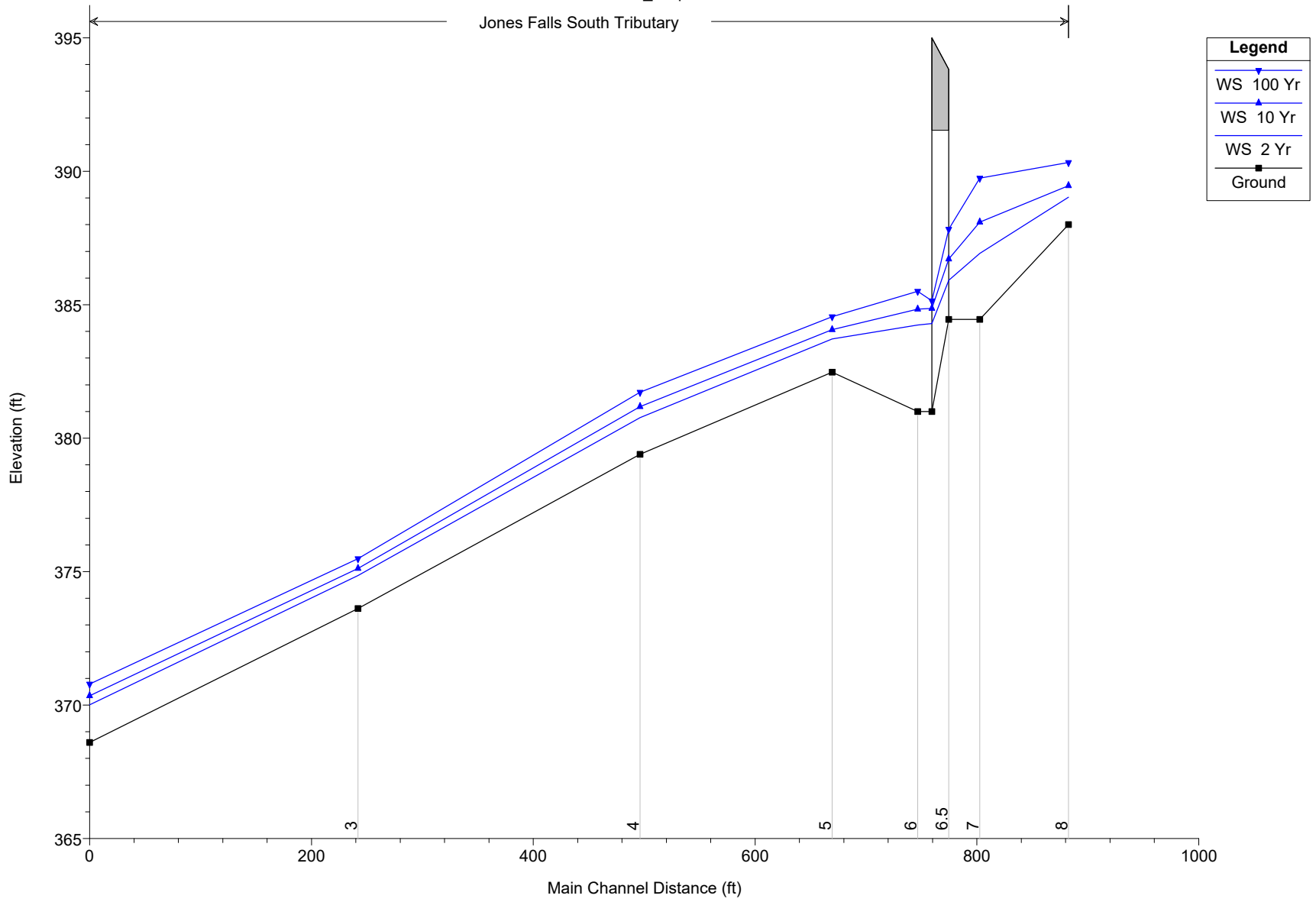
Jones Falls North Tributary



EcclestonMod\_Proposed Plan: Proposed 1/10/2021

Geom: EcclestonMod\_Proposed with Junctions

Jones Falls South Tributary





HEC-RAS Plan: Proposed

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
Jones Falls	15	2 Yr	363.30	381.40	384.15	383.82	137.29	134.97	0.57	0.59	1.31	0.46	0.55	2.65	5.03	1.13	2.80
Jones Falls	15	10 Yr	798.40	381.40	384.94	384.51	261.29	347.35	0.64	0.97	1.99	0.83	0.96	3.06	6.49	1.61	3.42
Jones Falls	15	100 Yr	1593.40	381.40	385.62	385.28	584.91	448.53	0.68	0.92	2.57	1.24	0.72	2.72	7.62	2.09	2.29
Jones Falls	14.5	2 Yr	363.30	378.97	381.32	380.96	204.32	242.87	0.56	0.55	0.90	0.30	0.60	1.78	3.87	0.82	1.32
Jones Falls	14.5	10 Yr	798.40	378.97	382.01	381.42	379.18	433.23	0.59	0.81	1.26	0.62	0.81	2.11	4.88	1.36	1.62
Jones Falls	14.5	100 Yr	1593.40	378.97	382.57	382.00	694.94	470.44	0.61	0.88	1.59	0.88	0.82	2.29	5.71	1.71	1.89
Jones Falls	14	2 Yr	363.30	374.70	377.82	377.63	185.95	204.85	0.56	0.51	1.24	0.33	0.47	1.95	4.88	0.90	1.13
Jones Falls	14	10 Yr	798.40	374.70	378.55	378.14	342.84	316.25	0.60	0.88	1.72	0.72	0.83	2.33	6.02	1.50	1.65
Jones Falls	14	100 Yr	1593.40	374.70	379.50	378.73	727.24	412.59	0.58	0.87	1.89	0.62	0.90	2.19	6.59	1.39	1.79
Jones Falls	13.6	2 Yr	363.30	372.69	375.14	375.14	63.46	76.06	0.94	2.35	4.08	2.37	1.49	5.72	7.84	4.41	4.20
Jones Falls	13.6	10 Yr	798.40	372.69	376.00	376.00	173.49	138.03	0.91	1.99	4.75	3.01	1.53	4.60	8.94	5.34	2.92
Jones Falls	13.6	100 Yr	1593.40	372.69	376.84	376.84	311.38	189.81	1.00	2.90	6.79	4.41	2.44	5.12	11.14	6.75	3.52
Jones Falls	13.5	2 Yr	363.30	371.25	373.34	372.78	148.28	112.23	0.47	0.68	0.96	0.65	0.66	2.45	3.71	2.29	2.29
Jones Falls	13.5	10 Yr	798.40	371.25	374.02	373.26	226.34	120.86	0.56	1.24	1.61	1.21	1.21	3.53	5.06	3.39	3.31
Jones Falls	13.5	100 Yr	1593.40	371.25	374.88	373.93	343.63	155.77	0.64	1.68	2.59	2.06	1.30	4.64	6.73	4.73	4.07
Jones Falls	13	2 Yr	363.30	369.25	370.73		140.12	192.19	0.69	0.91	1.52	0.84	0.89	2.59	4.35	2.41	2.43
Jones Falls	13	10 Yr	798.40	369.25	371.22		234.08	195.44	0.69	1.36	1.93	1.29	1.35	3.41	5.15	3.29	3.27
Jones Falls	13	100 Yr	1593.40	369.25	371.87		362.65	198.53	0.71	1.97	2.51	1.88	1.96	4.39	6.19	4.33	4.24
North Tributary	2	2 Yr	175.50	379.19	380.01	380.01	106.66	181.11	0.28	0.15	0.15	0.19	0.01	1.65	1.20	1.69	0.13
North Tributary	2	10 Yr	459.70	379.19	380.20	380.01	144.46	200.86	0.53	0.63	0.67	0.74	0.18	3.18	2.67	3.35	0.93
North Tributary	2	100 Yr	1025.90	379.19	380.81	380.45	273.00	219.23	0.53	0.90	0.98	1.00	0.54	3.76	3.56	4.02	2.12
North Tributary	1.5	2 Yr	175.50	375.11	376.45	376.36	86.30	179.18	0.67	0.60	1.24	0.54	0.56	2.03	3.80	1.72	1.78
North Tributary	1.5	10 Yr	459.70	375.11	376.72	376.63	135.86	181.58	0.89	1.53	2.59	1.43	1.46	3.38	5.73	3.04	3.12
North Tributary	1.5	100 Yr	1025.90	375.11	377.07	377.04	198.98	185.57	1.12	3.21	4.82	3.10	3.10	5.16	8.12	4.77	4.88
North Tributary	1	2 Yr	175.50	371.21	372.40		85.13	213.71	0.76	0.65	1.54	0.65	0.54	2.06	4.20	1.87	1.64
North Tributary	1	10 Yr	459.70	371.21	372.86		188.77	235.05	0.63	0.80	1.38	0.85	0.68	2.44	4.25	2.43	2.07
North Tributary	1	100 Yr	1025.90	371.21	373.51		348.39	257.43	0.58	1.00	1.50	1.10	0.86	2.94	4.72	3.05	2.55
South Tributary	8	2 Yr	151.70	388.00	389.03	389.03	34.45	57.16	1.00	2.11	2.11			4.40	4.40		
South Tributary	8	10 Yr	322.20	388.00	389.46	389.46	63.03	76.80	0.99	2.57	2.57			5.11	5.11		
South Tributary	8	100 Yr	622.10	388.00	390.33	389.94	152.27	143.46	0.62	1.08	1.45	0.17	0.17	4.09	4.22	1.27	1.27
South Tributary	7	2 Yr	151.70	384.45	386.93	385.94	49.96	32.36	0.40	0.70	0.70			3.04	3.04		
South Tributary	7	10 Yr	322.20	384.45	388.10	386.70	87.94	42.25	0.40	0.88	0.91		0.12	3.66	3.68		1.20
South Tributary	7	100 Yr	622.10	384.45	389.75	387.69	145.58	51.91	0.37	1.03	1.05		0.49	4.27	4.29		3.27
South Tributary	6.5		Bridge														
South Tributary	6	2 Yr	151.70	381.00	384.24	382.77	58.40	30.17	0.30	0.38	0.49	0.08	0.04	2.60	2.68	0.98	0.67
South Tributary	6	10 Yr	322.20	381.00	384.84	383.64	76.76	36.47	0.44	0.98	1.24	0.39	0.16	4.20	4.40	2.58	1.42
South Tributary	6	100 Yr	622.10	381.00	385.51	384.75	98.72	43.49	0.60	1.95	2.64	1.17	0.29	6.30	6.64	4.90	1.91
South Tributary	5	2 Yr	151.70	382.47	383.72	383.62	60.51	105.56	0.75	0.92	1.61	0.83	0.87	2.51	4.35	2.20	2.27
South Tributary	5	10 Yr	322.20	382.47	384.07	383.86	97.99	106.97	0.78	1.42	2.09	1.29	1.38	3.29	5.20	2.96	3.10
South Tributary	5	100 Yr	622.10	382.47	384.55	384.22	149.92	108.91	0.79	1.99	2.64	1.80	1.97	4.15	6.14	3.74	4.00
South Tributary	4	2 Yr	151.70	379.40	380.77		73.16	105.04	0.56	0.60	0.95	0.56	0.57	2.07	3.39	1.87	1.89
South Tributary	4	10 Yr	322.20	379.40	381.18		117.07	106.72	0.59	0.94	1.30	0.89	0.91	2.75	4.18	2.55	2.59

HEC-RAS Plan: Proposed (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
South Tributary	4	100 Yr	622.10	379.40	381.72		175.09	108.87	0.63	1.40	1.79	1.33	1.38	3.55	5.15	3.33	3.41
South Tributary	3	2 Yr	151.70	373.62	374.85	374.85	46.06	105.39	0.97	1.20	2.57	1.06	1.11	3.29	5.42	4.14	2.75
South Tributary	3	10 Yr	322.20	373.62	375.12	375.12	75.09	107.19	1.03	1.98	3.41	1.73	1.91	4.29	6.51	5.69	3.74
South Tributary	3	100 Yr	622.10	373.62	375.48	375.48	114.02	108.63	1.07	2.93	4.36	2.61	2.88	5.46	7.67	7.50	4.83
South Tributary	2	2 Yr	151.70	368.60	370.01	369.76	126.25	245.00	0.37	0.21	0.40	0.19	0.22	1.20	2.16	1.04	1.20
South Tributary	2	10 Yr	322.20	368.60	370.35		209.36	246.36	0.37	0.31	0.47	0.29	0.32	1.54	2.45	1.41	1.55
South Tributary	2	100 Yr	622.10	368.60	370.79		317.74	248.13	0.38	0.45	0.59	0.42	0.45	1.96	2.88	1.84	1.97
IntsTrib	8	2 Yr	66.30	391.30	392.61	392.61	13.99	20.62	1.01	1.53	1.53			4.74	4.74		
IntsTrib	8	10 Yr	138.90	391.30	393.07	393.07	25.18	27.38	1.01	1.88	1.88			5.52	5.52		
IntsTrib	8	100 Yr	261.20	391.30	394.73		100.24	78.31	0.35	0.23	0.39	0.07	0.07	2.61	2.87	0.91	0.91
IntsTrib	7	2 Yr	66.30	386.20	387.79	387.79	11.70	12.59	0.93	1.58	2.01		0.27	5.67	5.85		1.54
IntsTrib	7	10 Yr	138.90	386.20	390.11	388.54	86.64	58.39	0.20	0.09	0.19	0.04	0.08	1.60	2.17	0.88	1.35
IntsTrib	7	100 Yr	261.20	386.20	394.80	389.18	464.96	269.56	0.04	0.01	0.02	0.01	0.01	0.56	0.71	0.60	0.51
IntsTrib	6.5		Culvert														
IntsTrib	6	2 Yr	66.30	377.00	379.12	378.12	20.60	26.71	0.39	0.69	0.69			3.22	3.22		
IntsTrib	6	10 Yr	138.90	377.00	379.88	378.85	27.97	30.50	0.52	1.48	1.48			4.97	4.97		
IntsTrib	6	100 Yr	261.20	377.00	380.58	379.82	34.75	33.30	0.70	3.16	3.16			7.52	7.52		
IntsTrib	5	2 Yr	66.30	376.07	377.89	377.81	20.03	27.21	0.78	1.23	2.03	1.06	1.03	3.31	5.02	2.55	2.51
IntsTrib	5	10 Yr	138.90	376.07	378.32	378.22	32.12	29.43	0.85	1.96	2.90	1.79	1.71	4.32	6.29	3.68	3.59
IntsTrib	5	100 Yr	261.20	376.07	379.18	378.70	59.09	32.88	0.67	1.71	2.35	1.63	1.55	4.42	6.05	4.01	3.95
IntsTrib	4.5	2 Yr	66.30	372.19	374.07	373.93	25.60	36.22	0.63	0.75	1.44	0.64	0.66	2.59	4.30	1.99	2.02
IntsTrib	4.5	10 Yr	138.90	372.19	374.52	374.28	42.42	38.11	0.65	1.13	1.81	1.04	1.02	3.27	5.05	2.81	2.83
IntsTrib	4.5	100 Yr	261.20	372.19	374.68	374.68	48.24	38.74	1.01	3.01	4.65	2.80	2.72	5.41	8.20	4.74	4.75
IntsTrib	4	2 Yr	66.30	368.00	369.58	369.58	17.63	27.31	0.93	1.62	2.58	1.40	1.39	3.76	5.49	2.99	3.00
IntsTrib	4	10 Yr	138.90	368.00	369.95	369.95	28.04	28.79	1.01	2.57	3.65	2.34	2.32	4.95	6.87	4.28	4.29
IntsTrib	4	100 Yr	261.20	368.00	372.07	370.43	98.37	37.51	0.31	0.48	0.64	0.46	0.46	2.66	3.34	2.50	2.51
IntsTrib	3	2 Yr	66.30	362.90	367.43	364.99	54.16	20.87	0.13	0.09	0.09			1.22	1.22		
IntsTrib	3	10 Yr	138.90	362.90	369.57	365.58	100.57	109.85	0.11	0.10	0.10			1.38	1.38		
IntsTrib	3	100 Yr	261.20	362.90	372.12	366.32	594.90	280.77	0.04	0.01	0.02	0.00	0.01	0.44	0.59	0.28	0.41
IntsTrib	2.5		Culvert														
IntsTrib	2	2 Yr	66.30	363.47	365.47	365.19	14.87	17.64	0.68	0.78	1.36		0.24	4.46	5.00		1.60
IntsTrib	2	10 Yr	138.90	363.47	365.99	365.99	25.77	74.16	0.78	1.38	2.06		0.87	5.39	6.36		3.65
IntsTrib	2	100 Yr	261.20	363.47	366.55	366.55	37.97	102.29	0.85	2.20	2.89	0.60	1.75	6.88	7.84	2.82	5.73
IntsTrib	1	2 Yr	66.30	360.00	360.99	360.99	16.55	33.41	1.00	1.21	1.21			4.01	4.01		
IntsTrib	1	10 Yr	138.90	360.00	361.32	361.32	28.89	40.42	0.98	1.42	1.55		0.32	4.81	4.88		1.75
IntsTrib	1	100 Yr	261.20	360.00	361.74	361.74	47.53	49.02	0.95	1.64	1.89		0.63	5.50	5.71		2.80
Jones Falls 2	12	2 Yr	538.80	367.70	369.27		287.53	399.32	0.51	0.48	0.87	0.46	0.49	1.87	3.34	1.80	1.83
Jones Falls 2	12	10 Yr	1258.10	367.70	369.72		469.97	401.80	0.56	0.86	1.29	0.84	0.87	2.68	4.27	2.61	2.65
Jones Falls 2	12	100 Yr	2619.30	367.70	370.33		713.51	405.10	0.62	1.42	1.90	1.41	1.42	3.67	5.43	3.61	3.65
Jones Falls 2	11.2	2 Yr	538.80	366.78	368.06		255.95	442.53	0.62	0.64	1.02	0.68	0.55	2.11	3.40	2.19	1.77
Jones Falls 2	11.2	10 Yr	1258.10	366.78	368.49		449.55	458.33	0.63	0.99	1.37	1.06	0.88	2.80	4.20	2.91	2.47

HEC-RAS Plan: Proposed (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
Jones Falls 2	11.2	100 Yr	2619.30	366.78	369.11		749.49	509.43	0.64	1.35	1.81	1.53	1.15	3.49	5.15	3.75	2.98
Jones Falls 3	11.1	2 Yr	655.40	365.80	367.19		387.36	554.51	0.47	0.40	0.68	0.40	0.38	1.69	2.91	1.68	1.60
Jones Falls 3	11.1	10 Yr	1520.50	365.80	367.69		661.75	556.58	0.48	0.63	0.90	0.64	0.62	2.30	3.54	2.28	2.23
Jones Falls 3	11.1	100 Yr	3144.30	365.80	368.44		1082.38	559.73	0.47	0.87	1.10	0.87	0.86	2.90	4.16	2.89	2.85
Jones Falls 3	11	2 Yr	655.40	364.80	366.26		294.08	392.77	0.58	0.66	1.10	0.73	0.58	2.23	3.73	2.32	2.02
Jones Falls 3	11	10 Yr	1520.50	364.80	366.82		518.48	402.72	0.58	0.99	1.40	1.07	0.91	2.93	4.46	3.03	2.76
Jones Falls 3	11	100 Yr	3144.30	364.80	367.74		895.38	421.01	0.53	1.21	1.55	1.31	1.13	3.51	5.02	3.63	3.35
Jones Falls 3	10.5	2 Yr	655.40	363.61	365.42		315.78	344.31	0.49	0.56	0.89	0.56	0.54	2.08	3.43	2.03	2.00
Jones Falls 3	10.5	10 Yr	1520.50	363.61	366.14		562.99	347.19	0.47	0.80	1.06	0.79	0.78	2.70	4.00	2.66	2.64
Jones Falls 3	10.5	100 Yr	3144.30	363.61	367.22		941.71	351.54	0.45	1.03	1.24	1.03	1.02	3.34	4.62	3.30	3.29
Jones Falls 3	10	2 Yr	655.40	363.13	364.93		317.01	296.30	0.41	0.43	0.62	0.44	0.41	2.07	2.89	2.26	1.75
Jones Falls 3	10	10 Yr	1520.50	363.13	365.71		549.38	299.60	0.41	0.66	0.83	0.67	0.64	2.77	3.57	3.02	2.40
Jones Falls 3	10	100 Yr	3144.30	363.13	366.85		893.48	304.18	0.41	0.91	1.06	0.92	0.89	3.52	4.33	3.83	3.09
Jones Falls 3	9	2 Yr	655.40	361.68	363.73		269.54	220.15	0.49	0.70	0.99	0.71	0.66	2.43	3.73	2.43	2.26
Jones Falls 3	9	10 Yr	1520.50	361.68	364.60		461.20	223.73	0.51	1.08	1.36	1.09	1.05	3.30	4.67	3.29	3.14
Jones Falls 3	9	100 Yr	3144.30	361.68	365.81		734.59	228.59	0.52	1.56	1.84	1.58	1.52	4.28	5.79	4.28	4.12
Jones Falls 3	8	2 Yr	655.40	360.72	362.71		253.70	193.72	0.46	0.77	1.05	0.81	0.70	2.58	3.49	2.65	2.35
Jones Falls 3	8	10 Yr	1520.50	360.72	363.61		430.27	197.37	0.48	1.22	1.50	1.26	1.16	3.53	4.46	3.59	3.34
Jones Falls 3	8	100 Yr	3144.30	360.72	364.84		675.96	202.34	0.50	1.82	2.13	1.85	1.75	4.65	5.64	4.70	4.47
Jones Falls 3	7.2	2 Yr	655.40	359.62	361.84	361.25	260.16	192.72	0.49	0.74	1.03	0.72	0.71	2.52	3.86	2.42	2.43
Jones Falls 3	7.2	10 Yr	1520.50	359.62	362.26		340.49	194.39	0.75	2.12	2.79	2.10	2.05	4.47	6.56	4.32	4.34
Jones Falls 3	7.2	100 Yr	3144.30	359.62	364.07		699.87	201.68	0.53	1.68	1.99	1.69	1.61	4.49	6.09	4.40	4.41
Jones Falls 3	7.1	2 Yr	655.40	357.84	359.41		263.00	285.63	0.59	0.81	1.21	0.79	0.80	2.49	3.96	2.40	2.39
Jones Falls 3	7.1	10 Yr	1520.50	357.84	360.71		635.66	290.85	0.36	0.56	0.68	0.56	0.56	2.39	3.32	2.36	2.33
Jones Falls 3	7.1	100 Yr	3144.30	357.84	363.79		1553.43	303.31	0.19	0.30	0.33	0.30	0.29	2.02	2.63	2.01	1.98
Jones Falls 3	7	2 Yr	655.40	356.21	358.15		373.79	323.12	0.37	0.38	0.55	0.38	0.38	1.75	2.77	1.72	1.70
Jones Falls 3	7	10 Yr	1520.50	356.21	360.53		1154.75	333.19	0.16	0.15	0.17	0.15	0.15	1.32	1.79	1.31	1.30
Jones Falls 3	7	100 Yr	3144.30	356.21	363.74		2252.01	355.74	0.12	0.13	0.15	0.13	0.13	1.40	1.83	1.39	1.37
Jones Falls 3	6.5	2 Yr	655.40	354.03	357.21	355.83	296.72	152.51	0.32	0.42	0.60	0.47	0.39	2.21	3.16	2.20	2.10
Jones Falls 3	6.5	10 Yr	1520.50	354.03	360.45	356.56	1418.32	398.70	0.10	0.05	0.09	0.05	0.05	1.07	1.37	0.94	1.10
Jones Falls 3	6.5	100 Yr	3144.30	354.03	363.70	357.72	2801.52	452.05	0.07	0.04	0.07	0.04	0.05	1.12	1.27	0.96	1.17
Jones Falls 3	6	2 Yr	655.40	351.52	356.67	355.73	635.24	440.29	0.25	0.16	0.40	0.09	0.15	1.03	3.07	0.51	0.71
Jones Falls 3	6	10 Yr	1520.50	351.52	360.41	356.40	2738.29	599.02	0.09	0.05	0.08	0.04	0.06	0.56	1.52	0.42	0.53
Jones Falls 3	6	100 Yr	3144.30	351.52	363.68	357.13	4690.04	697.05	0.09	0.07	0.09	0.05	0.07	0.67	1.71	0.54	0.67
Jones Falls 3	5	2 Yr	655.40	351.18	356.46	354.55	294.36	392.98	0.28	0.32	0.43	0.06	0.28	2.23	2.99	0.38	1.02
Jones Falls 3	5	10 Yr	1520.50	351.18	360.30	355.85	835.37	619.46	0.17	0.28	0.34	0.20	0.29	1.82	2.61	0.97	1.23
Jones Falls 3	5	100 Yr	3144.30	351.18	363.67	357.27	5010.44	756.25	0.07	0.05	0.08	0.05	0.05	0.63	1.25	0.52	0.56
Jones Falls 3	4.5		Culvert														
Jones Falls 3	4	2 Yr	655.40	352.00	355.40	354.43	259.09	404.85	0.33	0.28	0.42	0.17	0.22	2.53	3.25	1.79	1.97
Jones Falls 3	4	10 Yr	1520.50	352.00	356.17	355.20	357.69	559.76	0.48	0.75	1.03	0.53	0.65	4.25	5.29	3.41	3.57
Jones Falls 3	4	100 Yr	3144.30	352.00	357.02	356.21	467.62	633.12	0.66	1.77	2.28	1.35	1.59	6.72	8.14	5.75	5.88

HEC-RAS Plan: Proposed (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl	Shear Total (lb/sq ft)	Shear Chan (lb/sq ft)	Shear LOB (lb/sq ft)	Shear ROB (lb/sq ft)	Vel Total (ft/s)	Vel Chnl (ft/s)	Vel Left (ft/s)	Vel Right (ft/s)
Jones Falls 3	3.5	2 Yr	655.40	352.00	355.29	354.74	260.90	387.48	0.36	0.29	0.54	0.20	0.23	2.51	3.32	1.85	2.07
Jones Falls 3	3.5	10 Yr	1520.50	352.00	356.01	355.30	392.37	501.46	0.46	0.65	1.03	0.52	0.57	3.88	4.77	3.28	3.51
Jones Falls 3	3.5	100 Yr	3144.30	352.00	356.82	356.07	540.16	588.71	0.58	1.37	1.94	1.16	1.24	5.82	6.78	5.25	5.49
Jones Falls 3	3	2 Yr	655.40	352.00	354.44	353.99	194.79	292.70	0.68	0.58	1.41	0.54	0.34	3.36	4.93	2.85	2.07
Jones Falls 3	3	10 Yr	1520.50	352.00	354.92	354.84	351.45	360.56	0.76	0.97	2.08	0.91	0.73	4.33	6.26	3.94	3.39
Jones Falls 3	3	100 Yr	3144.30	352.00	355.45	355.39	557.73	424.77	0.85	1.55	3.04	1.54	1.25	5.64	7.86	5.43	4.73
Jones Falls 4	2	2 Yr	721.70	350.23	352.56	352.43	278.63	479.52	0.56	0.37	0.83	0.34	0.34	2.59	3.67	2.43	2.41
Jones Falls 4	2	10 Yr	1659.40	350.23	352.97	352.72	470.51	518.95	0.58	0.61	1.07	0.57	0.58	3.53	4.35	3.45	3.41
Jones Falls 4	2	100 Yr	3405.50	350.23	353.51	353.19	817.66	623.99	0.59	0.75	1.32	0.79	0.69	4.16	5.05	4.33	3.92
Jones Falls 4	1	2 Yr	721.70	348.88	350.69	350.42	342.99	523.08	0.43	0.25	0.44	0.22	0.25	2.10	2.61	1.77	2.14
Jones Falls 4	1	10 Yr	1659.40	348.88	351.13	350.72	583.50	565.06	0.45	0.40	0.61	0.33	0.40	2.84	3.24	2.36	2.95
Jones Falls 4	1	100 Yr	3405.50	348.88	351.71	351.13	929.09	614.41	0.48	0.58	0.83	0.49	0.60	3.67	3.99	3.04	3.85

HEC-RAS HEC-RAS 5.0.7 March 2019  
 U.S. Army Corps of Engineers  
 Hydrologic Engineering Center  
 609 Second Street  
 Davis, California

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X      X  XXXXXX   XXXX       XXXX       XX       XXXX
X      X  X       X   X       X   X       X   X       X
X      X  X       X           X   X       X   X       X
XXXXXXXX XXXX     X           XXX  XXXX     XXXXXX     XXXX
X      X  X       X           X   X       X   X           X
X      X  X       X   X       X   X       X   X           X
X      X  XXXXXX   XXXX       X   X       X   X       XXXXX
  
```

PROJECT DATA

Project Title: EcclestonMod\_Proposed  
 Project File : EcclestonMod\_Propos.prj  
 Run Date and Time: 1/10/2021 1:38:44 PM

Project in English units

PLAN DATA

Plan Title: Proposed  
 Plan File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design  
 Data\Proposed HECRAS Model\EcclestonMod\_Propos.p01

Geometry Title: EcclestonMod\_Proposed with Junctions  
 Geometry File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working  
 Data\Design Data\Proposed HECRAS Model\EcclestonMod\_Propos.g02

Flow Title : EccNormalDepth\_ULT  
 Flow File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working  
 Data\Design Data\Proposed HECRAS Model\EcclestonMod\_Propos.f01

Plan Summary Information:

Number of:	Cross Sections =	44	Multiple Openings =	0
	Culverts =	3	Inline Structures =	0
	Bridges =	1	Lateral Structures =	0

Computational Information

Water surface calculation tolerance = 0.01  
 Critical depth calculation tolerance = 0.01

Maximum number of iterations = 20  
 Maximum difference tolerance = 0.3  
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary  
 Conveyance Calculation Method: At breaks in n values only  
 Friction Slope Method: Average Conveyance  
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: EccNormalDepth\_ULT  
 Flow File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design  
 Data\Proposed HECRAS Model\EcclestonMod\_Propos.f01

Flow Data (cfs)

River	Reach	RS	2 Yr	10 Yr
100 Yr				
Jones Falls	Jones Falls	15	363.3	798.4
1593.4				
Jones Falls	North Tributary	2	175.5	459.7
1025.9				
Jones Falls	South Tributary	8	151.7	322.2
622.1				
Jones Falls	IntsTrib	8	66.3	138.9
261.2				
Jones Falls	Jones Falls 2	12	538.8	1258.1
2619.3				
Jones Falls	Jones Falls 3	11.1	655.4	1520.5
3144.3				
Jones Falls	Jones Falls 4	2	721.7	1659.4
3405.5				

Boundary Conditions

River	Reach	Profile	Upstream
Downstream			
Jones Falls	Jones Falls 4	2 Yr	
Normal S = 0.00613			

Jones Falls Jones Falls 4 10 Yr  
 Normal S = 0.00613  
 Jones Falls Jones Falls 4 100 Yr  
 Normal S = 0.00613

GEOMETRY DATA

Geometry Title: EcclestonMod\_Proposed with Junctions  
 Geometry File : q:\2017\1710977\_002\_Eccleston\_PRM\_Turnke\Working Data\Design Data\Proposed HECRAS Model\EcclestonMod\_Propos.g02

Reach Connection Table

River	Reach	Upstream Boundary	Downstream Boundary
Jones Falls	Jones Falls		North Trib
Jones Falls	North Tributary		North Trib
Jones Falls	South Tributary		South Trib
Jones Falls	IntsTrib		Inters Trib
Jones Falls	Jones Falls 2	North Trib	South Trib
Jones Falls	Jones Falls 3	South Trib	Inters Trib
Jones Falls	Jones Falls 4	Inters Trib	

JUNCTION INFORMATION

Name: South Trib  
 Description:  
 Energy computation Method

Length across Junction		Tributary		Length	Angle
River	Reach	River	Reach		
Jones Falls	Jones Falls 2	to Jones Falls	Jones Falls 3	74	
Jones Falls	South Tributary	to Jones Falls	Jones Falls 3	327	0

Name: North Trib  
 Description:  
 Energy computation Method

Length across Junction		Tributary		Length	Angle
River	Reach	River	Reach		
Jones Falls	Jones Falls	to Jones Falls	Jones Falls 2	112	
Jones Falls	North Tributary	to Jones Falls	Jones Falls 2	246	

Name: Inters Trib  
 Description:

Energy computation Method

Length across Junction River	Reach		Tributary River	Reach	Length	Angle
Jones Falls	IntsTrib	to	Jones Falls	Jones Falls 4	507	
Jones Falls	Jones Falls 3	to	Jones Falls	Jones Falls 4	170	0

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls RS: 15

INPUT

Description:

Station Elevation Data num= 76

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-64.81	388	-18.82	386	0	384.95	10.51	384.63	28.87	384.1
28.94	384.27	39.73	383.95	45.82	383.82	48.75	383.58	66.63	383.12
77.82	382.91	97.75	383.32	105.52	383.13	107.91	381.73	109.11	381.48
112.78	381.42	113.29	381.4	117.45	381.57	118.5	381.88	118.85	382.16
119.91	383.19	122.49	383.07	124.08	383.04	124.54	382.98	124.86	382.97
127.48	381.9	128.4	381.53	128.82	381.49	129.85	381.68	137.78	383.54
152.38	383.34	182.38	385.15	183.51	385.21	188.97	385.23	201.5	385.4
217.33	385.08	228.51	384.75	256.65	384.31	264.53	384.58	267.67	384.77
289.28	384.76	291.21	384.83	292.85	384.58	303.94	384.73	348.05	384.47
350.59	384.93	352.63	384.92	359.02	384.42	368.24	384.57	401.91	385.11
403.25	385.13	405.83	385.11	407.12	385.11	407.75	385.12	410.52	385.15
442.48	385.72	443.04	385.73	443.56	385.74	444.06	385.75	444.53	385.75
444.98	385.76	445.39	385.77	445.8	385.77	446.18	385.78	446.55	385.78
446.9	385.79	447.23	385.79	447.55	385.8	447.85	385.8	450.73	385.84
451.08	385.85	451.41	385.85	451.73	385.86	455.45	385.95	456.22	385.96
579.98	388								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
-64.81	.11	105.52	.049	137.78	.11	410.52	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

105.52	119.91	306	304	297	.1	.3
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Ineffective Flow num= 1

Sta L	Sta R	Elev	Permanent
201.5	579.98	385.4	F

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls RS: 14.5



INPUT

Description:

Station Elevation Data										num=	31
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev		
0	390	31.7	388	48.67	386	69.09	384	78.17	383		
92.38	382	107.84	381	129.03	380.5	154.65	381	161.43	381.16		
168.17	381	175.06	380	180.75	379	181.81	378.97	185.75	379		
187.74	379	189.99	380	197.29	380.95	222.86	380	223.27	379.97		
223.67	380	286.46	380.04	315.37	381	368.42	382	376.18	382.08		
392.93	382	490.79	381.26	545.4	382	561.64	383	586.8	384		
664.47	386										

Manning's n Values								num=	4
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val		
0	.11	168.17	.049	197.29	.11	490.79	.04		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	168.17	197.29		422	398		.1	.3

Ineffective Flow					num=	3
Sta L	Sta R	Elev	Permanent			
0	161.43	381.16	F			
197.29	376.18	380.95	F			
376.18	664.47	382.08	F			

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls RS: 14

INPUT

Description:

Station Elevation Data										num=	84
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev		
0	393.41	20.17	392.15	27.9	390.54	30.04	390.17	39.43	387.67		
44.83	386.6	53.87	385	62.57	383.64	72.64	381.84	74.22	381.59		
87.3	380.1	94.05	379.49	96.13	379.19	111.3	379.02	118.68	378.84		
126.34	378.57	142.18	378.77	147.29	378.9	150.41	378.84	159.21	378.85		
159.87	378.89	171.47	377.04	173.59	376.65	175.12	376.75	194.4	377.52		
204.12	377.33	205.53	377.41	207.44	377.33	209.93	375.58	210.41	375.38		
211.32	374.79	213.49	374.74	215.02	374.7	218.14	374.72	219.9	375.13		
220.95	375.75	225.2	376.82	227	376.9	235.42	377.29	238	377.41		
239.38	377.49	240.43	377.47	258.76	376.48	259.19	376.37	262.57	375.62		
263.06	375.63	265.85	375.56	266.95	375.69	268.21	375.78	270.83	375.67		
279.55	376.4	285.8	376.78	289.08	377.3	289.5	377.38	307.02	377.18		
320.85	377.09	342.33	377.15	358.57	377.47	365	377.74	373.4	378.06		
394.81	378.73	412.38	378.13	427.04	378.29	429.77	378.48	437.37	378.47		
447.09	378.4	452.03	378.45	453.19	378.42	459.45	377.71	460.79	377.39		
464.24	378.11	465.27	378.27	467.4	378.28	478.15	377.93	479.58	377.88		
488.54	378.49	493.54	379.21	496.61	379.3	511.35	379.6	531.03	380.11		

539.1 380.17 542.93 380.26 550.7 380.58 568.17 381.15

Manning's n Values num= 4  
Sta n Val Sta n Val Sta n Val Sta n Val  
0 .11 207.44 .049 225.2 .11 550.7 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
207.44 225.2 145 146 147 .1 .3

Ineffective Flow num= 4  
Sta L Sta R Elev Permanent  
0 159.87 378.89 F  
159.87 194.4 377.52 F  
239.38 387.85 377.49 F  
394.81 568.17 378.73 F

CROSS SECTION

RIVER: Jones Falls  
REACH: Jones Falls RS: 13.6

INPUT

Description:

Station Elevation Data num= 41  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
0 394 12.73 393 23.59 392 34.32 391 45.06 390  
58.43 389 71.33 388 83.31 387 95.36 386 108.62 385  
124.87 384 135.45 383 140.16 382 144.82 381 149.48 380  
174.54 379 176.57 378 178.58 377 180.59 376 182.6 375  
184.61 374 197.14 373.5 198.47 373 200.08 372.9 203.1 372.69  
205.09 372.9 206.7 373 208.88 373.5 221.18 374 223.2 375  
252.87 375.43 297.54 375 298.96 374.9 303.71 375 315.82 375.44  
318.62 376 378.18 377 437.47 378 493.97 379 513.08 380  
544.28 382

Manning's n Values num= 7  
Sta n Val Sta n Val Sta n Val Sta n Val Sta n Val  
0 .04 135.45 .11 184.61 .07 197.14 .055 208.88 .07  
223.2 .11 437.47 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
197.14 208.88 133 146 114 .1 .3

Ineffective Flow num= 1  
Sta L Sta R Elev Permanent  
252.87 544.28 375.43 F

CROSS SECTION

RIVER: Jones Falls

REACH: Jones Falls RS: 13.5

INPUT

Description:

Station Elevation Data num= 58

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	384.47	3.6	385.33	4.06	385.45	9.41	385.2	21.69	383.88
23.74	383.44	35.99	381.7	42.94	380.89	80.81	376.99	83.66	375.56
84.77	375	85.39	374.69	85.91	374.44	86.21	374.29	88.78	373
90.24	372.27	90.91	372.05	93.71	372.05	136.56	372.05	140.57	372.07
142.72	372.05	147.74	372	148.49	371.97	149.68	371.5	150.94	371.33
151.43	371.3	152.07	371.26	153.7	371.25	154.75	371.32	156.19	371.5
157.38	371.98	158.88	372.03	167.5	372	173.22	371.99	196.2	371.99
196.99	372	203.06	374.42	203.55	374.48	209.68	374.47	214.9	374.63
221.52	374.61	226.51	373.86	227.72	373.65	230.29	373.93	236.91	374.83
249.15	374.99	251.55	374.94	258.37	375.64	266.93	375.82	278.23	376.02
279.55	376.05	312.46	376.65	316.16	376.71	316.55	376.72	345.44	377.24
355.33	377.68	374.6	378.5	378.37	378.68				

Manning's n Values num= 7

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.11	21.69	.04	90.91	.07	148.49	.055	157.38	.07
196.99	.11	316.55	.04						

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

148.49	157.38	196	219	219	.1	.3
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Ineffective Flow num= 1

Sta L	Sta R	Elev	Permanent
214.9	378.37	374.63	F

CROSS SECTION

RIVER: Jones Falls

REACH: Jones Falls RS: 13

INPUT

Description:

Station Elevation Data num= 71

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.89	5.68	378.2	21.77	377.04	22.92	376.93	24.39	376.85
25.53	376.59	36.71	375.64	47.72	375	59.08	374.4	71.55	374.05
73.21	374	83.14	373.34	85.75	373.22	86.18	373	86.62	372.79
86.97	372.61	88.21	372	89.39	371.43	90.25	371	90.67	370.84
91.01	370.83	92.12	370.69	92.77	370.6	94.44	370.5	95.79	370.5
97.76	370.23	98.21	370.24	103.96	369.91	105.03	370	109.57	370
136.7	370	140.61	370	141.32	370	142.19	370	143.95	370
144.5	370	145.43	370	148.69	370	149.09	369.99	150.34	369.5
151.6	369.33	152.86	369.25	154.77	369.28	155.28	369.31	155.66	369.34
156.87	369.5	158.05	369.97	158.61	370.01	159.28	370.01	162.06	370.02

173.72	370.02	280.79	370.01	282.05	370.03	282.49	370.19	287.59	372.09
288.02	372.12	299.68	373.36	300.99	373.9	302.21	373.27	307.65	374.02
314.53	374.32	315.57	374.45	318.94	374.55	324.97	374.53	339.64	375.13
381.22	376.04	392.56	376.29	431.54	377.85	435.5	378.02	436.23	378.04
452.78	378.54								

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	97.76	.07	148.69	.055	158.05	.07	280.79	.04

Bank Sta: Left Right Coeff Contr. Expan.  
 148.69 158.61 .1 .3

CROSS SECTION

RIVER: Jones Falls  
 REACH: North Tributary RS: 2

INPUT  
 Description:

Station Elevation Data num= 49

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-126.44	388	0	380.22	12.77	379.9	22.81	379.65	37.96	379.2
42.95	379.16	54.09	379.05	75.73	378.88	86.45	378.95	88.4	378.92
96.27	378.89	101.95	378.96	110.35	378.93	122.55	379.33	123.54	379.4
132.31	380	144.93	380	145.74	380.01	153.96	380.01	155.32	379.5
156.14	379.4	157.66	379.29	158.46	379.23	159.09	379.19	160.19	379.25
161.53	379.33	163	379.5	163.58	379.72	164.06	379.98	197.03	379.98
197.51	379.98	209	380.6	210.23	381	217.77	381.26	235.13	382.35
236.65	382.4	237.21	382.44	238.24	382.63	238.78	382.77	243.35	383.39
270.15	387.17	285.59	388.82	286.17	388.89	307.68	390.11	323.36	391.94
326.67	392.77	336.21	393.52	342.47	394.2	344.59	394.43		

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
-126.44	.04	96.27	.11	132.31	.07	153.96	.055	164.06	.07
197.03	.04								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 153.96 164.06 164 165 175 .1 .3

Ineffective Flow num= 1  
 Sta L Sta R Elev Permanent  
 -126.44 132.31 380 F

CROSS SECTION

RIVER: Jones Falls  
 REACH: North Tributary RS: 1.5

INPUT

Description:

Station Elevation Data num= 47

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-163.18	382	-79.65	380	0	378.54	24.34	377.77	28.79	377.66
50.42	377.35	56.67	377.22	63.87	377.18	83.78	377.13	87.93	377.1
93.02	377.3	101.5	377.24	104.06	376.01	104.61	376	117.48	376
157.34	376.02	158.44	376	158.81	375.85	159.12	375.73	159.7	375.5
160.25	375.42	161.17	375.32	163.09	375.15	163.49	375.11	163.86	375.13
164.23	375.16	165.19	375.23	166.13	375.22	167.5	375.5	168.08	375.72
169.93	375.97	172.32	375.98	278.94	376	279.38	376	286.01	377
294.3	377.38	309.99	378	318.34	378.91	324.06	379.17	337.47	380.31
343.27	381.12	344.96	381.35	366.36	384.61	368.48	384.91	369	384.98
383.2	387.03	383.97	387.09						

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
-163.18	.04	87.93	.11	104.06	.07	158.44	.055	169.93	.07
279.38	.04								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

158.44	169.93	173	177	182	.1	.3
--------	--------	-----	-----	-----	----	----

Ineffective Flow num= 1

Sta L	Sta R	Elev	Permanent
-163.18	93.02	377.3	F

CROSS SECTION

RIVER: Jones Falls  
 REACH: North Tributary RS: 1

INPUT

Description:

Station Elevation Data num= 56

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	377.17	7.39	377	13.62	376.88	36.29	376.37	62.64	375.62
65.09	375.52	65.91	375.57	71.21	375.82	77.97	376.11	78.8	376.15
83.33	376.04	84.8	375.34	85.53	375	87.04	374.21	87.38	374.04
88.6	373.46	89.1	373.22	89.56	373	91.66	372.01	92.42	372
190.31	372	212.12	372.01	213.02	372.01	214.59	371.5	215.42	371.4
217.65	371.25	218.3	371.21	219.88	371.31	221.06	371.39	222.05	371.5
223.17	371.95	262.63	372	263.6	372	267.58	372.02	271.14	372.05
277.33	372.1	282.56	372.15	300.54	372.31	322.57	372.81	330.98	373
334.85	373.13	360.31	374	395.29	374.92	398.34	375	402.05	375.13
402.94	375.16	403.82	375.19	405.38	375.25	407.95	375.34	411.58	375.47
415.77	375.62	425.87	376	427.87	377	428.72	377.04	434.55	377.42
439.01	377.96								

Manning's n Values			num=	5							
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	91.66	.07	213.02	.055	223.17	.07	425.87	.04		

Bank Sta: Left	Right	Coeff	Contr.	Expan.
213.02	223.17		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: South Tributary RS: 8

INPUT

Description:

Station Elevation Data			num=	9							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-32.1	392	0	391	110.6	390	123.66	388	133.45	388		
212.45	390	272.51	394	382.34	396	450.2	396				

Manning's n Values			num=	3	
Sta	n Val	Sta	n Val	Sta	n Val
-32.1	.045	110.6	.057	212.45	.045

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
110.6	212.45	75	80	75		.1	.3

Ineffective Flow		num=	2
Sta L	Sta R	Elev	Permanent
-32.1	56.22	395	F
221.59	450.2	395	F

CROSS SECTION

RIVER: Jones Falls  
 REACH: South Tributary RS: 7

INPUT

Description:

Station Elevation Data			num=	47							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	393.17	3.09	392.85	13.01	392.85	22.5	392.67	52.26	391.97		
66.09	392.38	75.87	392.64	92.5	391.43	97.86	391.37	116.16	390.34		
120.8	390.5	131.27	390.49	144.57	387.98	144.89	387.97	150.82	384.86		
151.07	384.58	152.67	384.79	153.39	384.66	158.59	384.56	162.95	384.45		
169.66	385.18	172.67	385.66	173.69	385.82	175.08	386.43	181.89	387.24		
184.64	387.76	185.99	387.76	187.24	389.95	188.3	390.53	188.9	390.62		
191.39	391.4	195.03	393.13	196.64	393.57	197	393.56	210.05	393.58		
212.34	393.64	213.21	393.64	228.47	393.49	263.39	393.37	264.3	393.44		
265.06	393.45	303.46	394.18	303.79	394.19	304.29	394.19	309.17	394.07		

326.52 393.83 332.83 393.82

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .045 144.89 .057 184.64 .045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
144.89 184.64 65 56 49 .3 .5  
Ineffective Flow num= 2  
Sta L Sta R Elev Permanent  
0 151 395 F  
186.01 332.83 395 F

BRIDGE

RIVER: Jones Falls  
REACH: South Tributary RS: 6.5

INPUT

Description: Cliffholme Rd Bridge  
Distance from Upstream XS = 28  
Deck/Roadway Width = 15.25  
Weir Coefficient = 2.6  
Upstream Deck/Roadway Coordinates

num= 6  
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord  
0 395 385.492 150.892 395 385.492 150.892 395 391.54  
170.892 395 391.54 170.892 395 385.492 332.826 395 385.492

Upstream Bridge Cross Section Data

Station Elevation Data num= 47  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
0 393.17 3.09 392.85 13.01 392.85 22.5 392.67 52.26 391.97  
66.09 392.38 75.87 392.64 92.5 391.43 97.86 391.37 116.16 390.34  
120.8 390.5 131.27 390.49 144.57 387.98 144.89 387.97 150.82 384.86  
151.07 384.58 152.67 384.79 153.39 384.66 158.59 384.56 162.95 384.45  
169.66 385.18 172.67 385.66 173.69 385.82 175.08 386.43 181.89 387.24  
184.64 387.76 185.99 387.76 187.24 389.95 188.3 390.53 188.9 390.62  
191.39 391.4 195.03 393.13 196.64 393.57 197 393.56 210.05 393.58  
212.34 393.64 213.21 393.64 228.47 393.49 263.39 393.37 264.3 393.44  
265.06 393.45 303.46 394.18 303.79 394.19 304.29 394.19 309.17 394.07  
326.52 393.83 332.83 393.82

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
0 .045 144.89 .057 184.64 .045

Bank Sta: Left Right Coeff Contr. Expan.  
144.89 184.64 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 151 395 F  
 186.01 332.83 395 F

Downstream Deck/Roadway Coordinates  
 num= 6  
 Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord  
 0 395 383.862 146.652 395 383.862 146.652 395 391.54  
 166.652 395 391.54 166.652 395 383.862 416.81 395 383.862

Downstream Bridge Cross Section Data  
 Station Elevation Data num= 38  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 0 389.29 12.9 388.91 14.45 388.94 44.82 388.41 54.81 388.24  
 71.93 387.62 85.4 387.32 86.73 387.3 107.09 386.93 110.5 386.5  
 111.58 386 112.66 385.5 113.8 385 128.62 383.5 129.63 383  
 132.65 382 135.68 381 143.73 381 146.77 382 149.9 383  
 150.95 383.5 151.99 385 156 385.5 163.73 385.98 164.34 386.34  
 167.64 388.88 168.77 388.99 173.15 388.95 185.41 389.39 188.48 389.46  
 222.71 390.49 228.29 390.41 234.95 390.59 268.52 391.39 275.81 391.6  
 302.82 392.14 316.4 391.65 340.08 391.29

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 0 .045 128.62 .057 150.95 .045

Bank Sta: Left Right Coeff Contr. Expan.  
 128.62 150.95 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 120.81 395 F  
 161.38 340.08 395 F

Upstream Embankment side slope = 0 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins =  
 Energy head used in spillway design =  
 Spillway height used in design =  
 Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data  
 Energy  
 Selected Low Flow Methods = Highest Energy Answer

High Flow Method  
 Energy Only



Additional Bridge Parameters

Add Friction component to Momentum

Do not add Weight component to Momentum

Class B flow critical depth computations use critical depth  
inside the bridge at the upstream end

Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: Jones Falls

REACH: South Tributary RS: 6

INPUT

Description:

Station Elevation Data num= 38

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	389.29	12.9	388.91	14.45	388.94	44.82	388.41	54.81	388.24
71.93	387.62	85.4	387.32	86.73	387.3	107.09	386.93	110.5	386.5
111.58	386	112.66	385.5	113.8	385	128.62	383.5	129.63	383
132.65	382	135.68	381	143.73	381	146.77	382	149.9	383
150.95	383.5	151.99	385	156	385.5	163.73	385.98	164.34	386.34
167.64	388.88	168.77	388.99	173.15	388.95	185.41	389.39	188.48	389.46
222.71	390.49	228.29	390.41	234.95	390.59	268.52	391.39	275.81	391.6
302.82	392.14	316.4	391.65	340.08	391.29				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.045	128.62	.057	150.95	.045

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	128.62	150.95		98.7695	77.1617	53.891	.3	.5

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	120.81	395	F
161.38	340.08	395	F

CROSS SECTION

RIVER: Jones Falls

REACH: South Tributary RS: 5

INPUT

Description:

Station Elevation Data num= 28

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	388	49.1	386.31	56.19	386	60.2	384	61.2	383.5
62.2	383.17	75.1	383.17	76.5	382.61	78.6	382.47	80.7	382.61

82.1	383.17	164.89	383.17	165.89	383.5	166.89	384	167.89	384.5
168.98	385	171.21	386	173.55	386.85	176.61	388.03	177.1	388.04
177.93	387.98	188.26	388.46	198.4	388.79	199.97	388.83	200.64	388.83
202.67	389.03	215.11	389.68	220.93	390				

Manning's n Values		num=		5					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	56.19	.07	75.1	.055	82.1	.07	166.89	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75.1	82.1		200.66	173.213		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: South Tributary RS: 4

INPUT  
 Description:

Station Elevation Data		num=		38					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	386	31.01	383.83	54.7	382.21	55.22	382	55.91	381.66
57.22	381	59.09	380.1	84.61	380.1	87.24	380.1	88.64	379.54
90.74	379.4	92.84	379.54	95.34	380.1	105.03	380.1	108.26	380.1
112.3	380.1	160.57	380.1	161.53	380.17	162.39	380.6	163.2	381
163.82	381.31	164.2	381.5	165.21	382	166.06	382.42	167.21	383
168.64	383.71	169.23	384	169.8	384.29	171.22	385	190.86	386.45
194.51	386.71	195.83	386.81	210.46	387.98	224.08	389.07	224.95	389.15
225.42	389.22	227.2	389.4	229.72	389.82				

Manning's n Values		num=		5					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	54.7	.07	87.24	.055	95.34	.07	171.22	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	87.24	95.34		254.6731254	1741253.6741		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: South Tributary RS: 3

INPUT  
 Description:

Station Elevation Data		num=		81					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	383	5.1	382.75	11.15	382.44	25.65	381.32	34.08	380.79
39.13	380.6	53.24	379.32	55.88	378	57.33	377.28	57.89	377

59.32	376.29	59.89	376	60.56	375.67	61.65	375.12	65.34	374.45
66.04	374.45	66.61	374.45	66.94	374.45	76.98	374.4	77.9	374
79.69	373.67	80.27	373.62	81.08	373.66	81.61	373.76	82.16	373.77
82.86	374	83.79	374.4	84.3	374.4	89.73	374.4	91.26	374.4
97.8	374.4	99.66	374.4	100.77	374.43	101.61	374.45	101.98	374.46
105.99	374.47	109.3	374.49	110.46	374.49	113.6	374.5	114.67	374.5
117.36	374.51	121.92	374.53	124.2	374.54	127.03	374.54	127.95	374.55
128.86	374.54	130.71	374.54	131.67	374.53	132.65	374.53	133.64	374.52
136.24	374.52	136.57	374.5	137.13	374.28	137.81	374	138.3	373.9
139.28	373.8	139.9	373.73	141.25	373.84	142.3	374	143.39	374.43
144.06	374.42	144.84	374.48	145.43	374.5	149.94	374.5	151.08	374.53
162.47	374.53	166.05	374.53	167.91	374.53	168.39	374.53	168.59	375
170.6	376	172.13	376.77	172.61	377	173.33	377.37	173.79	377.38
185.02	377.81	197.34	379.06	205.66	379.85	215.57	380.65	230.29	382.2
251.86	385.03								

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	76.98	.055	83.79	.07	136.24	.055	143.39	.07
168.39	.04								

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	76.98	83.79		220	242	244		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: South Tributary RS: 2

INPUT

Description:

Station Elevation Data num= 113

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	377.29	22.27	376.38	31.38	375.85	47.14	375.11	62.86	374.39
90.36	373.13	92.1	372.27	92.63	372	93.08	371.77	94.64	371
96.64	370	97.02	369.81	97.68	369.52	153.24	369.58	155.09	369.58
156.15	369.57	158.83	369.54	163.09	369.5	163.33	369.34	164.59	368.82
165.34	368.6	165.96	368.68	166.32	368.79	166.89	369	167.54	369.31
167.94	369.49	168.37	369.5	173.7	369.5	174.95	369.51	176.5	369.51
177.92	369.52	179.3	369.52	180.65	369.53	181.99	369.54	183.16	369.54
184.53	369.55	186.28	369.55	187.58	369.56	190.23	369.56	191.56	369.57
197.72	369.57	198.76	369.58	199.8	369.58	200.84	369.57	211.44	369.57
216.8	369.55	218.89	369.54	224.03	369.54	228.51	369.53	230.3	369.53
232.85	369.52	236.86	369.52	241.62	369.51	243.68	369.51	246.6	369.5
250.83	369.5	254.47	369.49	257.13	369.49	257.73	369.5	259.79	369.5
260.67	369.51	264.69	369.51	265.08	369.52	270.39	369.52	270.99	369.51
271.51	369.51	272.99	369.39	274.41	369.22	274.85	369.16	275.56	369
276.53	368.8	277.66	368.93	280.02	369.18	280.92	369.19	281.49	369.23
282.13	369.24	282.76	369.27	283.43	369.29	284.57	369.32	285.78	369.34

286.91	369.36	287.32	369.38	288.09	369.39	289.39	369.4	290.37	369.41
290.95	369.42	292.39	369.42	294.5	369.44	295.6	369.44	296.55	369.45
296.89	369.46	298.05	369.46	299.28	369.47	300.38	369.47	301.55	369.48
305.79	369.49	316.38	369.5	334.05	369.5	338.95	369.51	340.55	369.51
341.6	370	343.1	370.74	343.61	371	343.92	371.15	344.4	371.39
345.62	372	346.75	372.56	348.59	373.48	363.28	373.63	369.97	373.95
370.39	373.97	388.17	375.09	398.97	375.88				

Manning's n Values num= 7

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	97.68	.07	163.09	.055	167.94	.07	272.99	.055
288.09	.07	340.55	.04						

Bank Sta: Left Right Coeff Contr. Expan.  
 163.09 167.94 .1 .3

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 8

INPUT  
 Description:

Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	399	5.8	398.83	34.6	398	134.2	396	208.9	394
224.8	392	230.3	391.3	236.6	392	249.7	394	277.7	396
343.2	398	344.7	398.05	366.1	398.25	456.6	402		

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	5.8	.045	208.9	.046	249.7	.045	344.7	.013
366.1	.045								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 208.9 249.7 135 134 134 .1 .3

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 7

INPUT  
 Description:

Station Elevation Data num= 15

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	395	12.5	394.9	23.7	394	59.4	392	189.7	390
198.5	388	199	387.9	203.3	386.2	208.3	386.2	208.4	387.5

214.5 388 239.4 390 260.9 392 277.4 394 292.3 396

Manning's n Values num= 4  
 Sta n Val Sta n Val Sta n Val Sta n Val  
 0 .013 12.5 .045 198.5 .046 214.5 .045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 199 208.4 150 152 154 .3 .5  
 Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 182.82 395 F  
 281.48 292.3 395 F

CULVERT

RIVER: Jones Falls  
 REACH: IntsTrib RS: 6.5

INPUT  
 Description: Culvert under Greenspring Valley Rd.  
 Distance from Upstream XS = 50  
 Deck/Roadway Width = 44.8  
 Weir Coefficient = 2.6  
 Upstream Deck/Roadway Coordinates

num= 3  
 Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord  
 12.5 395 235 395 292.3 396

Upstream Bridge Cross Section Data

Station Elevation Data num= 15  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 0 395 12.5 394.9 23.7 394 59.4 392 189.7 390  
 198.5 388 199 387.9 203.3 386.2 208.3 386.2 208.4 387.5  
 214.5 388 239.4 390 260.9 392 277.4 394 292.3 396

Manning's n Values num= 4  
 Sta n Val Sta n Val Sta n Val Sta n Val  
 0 .013 12.5 .045 198.5 .046 214.5 .045

Bank Sta: Left Right Coeff Contr. Expan.  
 199 208.4 .3 .5  
 Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 182.82 395 F  
 281.48 292.3 395 F

Downstream Deck/Roadway Coordinates

num= 2  
 Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord

0 395 318.418 395

Downstream Bridge Cross Section Data

Station Elevation Data num= 134

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	392.47	.83	392.45	1.52	392.45	2.37	392.44	3.27	392.43
4.22	392.42	6.27	392.41	17.13	392.14	18.01	392.11	19.09	392.07
20.56	392.02	21.15	392	21.7	391.88	22.34	391.73	22.95	391.6
23.8	391.4	25.57	391	28.18	390.4	29.86	390	33.99	389
34.99	388.76	38.12	388	39.4	387.69	42.25	387	44.42	386.47
46.38	386	48.73	385.48	51.45	385.16	52.53	385.05	53.16	385
53.85	384.97	55.11	384.92	56.42	384.9	63.19	384.85	68.29	384.8
81.65	384.69	94.26	384.5	94.56	384.49	94.86	384.49	96.6	384.47
98.92	384.43	100.61	384.4	101.95	384.38	102.27	384.37	102.59	384.37
104.01	384.34	106.1	384.31	113.59	384.17	117.63	384.14	122.67	384.11
124.85	384.14	139.99	384.36	152.61	384.39	153.33	384.4	154.02	384.41
154.69	384.41	155.34	384.42	157.83	384.44	158.42	384.45	159	384.45
159.56	384.46	160.12	384.46	162.98	384.47	166.11	384.49	169.1	383
170.89	382.1	172.88	381.11	174.87	380.11	176.1	379.5	177.6	379
178.24	378.78	179.1	378.5	180.43	378.06	182.1	377.5	183.27	377.11
183.6	377	197.56	377	197.89	377.11	199.06	377.5	200.08	377.84
200.57	378	201.82	378.42	202.51	378.65	203.57	379	204.58	379.34
205.07	379.5	206.06	380	207.63	380.78	208.06	381	209.62	381.78
210.07	382	211.62	382.78	212.07	383	213.61	383.77	214.07	384
215.6	384.77	216.06	385	218.06	386	220.07	387	222.16	388
228.17	389	228.93	389.29	230.63	389.38	231.92	389.45	232.97	389.5
234.77	389.6	235.33	389.64	238.59	389.79	238.97	389.81	239.37	389.83
239.68	389.85	240.31	389.88	241.88	389.93	243.7	390	265.64	390.79
270.06	390.96	270.93	390.99	271.91	391.01	272.85	391.01	273.74	391.02
278.18	391.03	279.17	391.04	281.94	391.04	296.67	391	301.69	390.98
303.96	390.98	306.13	390.99	308.31	391.01	310.35	391.03	310.91	391.04
322.01	391.18	322.5	391.19	322.86	391.18	323.67	391.01		

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	6.27	.04	166.11	.055	215.6	.04

Bank Sta: Left Right Coeff Contr. Expan.  
 174.87 206.06 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 187.03 395 F  
 196.73 323.67 395 F

Upstream Embankment side slope = 3.5 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 5 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins =  
 Energy head used in spillway design =  
 Spillway height used in design =

Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name	Shape	Rise	Span			
GrnsprngVal	Circular	5				
FHWA Chart # 1 - Concrete Pipe Culvert						
FHWA Scale # 1 - Square edge entrance with headwall						
Solution Criteria = Highest U.S. EG						
Culvert Upstrm Dist	Length	Top n	Bottom n	Depth Blocked	Entrance Loss Coef	
Exit Loss Coef	24	118.04	.013	.013	0	.5

1

Upstream Elevation = 384.53  
Centerline Station = 205.8  
Downstream Elevation = 378.84  
Centerline Station = 191.8

CULVERT OUTPUT Profile #100 Yr Culv Group: GrnsprngVal

Q Culv Group (cfs)	261.20	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	13.30
Q Barrel (cfs)	261.20	Culv Vel DS (ft/s)	24.10
E.G. US. (ft)	394.80	Culv Inv El Up (ft)	384.53
W.S. US. (ft)	394.80	Culv Inv El Dn (ft)	378.84
E.G. DS (ft)	381.46	Culv Frctn Ls (ft)	2.86
W.S. DS (ft)	380.58	Culv Exit Loss (ft)	9.11
Delta EG (ft)	13.34	Culv Entr Loss (ft)	1.38
Delta WS (ft)	14.22	Q Weir (cfs)	
E.G. IC (ft)	394.80	Weir Sta Lft (ft)	
E.G. OC (ft)	393.63	Weir Sta Rgt (ft)	
Culvert Control	Inlet	Weir Submerg	
Culv WS Inlet (ft)	389.53	Weir Max Depth (ft)	
Culv WS Outlet (ft)	381.54	Weir Avg Depth (ft)	
Culv Nml Depth (ft)	2.37	Weir Flow Area (sq ft)	
Culv Crt Depth (ft)	4.49	Min El Weir Flow (ft)	395.01

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CROSS SECTION

RIVER: Jones Falls  
REACH: IntsTrib RS: 6

INPUT

Description:

Station Elevation Data									
num= 134									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	392.47	.83	392.45	1.52	392.45	2.37	392.44	3.27	392.43
4.22	392.42	6.27	392.41	17.13	392.14	18.01	392.11	19.09	392.07
20.56	392.02	21.15	392	21.7	391.88	22.34	391.73	22.95	391.6
23.8	391.4	25.57	391	28.18	390.4	29.86	390	33.99	389
34.99	388.76	38.12	388	39.4	387.69	42.25	387	44.42	386.47
46.38	386	48.73	385.48	51.45	385.16	52.53	385.05	53.16	385
53.85	384.97	55.11	384.92	56.42	384.9	63.19	384.85	68.29	384.8
81.65	384.69	94.26	384.5	94.56	384.49	94.86	384.49	96.6	384.47
98.92	384.43	100.61	384.4	101.95	384.38	102.27	384.37	102.59	384.37
104.01	384.34	106.1	384.31	113.59	384.17	117.63	384.14	122.67	384.11
124.85	384.14	139.99	384.36	152.61	384.39	153.33	384.4	154.02	384.41
154.69	384.41	155.34	384.42	157.83	384.44	158.42	384.45	159	384.45
159.56	384.46	160.12	384.46	162.98	384.47	166.11	384.49	169.1	383
170.89	382.1	172.88	381.11	174.87	380.11	176.1	379.5	177.6	379
178.24	378.78	179.1	378.5	180.43	378.06	182.1	377.5	183.27	377.11
183.6	377	197.56	377	197.89	377.11	199.06	377.5	200.08	377.84
200.57	378	201.82	378.42	202.51	378.65	203.57	379	204.58	379.34
205.07	379.5	206.06	380	207.63	380.78	208.06	381	209.62	381.78
210.07	382	211.62	382.78	212.07	383	213.61	383.77	214.07	384
215.6	384.77	216.06	385	218.06	386	220.07	387	222.16	388
228.17	389	228.93	389.29	230.63	389.38	231.92	389.45	232.97	389.5
234.77	389.6	235.33	389.64	238.59	389.79	238.97	389.81	239.37	389.83
239.68	389.85	240.31	389.88	241.88	389.93	243.7	390	265.64	390.79
270.06	390.96	270.93	390.99	271.91	391.01	272.85	391.01	273.74	391.02
278.18	391.03	279.17	391.04	281.94	391.04	296.67	391	301.69	390.98
303.96	390.98	306.13	390.99	308.31	391.01	310.35	391.03	310.91	391.04
322.01	391.18	322.5	391.19	322.86	391.18	323.67	391.01		

Manning's n Values							
num= 4							
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	6.27	.04	166.11	.055	215.6	.04

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	174.87	206.06		116	112	107		.3	.5
Ineffective Flow									
num= 2									
Sta L	Sta R	Elev	Permanent						
0	187.03	395	F						
196.73	323.67	395	F						

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 5

INPUT  
 Description:



Station Elevation Data									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	389.05	1.56	389	4.48	388.82	5.1	388.78	5.56	388.74
5.87	388.71	8.54	388	11.14	387.25	12.01	387	15.44	386.01
16.02	385.85	19	385	19.68	384.81	20.61	384.55	21.79	384.22
22.57	384	23.15	383.84	26.16	383	30.84	382.08	31.38	382
35.75	381.46	39.47	381	39.83	380.97	40.76	380.92	41.83	380.89
43.08	380.88	44.56	380.88	49.66	380.91	55.1	380.95	60.79	380.99
62	381	63.32	381.01	65.77	381.02	68.21	381.03	70.64	381.04
71.43	381.04	81.18	381.08	83.67	381.09	86.17	381.1	88.65	381.11
91.1	381.13	92.99	381.14	108.51	381.23	109.93	381.25	110.58	381.25
111.35	381.26	112.07	381.27	112.77	381.27	113.47	381.28	114.16	381.28
117.71	381.3	119.39	381.32	120.95	381.29	127.55	381.37	129.69	381.37
130.03	381.36	130.35	381.36	133.17	381.29	135.83	381.25	138.33	380
139.06	379.64	140.33	379	141.05	378.64	142.33	378	143.67	377.58
145.32	377.38	146.37	377.31	153.38	377.09	154.37	377.09	155.38	377.07
156.65	376.56	157.67	376.07	158.57	376.5	159.66	376.92	160.24	377.03
160.76	377.03	166.87	377.25	167.46	377.47	170.49	378	170.95	378.23
172.49	379	172.96	379.24	174.49	380	174.97	380.24	176.49	381
177.14	381.33	178.49	382	180.35	382.93	181.06	383	188.36	383.08
189.28	383.09	190.08	383.1	191.37	383.1	194.09	383.11	196.61	383.11
197.46	383.13	210.7	383.14	211.67	383.15	212.16	383.15	212.65	383.16
214.8	383.18	215.33	383.19	218.05	383.22	218.98	383.2	219.9	383.2
228.02	383	228.85	382.99	229.32	383	231.48	383.18	232.72	383.3
238.03	383.77	239.48	383.9	240.62	384	242.24	384.11	246.79	384.42
249.21	384.57	249.9	384.61	250.57	384.65	251.1	384.69	251.82	384.73
252.82	384.79	253.73	384.85	254.85	384.92	255.17	384.94	256.25	385
260.02	385.18	277.04	386	281.45	386.2	284.89	386.36	289.82	386.59

Manning's n Values									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	8.54	.04	142.33	.07	155.38	.055	160.24	.07
170.49	.04								

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	155.38	160.24		179	181	181		.1	.3

Ineffective Flow				
Sta L	Sta R	Elev	Permanent	
0	121.57	395	F	
121.57	129.69	381.37	F	
194.76	289.82	395	F	

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 4.5

INPUT  
 Description:

Station Elevation Data num= 106									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	384.26	.87	384.22	1.32	384.2	1.8	384.18	2.31	384.16
2.86	384.14	6.28	384	16.24	383.5	16.84	383.46	17.54	383.42
18.36	383.37	19.36	383.3	24.13	383	25.86	382.54	28.07	382
29.53	381.62	31.87	381	32.99	380.7	35.66	380	38.72	379.18
39.43	379	40.27	378.75	40.76	378.6	42.93	378	46.86	377
54.58	376	55.74	375.97	57.15	375.97	58.91	375.98	60.82	376
66.57	376.07	67.11	376.08	68.78	376.11	75.31	376.21	75.8	376.22
76.34	376.23	78.38	376.27	79.05	376.28	81.15	376.32	81.95	376.34
86.4	376.45	87.97	376.49	98.07	376.7	99.46	376.73	101	376.76
102.67	376.79	113.96	376.96	115.05	376.98	115.4	376.98	116.2	376.97
116.66	376.95	117.38	376.93	118.11	376.9	119.84	376.83	126.8	376.53
127.96	376	128.35	375.82	130.14	375	130.68	374.75	132.31	374
137.22	373.68	140.48	373.43	141.47	373.36	144.02	373.34	148.65	373.3
151.19	373.23	151.53	373.2	152.87	373.18	153.33	373	153.76	372.83
154.59	372.5	155.34	372.19	155.97	372.43	157.49	373	159.77	373.13
165.53	373.75	167.17	373.76	168.23	374	169.09	374.43	170.23	375
171.34	375.56	172.23	376	173.99	376.88	174.45	377.06	174.96	377.08
175.91	377.12	176.62	377.15	177.91	377.22	181.3	377.41	182.94	377.5
191.93	378	194.46	378.18	206.26	379	212.02	379.4	220.54	380
230.8	380.81	233.2	381	238.64	381.44	239.11	381.48	240.65	381.6
243.27	381.82	245.48	382	247.24	382.14	257.43	383	260.7	383.27
264.26	383.57								

Manning's n Values num= 6									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	6.28	.04	132.31	.07	152.87	.055	157.49	.07
168.23	.04								

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 152.87 157.49 162 172 179 .1 .3

Ineffective Flow num= 3				
Sta L	Sta R	Elev	Permanent	
0	53.91	384.26	F	
53.91	115.05	376.98	F	
227.38	264.26	384.26	F	

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 4

INPUT

Description:

Station Elevation Data num= 75									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	380.49	2.78	380.47	3.46	380.44	14.42	380	19.89	379.69
20.78	379.62	29.35	379	32.88	378.18	33.54	378.03	34.88	377.67

35.59	377.49	36.56	377.23	37.44	377	39.43	376.47	40.44	376.2
41.21	376	44.48	375.13	44.98	375	47.96	374.21	48.76	374
59.16	373.78	74.62	373.4	77.35	372.03	79.37	371.03	81.44	370
83.32	369	93.31	369	94.57	368.5	95.82	368	96.54	368.29
97.07	368.5	98.32	369	108.32	369	110.43	370	114.63	372
116.73	373	118.84	374	121.55	374.23	126.36	374.1	127.49	374.07
128.68	374.05	129.9	374.04	131.16	374.03	134.01	374.01	140.19	374.01
140.78	374.02	154.08	374.28	155.91	374.34	157.74	374.38	159.31	374.42
160.66	374.46	161.8	374.49	176.64	375	189.9	375.47	204.96	376
206.35	376.05	209.64	376.27	211.19	376.36	212.41	376.43	213.39	376.49
214.17	376.54	215.68	376.65	217.89	376.8	220.72	377	221.17	377.03
223.22	377.18	234.29	378	240.86	378.48	243.17	378.66	247.79	379
249.65	379.14	253.49	379.43	255.37	379.57	256.45	379.66	258.67	379.84

Manning's n Values num= 6

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.013	29.35	.04	83.32	.07	93.31	.055	98.32	.07
108.32	.04								

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	93.31	98.32		202 222	233		.1	.3
Ineffective Flow			num=	1				
Sta L	Sta R	Elev	Permanent					
121.55	258.67	374.23	F					

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 3

INPUT

Description:

Station	Elevation	Data	num=	148						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	
0	373.19	1.01	373.14	4.2	373	17.26	373	22.84	372.81	
23.68	372.81	24.13	372.8	24.59	372.8	26.42	372.78	26.86	372.78	
27.28	372.77	28.09	372.77	36.45	372.71	41.06	372.68	41.64	372.62	
42.5	372.56	57.3	372.43	57.77	372.4	70.69	372.28	71.37	372.27	
72.03	372.27	72.82	372.26	73.63	372.25	74.46	372.24	75.04	372.24	
75.87	372.23	76.42	372.22	79.86	372.19	80.37	372.19	82.06	372.17	
86.98	372.09	91.37	372	105.06	371.72	116.72	371.48	118.17	371.45	
119.45	371.43	120.4	371.41	121.54	371.39	122.52	371.37	123.55	371.35	
124.56	371.34	125.63	371.33	130.35	371.27	133.77	371.22	134.92	371.21	
136.11	371.19	137.35	371.16	138.68	371.13	144.05	371	151.8	370.8	
152.23	370.79	153.47	370.75	154.04	370.73	155.2	370.68	155.97	370.64	
157	370.58	157.86	370.53	158.94	370.46	161.05	370.31	165.48	370	
167.65	369.85	168.83	369.77	169.99	369.69	170.68	369.59	171.79	369.45	
173.56	369.17	174.71	369	179.95	369	180.32	368.85	182.44	368	
182.76	367.87	184.89	367	187.35	366	189.72	365.05	191.1	364.9	

195.78	364.5	196.7	364.22	197.44	364	198.89	363.56	199.49	363.38
200.22	363.08	200.63	362.9	201.66	363.28	202.23	363.5	203.54	364
204.24	364.26	204.39	365	204.53	366	204.66	367	204.8	368
204.97	369	235.63	369	247.89	369.16	248.23	369.17	248.6	369.18
249.02	369.18	249.98	369.2	250.91	369.21	251.43	369.22	252.35	369.23
253.24	369.24	256.57	369.28	257.35	369.29	260.89	369.35	261.68	369.36
262.45	369.36	263.19	369.37	266.93	369.4	269.66	369.45	273.09	369.48
273.8	369.49	276.53	369.49	282.27	369.6	285.09	369.58	286.01	369.61
286.99	369.63	288.04	369.67	288.62	369.67	289.71	369.71	290.22	369.71
295.92	369.94	297.44	370	298.26	370.03	301.11	370.15	303.31	370.24
305.08	370.31	306.68	370.38	309.12	370.48	313.21	370.65	319.83	370.91
321.86	370.99	328.67	371.25	329.28	371.28	329.64	371.3	333.56	371.45
334.01	371.47	336.29	371.56	337.68	371.61	338.36	371.64	339.43	371.67
340.11	371.7	340.86	371.72	341.67	371.74	342.41	371.76	343.06	371.78
352.37	372	352.99	372.01	446.13	372.75				

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 0 .04 179.95 .055 204.97 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 179.95 204.97 112 116 119 .3 .5  
 Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 182.06 371.13 F  
 204.02 446.13 371.13 F

CULVERT

RIVER: Jones Falls  
 REACH: IntsTrib RS: 2.5

INPUT

Description: Intersection Trib Culvert under Park Heights Ave.  
 Distance from Upstream XS = 50  
 Deck/Roadway Width = 45  
 Weir Coefficient = 2.6

Upstream Deck/Roadway Coordinates  
 num= 2  
 Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord  
 0 378.5 446.13 368.4

Upstream Bridge Cross Section Data

Station Elevation Data num= 148

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	373.19	1.01	373.14	4.2	373	17.26	373	22.84	372.81
23.68	372.81	24.13	372.8	24.59	372.8	26.42	372.78	26.86	372.78
27.28	372.77	28.09	372.77	36.45	372.71	41.06	372.68	41.64	372.62
42.5	372.56	57.3	372.43	57.77	372.4	70.69	372.28	71.37	372.27

72.03	372.27	72.82	372.26	73.63	372.25	74.46	372.24	75.04	372.24
75.87	372.23	76.42	372.22	79.86	372.19	80.37	372.19	82.06	372.17
86.98	372.09	91.37	372	105.06	371.72	116.72	371.48	118.17	371.45
119.45	371.43	120.4	371.41	121.54	371.39	122.52	371.37	123.55	371.35
124.56	371.34	125.63	371.33	130.35	371.27	133.77	371.22	134.92	371.21
136.11	371.19	137.35	371.16	138.68	371.13	144.05	371	151.8	370.8
152.23	370.79	153.47	370.75	154.04	370.73	155.2	370.68	155.97	370.64
157	370.58	157.86	370.53	158.94	370.46	161.05	370.31	165.48	370
167.65	369.85	168.83	369.77	169.99	369.69	170.68	369.59	171.79	369.45
173.56	369.17	174.71	369	179.95	369	180.32	368.85	182.44	368
182.76	367.87	184.89	367	187.35	366	189.72	365.05	191.1	364.9
195.78	364.5	196.7	364.22	197.44	364	198.89	363.56	199.49	363.38
200.22	363.08	200.63	362.9	201.66	363.28	202.23	363.5	203.54	364
204.24	364.26	204.39	365	204.53	366	204.66	367	204.8	368
204.97	369	235.63	369	247.89	369.16	248.23	369.17	248.6	369.18
249.02	369.18	249.98	369.2	250.91	369.21	251.43	369.22	252.35	369.23
253.24	369.24	256.57	369.28	257.35	369.29	260.89	369.35	261.68	369.36
262.45	369.36	263.19	369.37	266.93	369.4	269.66	369.45	273.09	369.48
273.8	369.49	276.53	369.49	282.27	369.6	285.09	369.58	286.01	369.61
286.99	369.63	288.04	369.67	288.62	369.67	289.71	369.71	290.22	369.71
295.92	369.94	297.44	370	298.26	370.03	301.11	370.15	303.31	370.24
305.08	370.31	306.68	370.38	309.12	370.48	313.21	370.65	319.83	370.91
321.86	370.99	328.67	371.25	329.28	371.28	329.64	371.3	333.56	371.45
334.01	371.47	336.29	371.56	337.68	371.61	338.36	371.64	339.43	371.67
340.11	371.7	340.86	371.72	341.67	371.74	342.41	371.76	343.06	371.78
352.37	372	352.99	372.01	446.13	372.75				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.04	179.95	.055	204.97	.04

Bank Sta: Left Right Coeff Contr. Expan.

179.95	204.97	.3	.5
--------	--------	----	----

Ineffective Flow num= 2

Sta L	Sta R	Elev	Permanent
0	182.06	371.13	F
204.02	446.13	371.13	F

Downstream Deck/Roadway Coordinates num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	376.2		303.7	369.8	

Downstream Bridge Cross Section Data Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	372.5	15.7	372	88.3	370	149.6	368	155.3	366
157.93	363.47	162.93	363.47	163.3	365	185.2	366	203.4	366
241.82	365.5	300	369.8	306.44	369.8	321.8	370		

Manning's n Values num= 4  
 Sta n Val Sta n Val Sta n Val Sta n Val  
 0 .045 155.3 .046 163.3 .045 300 .013

Bank Sta: Left Right Coeff Contr. Expan.  
 155.3 163.3 .3 .5

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 154.76 369.8 F  
 176.73 321.8 369.8 F

Upstream Embankment side slope = 2.4 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 3.5 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins =  
 Energy head used in spillway design =  
 Spillway height used in design =  
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span  
 ParkHgtsAve Circular 5  
 FHWA Chart # 1 - Concrete Pipe Culvert  
 FHWA Scale # 1 - Square edge entrance with headwall  
 Solution Criteria = Highest U.S. EG  
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef  
 Exit Loss Coef  
 1 5.44 84.85 .024 .024 0 .5

Upstream Elevation = 363.83  
 Centerline Station = 196.84  
 Downstream Elevation = 363.44  
 Centerline Station = 160.43

CULVERT OUTPUT Profile #100 Yr Culv Group: ParkHgtsAve

Q Culv Group (cfs)	200.51	Culv Full Len (ft)	54.89
# Barrels	1	Culv Vel US (ft/s)	10.21
Q Barrel (cfs)	200.51	Culv Vel DS (ft/s)	11.79
E.G. US. (ft)	372.12	Culv Inv El Up (ft)	363.83
W.S. US. (ft)	372.12	Culv Inv El Dn (ft)	363.44
E.G. DS (ft)	367.34	Culv Frctn Ls (ft)	1.67
W.S. DS (ft)	366.55	Culv Exit Loss (ft)	2.30
Delta EG (ft)	4.78	Culv Entr Loss (ft)	0.81
Delta WS (ft)	5.57	Q Weir (cfs)	60.69
E.G. IC (ft)	371.99	Weir Sta Lft (ft)	283.46
E.G. OC (ft)	372.12	Weir Sta Rgt (ft)	362.14
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	368.83	Weir Max Depth (ft)	0.95

Culv WS Outlet (ft)	367.48	Weir Avg Depth (ft)	0.42
Culv Nml Depth (ft)	5.00	Weir Flow Area (sq ft)	32.89
Culv Crt Depth (ft)	4.04	Min El Weir Flow (ft)	371.14

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 2

INPUT

Description:

Station Elevation Data	num=	14							
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev									
0 372.5 15.7 372 88.3 370 149.6 368 155.3 366									
157.93 363.47 162.93 363.47 163.3 365 185.2 366 203.4 366									
241.82 365.5 300 369.8 306.44 369.8 321.8 370									

Manning's n Values	num=	4				
Sta n Val Sta n Val Sta n Val Sta n Val						
0 .045 155.3 .046 163.3 .045 300 .013						

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.								
155.3 163.3 209 187 167 .3 .5								

Ineffective Flow	num=	2	
Sta L Sta R Elev Permanent			
0 154.76 369.8 F			
176.73 321.8 369.8 F			

CROSS SECTION

RIVER: Jones Falls  
 REACH: IntsTrib RS: 1

INPUT

Description:

Station Elevation Data	num=	10						
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev								
0 363 43.1 364 84.7 365 156.97 364 186.15 363								
207.34 362 239.78 362 255.12 360 281.17 361 371.98 368								

Manning's n Values	num=	3			
Sta n Val Sta n Val Sta n Val					
0 .04 239.78 .046 281.17 .045					

Bank Sta: Left	Right	Coeff	Contr.	Expan.
239.78	281.17		.1	.3
Ineffective Flow	num=	1		
Sta L	Sta R	Elev	Permanent	
294.42	371.98	371.13	F	

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 2      RS: 12

INPUT

Description:

Station Elevation Data	num=	152							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	377.02	20.49	376.53	27.96	376.29	46.34	375.72	59.4	375.36
70.73	375.14	90.58	374.57	103.06	373.94	104.46	373.88	109.15	372.61
114.32	373.79	123.55	373.82	124.48	373.8	126.29	373.79	130.45	373.59
131.55	373.46	132.13	373.17	132.48	373	133.24	372.62	134.49	372
135.35	371.57	136.5	371	137.36	370.57	138.42	370.05	139.01	369.75
140.31	369.1	140.85	368.84	143.75	368.62	148.53	368.62	152.77	368.62
157.67	368.63	169.45	368.62	176.5	368.63	180.15	368.63	183.63	368.64
187.37	368.64	188.33	368.63	188.82	368.63	189.55	368.62	190.38	368.62
190.84	368.61	191.41	368.6	191.85	368.6	192.36	368.59	193.46	368.57
193.83	368.57	194.29	368.56	194.69	368.5	195.38	368.22	196.35	368
197.52	367.88	198.24	367.8	198.6	367.76	199	367.72	200.83	367.79
201.67	367.86	203.09	367.99	204.4	368.5	205.49	368.54	206.25	368.54
207.24	368.55	208.34	368.55	208.88	368.56	209.92	368.57	210.93	368.57
211.34	368.58	227.66	368.58	231.38	368.57	231.96	368.57	241.66	368.56
243.05	368.56	246.6	368.55	255.74	368.54	256.76	368.54	261.83	368.53
272.07	368.56	272.75	368.57	276.31	368.57	279.66	368.5	281.21	368
282.07	368.36	282.51	368.5	294.62	368.5	297.28	368.65	304.65	368.7
306.4	368.71	309.8	368.73	313.46	368.75	316.23	368.76	318.09	368.77
322.62	368.79	327.22	368.82	327.78	368.82	330.45	368.83	335.82	368.84
337.82	368.84	341.43	368.82	346.53	368.8	352.26	368.79	353.01	368.79
353.53	368.78	355.99	368.76	360	368.71	366.06	368.65	368.8	368.62
370.65	368.61	378.02	368.53	378.69	368.52	382.46	368.5	382.81	368.5
390.44	368.45	392.43	368.44	393.54	368.44	396.08	368.43	398.59	368.42
406.19	368.33	406.67	368.33	411.84	368.3	418.41	368.5	421.2	368.51
425.98	368.5	427.94	368.47	429.46	368.44	430.57	368	432.02	367.81
432.69	367.77	433.76	367.7	434.32	367.72	435.83	367.82	437.16	368
438.42	368.5	438.86	368.5	442.36	368.51	534	368.51	537.7	369
538.36	369	541.81	370	543.3	370.43	546.96	370.85	548.21	371
552.1	371.45	560.73	371.94	562.68	372.02	571.38	372.46	574.08	372.52
576.03	372.54	582.37	372.74	608.52	373.45	642.4	374.43	659.9	375
669.78	375.35	687.83	375.97						

Manning's n Values      num=      9



Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	143.75	.07	193.46	.055	204.4	.07	279.66	.055
282.51	.07	429.46	.055	438.42	.07	534	.04		

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 429.46 438.42 71 119 122 .1 .3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 2 RS: 11.2

INPUT

Description:

Station Elevation Data num= 157

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	376.29	22.28	375.62	31.37	375.32	46.47	374.92	62.56	374.35
72.17	374.09	89.78	373.47	100.07	372.95	100.83	372.91	106.66	371.76
107.05	371.78	119.95	372.73	120.83	372.76	125.13	372.62	128.49	372.62
130.46	372.65	130.81	372.68	132.17	372	132.69	371.74	133.08	371.54
134.18	371	135.24	370.47	136.17	370	137.1	369.54	138.17	369
139.24	368.47	140.18	368	141.2	367.5	141.9	367.5	148.3	367.53
152.72	367.54	153.21	367.53	154.61	367.48	161.28	367.47	169.91	367.46
172.39	367.46	178.83	367.45	182.85	367.45	185.55	367.44	189.3	367.44
194.16	367.43	198.85	367.43	199.43	367.44	199.88	367.45	200.41	367.46
200.94	367.47	201.63	367.47	202.18	367.48	203.44	367.48	204.04	367.49
204.48	367.5	206.44	367.5	207.91	367.32	208.37	367.25	208.79	367.2
209.45	367	209.89	366.89	213.56	366.9	214.04	367	214.55	367.2
215.36	367.37	215.93	367.5	221.24	367.5	222.5	367.49	223.56	367.47
224.14	367.46	225.59	367.46	226.84	367.44	227.18	367.44	228.02	367.43
229.14	367.42	230.19	367.41	231.04	367.4	232.12	367.39	232.83	367.38
233.88	367.37	234.69	367.37	235.77	367.36	236.54	367.35	238.27	367.34
242.76	367.33	244.58	367.3	245.37	367.29	253.87	367.26	255.97	367.25
257.04	367.24	263.25	367.21	263.6	367.2	264.7	367.16	267.21	367.14
267.92	367.13	271.08	367	271.69	366.7	272.28	366.96	277.46	367.16
277.85	367.16	285.89	367.24	290.35	367.33	290.98	367.34	291.36	367.34
291.84	367.37	295.63	367.44	296.17	367.44	297.94	367.45	308.93	367.49
311.16	367.5	315.02	367.52	321.72	367.55	322.45	367.55	326.43	367.56
327.51	367.56	328.09	367.57	341.15	367.65	343.34	367.65	346.49	367.63
347.35	367.63	367.54	367.68	368.4	367.68	386.33	367.55	392.15	367.5
393.65	367	394.98	366.82	395.53	366.79	398.27	366.78	400.04	366.98
401.08	367.34	401.38	367.4	402.78	367.45	405.87	367.5	409.03	367.51
541.12	367.51	546.19	367.53	549.55	367.53	580.58	368	604.22	368.68
611.19	368.83	615.2	368.85	645.62	368.99	647.14	369	647.72	369.28
648.13	369.49	649.15	370	650.58	370.72	651.15	371	651.99	371.42
652.31	371.58	653.16	372	654.15	372.5	655.82	373.33	664.56	373.66
668.29	373.54	694.11	374.89	697.12	375.05	699.75	375.18	724.41	376.24
754.67	377.21	756.67	377.28						

Manning's n	Values		num=	9					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	141.2	.07	206.44	.055	215.93	.07	267.21	.055
277.46	.07	392.15	.055	405.87	.07	647.14	.04		

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	392.15	405.87		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 11.1

INPUT

Description:

Station Elevation Data		num=	214						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	375.24	13.07	374.8	31.03	374.22	39.02	373.95	61.25	373.15
63.89	373.06	65.68	373	119.83	373	121.29	372.27	121.84	372
123.84	371	125.62	370.11	126.78	369.53	127.84	369	128.87	368.49
129.69	368.07	130.21	367.82	131.84	367	132.81	366.51	136.58	366.5
168.16	366.5	173.84	366.51	178.73	366.51	186.85	366.52	192.74	366.52
203.74	366.5	206.86	366.5	209.63	366.4	211.21	366	212.54	365.71
213.11	365.75	214.74	366	215.53	366.21	218.65	366.5	219.84	366.45
221.85	366.5	224.38	366.51	228.03	366.54	229.79	366.5	234.67	366.5
235.9	366	236.8	365.64	237.11	365.56	237.42	366	237.79	366.5
240.7	366.52	317.92	366.5	329.76	366.49	330.19	366.49	330.71	366.48
332.36	366.44	335.34	366	342.05	365.8	346.37	366	346.78	366.19
347.3	366.43	349.83	366.35	350.62	366.44	363.45	366.38	363.85	366.4
364.68	366.41	366	366.45	366.86	366.48	368.65	366.47	377.1	366.46
378.38	366.46	380.66	366.45	385.47	366.45	385.97	366.44	397.28	366.44
402.7	366.43	407.35	366.42	413.23	366.42	414.16	366.43	415.83	366.43
416.59	366.44	418.18	366.44	419.01	366.45	420.35	366.45	420.84	366.46
421.77	366.46	422.17	366.47	423.16	366.47	424.19	366.46	424.75	366.46
425.47	366.47	425.82	366.47	426.67	366.44	427.06	366.45	427.65	366.37
428.39	366.38	429.03	366.38	431.25	366	433.65	366.48	434.77	366.51
435.13	366.52	435.83	366.53	436.87	366.54	437.35	366.54	438.35	366.56
438.77	366.55	439.15	366.54	439.88	366.52	440.74	366.5	441.52	366.48
441.92	366.47	442.47	366.46	442.78	366.47	443.12	366.47	444.08	366.46
445.94	366.45	454.16	366.45	455.17	366.46	456.15	366.46	457.44	366.47
458.67	366.48	459.88	366.48	461.04	366.49	462.22	366.49	463.83	366.5
467.65	366.51	472.99	366.53	474.24	366.54	475.46	366.54	477.79	366.55
478.9	366.55	483.11	366.56	487.44	366.56	489.16	366.57	491.49	366.57
493.02	366.58	497.23	366.58	501.24	366.59	504.78	366.59	508.11	366.6
511.82	366.61	518.79	366.61	537.73	366.58	543.39	366.59	544.87	366.1
549.77	366.59	555.01	366.6	563.48	366.6	567.51	366.61	575.59	366.61
578.55	366.62	583.12	366.62	587.06	366.63	592.07	366.63	594.03	366.64
605.43	366.64	608.95	366.62	609.79	366.61	610.66	366.6	611.56	366.6
612.5	366.59	613.45	366.59	616.5	366.57	617.38	366.56	617.7	366.55

618.17	366.54	619.17	366.52	619.96	366.51	620.56	366.51	621.07	366.5
623.53	366.05	625.68	366.44	626.18	366.5	626.53	366.5	627.6	366.46
628.51	366.43	629.36	366.41	630.23	366.41	631.28	366.43	632.51	366.47
633.01	366.48	633.69	366.5	635.08	366.5	636.12	366.51	637.33	366.51
639.26	366.52	640.9	366.52	641.55	366.53	643.94	366.53	645.38	366.54
647.84	366.54	648.6	366.55	662.93	366.55	665.53	366.56	667.99	366.56
669.44	366.59	671.68	366.59	673.75	366.58	674.95	366.63	678.45	366.61
679.79	366.61	680.66	366.67	684.48	366.63	684.74	366.63	687.74	368
689.56	368.84	689.92	369	691.07	369.53	692.1	370	693.29	370.54
694.28	371	695.2	371.42	695.73	371.66	697.58	372.58	703.54	372.67
711.78	373.25	732.36	374.67	753.71	376.41	764.06	377.23		

Manning's n Values num= 15

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	132.81	.07	209.63	.055	218.65	.07	234.67	.055
237.79	.07	332.36	.055	347.3	.07	425.82	.055	433.65	.07
543.39	.055	549.77	.07	619.17	.055	626.18	.07	684.48	.04

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
332.36	347.3	112	91	53		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 11

INPUT

Description:

Station Elevation Data num= 233

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	375.91	9.58	375.41	36.89	373.9	40.01	373.72	41.2	373.66
51.45	373.11	53.59	373	129.85	373	131.85	372	132.16	371.85
132.64	371.61	133.86	371	134.92	370.46	135.86	370	136.79	369.54
137.87	369	138.92	368.47	139.78	368.04	140.55	367.66	141.87	367
142.89	366.49	144.83	365.51	147.61	365.51	152.75	365.5	153.07	365.5
155.01	365.53	157.36	365.54	171.47	365.54	172.5	365.55	174.7	365.56
181.47	365.56	185.15	365.55	191.19	365.53	194.61	365.52	198.22	365.51
207.74	365.51	209.09	365.5	218.99	365.5	219.8	365.45	221.67	365
222.51	364.92	223.53	364.82	226.03	364.97	226.94	365.24	231.76	365.07
236.6	365.14	237.09	365.14	239.47	365.17	241.84	365.18	243.39	365.19
243.83	365.2	244.23	365.21	244.67	365.21	246.45	365.22	248.75	365.25
249.61	365.26	253.19	365.28	255.11	365.3	256.07	365.31	261.43	365.34
266.15	365.36	277.29	365.43	279.95	365.44	282.35	365.45	286.15	365.47
295.09	365.5	296.52	365.51	297.24	365.5	306.15	365.5	307.42	365
308.13	364.91	308.85	364.86	309.64	364.8	311.85	364.81	312.29	364.84
312.95	364.88	313.35	364.93	313.9	365	314.79	365.34	315.19	365.5
316.36	365.51	316.73	365.51	319.46	365.5	329.36	365.4	334.03	365.35
334.92	365.33	335.39	365.32	336.11	365.3	342.88	365.31	346.5	365.31
347.41	365.32	347.97	365.32	349.43	365.33	350.06	365.33	350.67	365.34

351.22	365.34	352.21	365.35	352.81	365.36	353.12	365.36	353.64	365.37
354.88	365.38	355.31	365.38	355.88	365.39	356.43	365.4	357.09	365.41
357.65	365.42	358.21	365.42	358.89	365.43	360.47	365.44	361.23	365.44
361.87	365.45	363.09	365.47	364.26	365.5	365.58	365.5	366.2	365.51
368.52	365.51	371.11	365.05	373.55	365.5	375.66	365.5	376.31	365.47
376.79	365.47	377.37	365.46	378.34	365.46	378.88	365.47	379.62	365.49
381.22	365.51	382.61	365.52	383.8	365.53	390.93	365.66	394.26	365.86
396.96	366	398.01	366.21	398.54	366.32	400.98	366.71	402.75	367
404.57	367.29	407.08	367.7	408.41	367.74	410.07	367.36	410.97	367.04
411.87	366.66	413.13	366.12	414.45	365.95	414.92	365.91	426.08	365.62
429.56	365.62	432.96	365.63	440.01	365.63	440.48	365.65	441.07	365.66
441.92	365.68	443.4	365.68	444.47	365.7	447.45	365.7	448.67	365.72
449.17	365.2	450.93	365.72	451.37	365.73	464.29	365.73	466.67	365.72
469.28	365.72	470.47	366	472.27	366.59	473.51	367	475.53	367.67
476.55	368	478.8	368.74	479.59	369	486.81	369.16	488.01	369.2
488.96	369.23	489.34	369.25	503.59	369	521.46	369	522.36	368.72
523.15	368.43	523.66	368.22	524.16	368	526.33	367.08	528.02	366.3
528.56	366.05	528.97	365.87	530.9	365.75	531.26	365.76	532.94	365.77
533.62	365.76	534.13	365.76	534.63	365.75	535.29	365.75	536.37	365.74
542.18	365.5	543.09	365.3	544.05	365.09	545.3	365.24	546.53	365.5
548	365.54	548.52	365.55	549.25	365.54	551.78	365.54	552.83	365.55
554.25	365.56	554.77	365.56	555.98	365.58	557.13	365.59	558.26	365.6
559.11	365.6	560.25	365.61	561.53	365.62	562.97	365.62	563.28	365.63
568.34	365.63	569.12	365.64	583.27	365.64	586.88	365.63	594.93	365.63
601.49	365.83	602.53	365.83	606.71	365.82	607.05	366	609.09	367
611.12	368	613.04	368.95	614.79	369.81	615.18	370	615.58	370.19
616.31	370.55	617.21	371	618.28	371.53	619.16	371.96	634.67	372.74
648.57	373.44	655.93	373.95	670.07	374.9				

Manning's n Values num= 13

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	144.83	.07	218.99	.055	226.94	.07	306.15	.055
315.19	.07	368.52	.055	373.55	.07	448.67	.055	450.93	.07
542.18	.055	546.53	.07	601.49	.04				

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	306.15	315.19		80	116		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 10.5

INPUT

Description:

Station Elevation Data num= 175

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.37	1.15	378.36	30.47	376.82	45.17	376.11	63.35	374.63
73.05	373.81	85.31	373	116.88	373	118.88	372	119.27	371.81

120.88	371	122.88	370	123.93	369.48	124.89	369	125.86	368.52
126.65	368.12	127.71	367.6	128.9	367	129.82	366.54	130.9	366
132.55	365.18	132.91	365	134.08	364.51	136.99	364.51	141.69	364.5
169.84	364.5	170.53	364.51	175.62	364.51	177.8	364	179.64	363.92
181.06	364.17	181.41	364.26	182.36	364.5	185.4	364.5	186.3	364.45
187.74	364.42	188.75	364.39	190.35	364.37	190.74	364.38	191.94	364.38
192.75	364.37	194.79	364.37	196.17	364.38	198.63	364.38	198.96	364.39
201.37	364.39	202.27	364.4	205.65	364.43	214.59	364.49	215.6	364.5
222.4	364.53	223.86	364.54	228.71	364.54	233.1	364.55	239.95	364.57
240.7	364.57	243.67	364.58	249.82	364.59	252.3	364.6	257.37	364.61
259.79	364.62	268.28	364.62	270.01	364.6	272.64	364.59	278.47	364.53
281.31	364.51	282.05	364.51	283.09	364.5	283.42	364.5	284.23	364.17
284.66	364	286.5	363.74	287.44	363.66	287.98	363.61	288.54	363.66
289.34	363.73	291.12	364	292.26	364.46	292.96	364.51	296.14	364.51
297.88	364.5	301.53	364.51	303.12	364.5	305.16	364.47	306.35	364.45
309.32	364.41	310.4	364.44	322.04	364.44	324.05	364.45	325.52	364.45
334.39	364.47	337.12	364.47	338.13	364.48	342.32	364.48	345.2	364.49
349.52	364.49	354.28	364.5	358.88	364.5	361.73	364.51	363.77	364
366.46	364.51	370.76	364.52	375.57	364.53	380.43	364.54	383.96	364.55
393.36	364.55	394.22	364.56	399.37	364.56	400.19	364.57	401.88	364.57
402.63	364.56	404.85	364.56	405.98	364.55	406.84	364.55	407.74	364.54
408.67	364.54	409.62	364.53	409.97	364.53	410.62	364.52	411.64	364.52
412.18	364.51	413.9	364.51	414.57	364.52	414.99	364.52	415.67	364.5
417.64	364.06	419.89	364.46	420.77	364.51	424.81	364.51	425.85	364.52
427.03	364.53	428	364.54	428.35	364.54	428.93	364.55	429.57	364.55
430.54	364.57	431.61	364.58	432.54	364.59	432.84	364.6	433.79	364.6
434.68	364.61	436.98	364.61	438.73	364.62	442.11	364.62	443.79	364.63
449.58	364.63	450.86	364.64	460.31	364.64	463.94	364.65	466.64	364.73
468.3	364.73	468.98	364.74	474.4	364.74	475.53	365	476.65	365.56
477.54	366	478.24	366.34	479.56	367	481.39	367.9	482.24	368.33
483.6	369	485.08	369.73	485.61	370	486.19	370.29	488.01	371.19
489.97	371.37	491.04	371.46	497.09	371.33	499.9	371.31	503.12	371.44
506.27	371.73	516.56	372.53	525.51	373.08	528.72	373.28	539.11	373.96

Manning's n Values

num= 11

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	134.08	.07	175.62	.055	182.36	.07	283.09	.055
292.26	.07	361.73	.055	366.46	.07	414.99	.055	420.77	.07
474.4	.04								

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	283.09	292.26		78	62	43		.1	.3

CROSS SECTION

RIVER: Jones Falls

REACH: Jones Falls 3 RS: 10

INPUT

Description:

Station Elevation Data									
num= 152									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.65	.55	378.64	13	378.43	15.74	378.3	27.99	377.53
56.19	375.7	69.37	374.64	77.63	374.04	88.91	373	97.14	373
98.33	372.41	99.16	372	100.73	371.22	101.13	371.03	101.93	370.63
103.2	370	104.54	369.34	104.98	369.12	106.25	368.49	107.24	368
108.32	367.47	109.22	367.02	111.28	366	113.29	365.01	113.93	364.69
114.53	364.39	115.23	364.05	141.12	363.95	143.44	363.96	144.1	363.94
144.79	363.93	145.25	363.92	146.75	363.55	147.58	363.42	151.72	363.32
152.45	363.34	153.97	363.5	154.69	363.68	167.49	363.6	173.71	363.64
177.06	363.66	178.01	363.67	179.78	363.69	183.02	363.72	183.34	363.72
185.54	363.74	186.55	363.77	186.88	363.78	187.27	363.79	189.28	363.81
210.8	363.83	254.85	364	255.23	364	255.86	363.99	258.88	363.97
259.65	363.96	261.77	363.95	262.29	363.94	265.28	363.9	265.89	363.83
266.33	363.82	266.9	363.84	267.75	363.5	269.83	363.19	270.5	363.13
272.14	363.2	273.62	363.4	274.3	363.5	275.33	363.91	276.29	364.01
276.83	364.01	278.93	364	286.51	363.92	287.14	363.94	307.05	363.94
307.66	363.95	308.12	363.96	310.84	363.96	314.12	363.97	315.46	363.97
316.5	363.96	317.82	363.96	318.17	363.95	318.87	363.94	319.53	363.92
320.33	363.9	321.54	363.87	323.44	363.85	325.75	363.84	327.75	363.76
328.28	363.76	328.96	363.75	331.76	363.63	334.91	363.5	335.55	363.45
336.23	363.5	336.87	363.64	337.2	363.69	338.17	363.76	342.3	363.76
342.71	363.76	343.46	363.77	344.06	363.77	344.69	363.78	346.65	363.84
347.09	363.84	347.4	363.85	348.03	363.85	348.99	363.86	349.8	363.86
350.78	363.89	351.62	363.89	352.46	363.9	353.63	363.9	354.51	363.92
356.8	363.92	357.28	363.93	359.8	363.93	360.43	363.94	362.56	363.94
363.56	363.95	364.65	363.95	365.15	363.96	367.06	363.96	372.2	363.97
376.34	363.98	385.43	363.98	386.41	363.99	397.12	363.99	407.8	364
409.68	364.92	410.03	365	410.36	365.16	412.03	366	412.54	366.26
414.03	367	414.72	367.34	416.04	368	416.88	368.42	418.04	369
418.87	369.42	419.25	369.61	420.39	370.17	421.21	370.24	423.46	370.12
424.84	369.93	426.1	370.23	428.34	370.55	435.42	371.03	450.33	372
463.05	373.11	474.62	373.84						

Manning's n Values									
num= 9									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	115.23	.07	144.1	.055	154.69	.055	266.33	.055
275.33	.07	335.55	.055	336.87	.07	407.8	.04		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	266.9	275.33		140	165	170		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3      RS: 9

INPUT

Description:

Station Elevation Data									
num= 85									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.67	9.78	378.53	43.31	377.84	58.23	376.62	71.1	376.1
74.31	375.86	75.67	375.32	89.64	374.14	94.24	373.54	95.34	373
96.27	372.54	96.58	372.39	97.37	372	98.01	371.68	99.39	371
101.29	370.06	102.39	369.52	103.44	369	104.42	368.52	105.15	368.16
105.47	368	105.78	367.85	106.33	367.57	107.49	367	108.38	366.56
109.52	366	110.63	365.45	111.54	365	112.49	364.53	113.57	364
114.91	363.34	115.59	363	117.13	362.6	119.02	362.6	141.72	362.5
142.96	362.16	143.89	362	146.19	361.98	146.61	362	148.31	362.36
148.69	362.49	216.61	362.49	217.61	362.47	219.56	362.47	224.88	362.46
227.98	362.47	229.74	362.49	230.18	362.49	233.19	362.45	234.33	362
235.42	361.85	235.86	361.8	236.79	361.73	237.58	361.68	239.23	361.96
240.5	362.19	241.57	362.38	242.44	362.38	244.25	362.42	246.87	362.47
247.77	362.5	251.23	362.51	263.08	362.53	264.14	362.53	265.79	362.54
270.93	362.58	331.92	362.58	332.89	363.58	334.4	363.75	334.89	364
335.38	364.24	336.07	364.59	336.89	365	337.6	365.35	338.89	366
340.23	366.67	341.96	367.53	351.1	367.92	354.78	368.08	383.98	370
409.52	372	434.59	374	453.16	376	468.82	378	487.44	380

Manning's n Values									
num= 7									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	117.13	.07	141.72	.055	148.69	.07	233.19	.055
241.57	.07	331.92	.04						

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	233.19	241.57		93	123		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 8

INPUT

Description:

Station Elevation Data									
num= 77									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	378.06	9.78	377.91	24.98	377.26	31.55	376.69	76.81	371.01
79.31	370.54	98.37	367.67	104.3	366.76	105.86	366	106.62	365.63
107.9	365	109.26	364.34	109.95	364	110.62	363.67	111.99	363
113.31	362.35	114.03	362	115.34	361.51	116.04	361.5	156.58	361.5
157.08	361.4	158.43	361	161.55	361	162.65	361.09	163.48	361.38
163.87	361.38	171.94	361.16	176.89	361.25	185.45	361.23	185.94	361.23
187.34	361.22	189.85	361.19	191.09	361.18	192.85	361.18	194.43	361.17
195.36	361.16	196.61	361.17	203.58	361.11	204.77	361.12	205.45	361.14
206.85	361.19	207.91	361.22	211.95	361.11	212.53	361.24	213.09	361.24
214.08	361	215.4	360.83	216.13	360.78	217.11	360.72	217.72	360.73
218.99	360.81	219.5	360.85	220.65	361	221.72	361.42	222.69	361.5

239.74	361.5	240.76	361.49	242.22	361.49	242.57	361.51	242.96	361.51
245.37	361.51	303.05	361.5	304.87	362	308.87	364	309.8	364.46
310.87	365	311.35	365.24	312.33	365.73	317.35	366	351.17	368
371.09	370	388.6	372	405.71	374	425.87	376	443.07	378
461.81	380	486.33	382						

Manning's n Values num= 7

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	115.34	.07	156.58	.055	163.48	.07	213.09	.055
217.72	.07	303.05	.04						

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	213.09	221.72		109	102	67		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 7.2

INPUT

Description:

Station Elevation Data num= 43

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	376.16	7.08	376.15	17.84	375.76	28.97	375.07	48.26	373.06
65.48	371.03	90.66	367.11	110.49	365.02	114.57	363	115.41	362.58
116.57	362	118.39	361.1	120.04	360.5	120.94	360.5	191.28	360.5
234.61	360.5	249.56	360.5	249.88	360.37	250.82	360	252.05	359.82
252.76	359.71	253.25	359.66	253.63	359.63	254.39	359.62	255.25	359.7
257.37	360	258.59	360.48	267.01	360.5	267.43	360.5	285.18	360.5
306.51	360.5	307.92	361	309.89	361.98	311.82	362.94	314.62	364.34
315.95	365	319.79	366	348.06	368	367.55	370	389.66	372
405.57	374	427.67	376	448.9	378				

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	120.04	.07	249.56	.055	258.59	.07	306.51	.04

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	249.56	258.59		220	203	229		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 7.1

INPUT

Description:

Station Elevation Data num= 69



Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	373.23	33.53	372	51.65	371.16	79.25	370.01	92.6	369.45
101.24	369.05	152.88	366.28	154.06	366.22	157.67	366.03	163.41	363.17
165.77	362	167.7	361.04	168.74	360.52	169.8	360	171.14	359.33
171.8	359	180.18	358.5	182.09	358.5	184.88	358.5	188.84	358.5
189.86	358.5	194.67	358.5	195.55	358.5	200.46	358.5	230.72	358.5
248.95	358.5	250.81	358.5	273.24	358.5	310.3	358.5	339.51	358.5
339.96	358.5	341.97	358.52	343.59	358.51	346.21	358.5	346.63	358.35
347.62	358	348.6	357.88	353.5	357.84	354.93	358	356.07	358.39
357.34	358.48	376.48	358.48	380.86	358.49	382.81	358.47	383.83	358.48
384.53	358.49	454.05	358.5	455.78	359	456.18	359.2	457.8	360
458.58	360.39	459.82	361	460.84	361.51	461.45	361.8	461.84	362
463.14	362.64	463.86	363	464.9	363.51	465.88	364	466.7	364.41
467.72	364.91	469.59	365.07	485.13	366.44	504.37	368.45	508.29	368.71
520.23	370	546.62	372	585.38	374	616.46	376		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	180.18	.07	346.21	.055	357.34	.07	454.05	.04

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	346.21	357.34		147	177	171		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 7

INPUT

Description:

Station Elevation Data num= 64

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	373.38	12.14	373.16	12.95	373.13	17.18	372.96	59.12	371.15
67.45	370.74	88.82	369.59	120.8	367.9	137.36	366.93	154.78	365.77
167.41	364.54	169.43	363.64	170.86	363	172.12	362.43	173.08	362
173.96	361.61	175.31	361	176.06	360.67	177.55	360	178.89	359.4
179.78	359	182.01	358	182.93	357.59	183.37	357.39	184.06	357.07
186.44	356.99	258.45	357	304.38	357	333.32	357	333.96	357
335.51	356.5	336.74	356.34	338.63	356.21	339.94	356.28	340.42	356.31
340.75	356.33	341.38	356.41	342.03	356.5	343.23	356.98	347.34	356.99
351	357	351.55	356.99	361.74	356.98	362.34	356.99	367.49	357
426.75	357	502.51	357	503.41	357.67	504.5	358	505.68	358.59
506.51	359	507.98	359.74	508.51	360	510.3	360.9	511.37	361.43
512.9	362.19	525.38	363.79	526.4	363.92	567.05	367.63	581.86	369.03
606.44	370.72	637.38	374.55	649.9	376	664.56	378		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	186.44	.07	333.96	.055	343.23	.07	502.51	.04

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
333.96	343.23	213	234	230		.1	.3

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 6.5

INPUT

Description:

Station Elevation Data	num=	68
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
0 374.54 8.08 374.32 32.74 373.78 43.18 373.67 46.53 373.62		
66.59 371.89 79.18 371.12 93.25 370 94.27 369.96 98.02 369.68		
117.07 368.43 124.62 367.87 139 366.24 163.32 363.88 171.42 363.03		
184.51 361.41 201.18 360.1 204.42 359.97 228.36 359.03 228.68 359.02		
238.79 358.27 247.95 357.91 248.73 357.88 252.42 356.06 254.57 355		
255.13 354.99 293.28 354.51 293.97 354.5 297.26 354.04 297.77 354.03		
300.76 354.5 301.99 354.5 337.84 354.97 369.77 354.98 371.93 355		
372.35 355.16 374.25 356 376.45 356.98 378.8 357.04 393.88 357.24		
409.81 357.48 413.16 357.6 420.33 357.89 431.66 357.55 442.47 357.36		
471.28 357.42 474.12 357.42 476.69 357.39 496.79 357.23 523.89 357.51		
526.63 357.55 550.81 357.57 558.58 357.54 559.2 357.41 561.4 357.11		
565.4 356.58 566.27 356.69 572.08 357.21 573.67 357.62 577.15 357.69		
584.3 358.46 586.57 358.8 604.26 362.11 608.32 362.7 615.45 363.32		
618.32 364 642.63 366 677.67 368		

Manning's n Values	num=	5
Sta n Val Sta n Val Sta n Val Sta n Val Sta n Val		
0 .04 254.57 .07 293.97 .055 300.76 .07 371.93 .04		

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff	Contr.	Expan.
293.97	300.76	273	271	241		.1	.3

Ineffective Flow	num=	1
Sta L Sta R Elev Permanent		
420.33 677.67 357.89 F		

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 6

INPUT

Description:

Station Elevation Data	num=	81
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
0 369.21 .44 369.16 31.84 365.2 45.45 364.4 58.99 363.32		

61.77	362.83	68.58	362.38	78.06	361.85	81.92	362	85.78	361.6
105.28	358.42	113.57	356.96	115.29	356.28	117.75	356.24	123.55	356.09
148.61	355.61	171.77	356.49	176.97	356.8	195.14	356.96	197.78	357.01
204.4	356.99	227.46	357.15	234.14	357.18	245.38	357.45	260.69	357.57
262.71	357.47	281.48	356.31	284.5	356.22	290.47	356.03	293.88	355.92
314.03	355.02	314.58	355	319.02	354.5	319.76	352.22	319.97	351.99
321.14	351.7	321.57	351.66	324.58	351.9	325.21	351.86	332.41	351.59
335.5	351.52	336.05	351.76	337.65	353.56	338.63	353.61	340.26	353.7
342.46	353.82	346.69	354.04	355.6	354.7	364.82	355.47	375.63	355.05
378.57	354.93	391.58	354.95	399.63	354.95	400.65	354.96	411.03	355.06
414.21	355.09	414.59	355.09	423.2	355.19	435.1	354.89	446.74	355.03
452.75	355.04	471.93	355.12	474.95	355.14	478	355.19	504.7	355.15
510.63	355.06	511.02	355.07	522.06	355.21	536.46	355.28	538.09	355.1
541.87	355.24	566.01	355.05	577.12	355.03	581.35	355	609.07	355.15
616.82	355.17	639.36	355.17	663.67	357.45	681.8	359	703.74	362
760.76	364								

Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	31.84	.11	319.02	.049	337.65	.11

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 319.02 337.65 122 82 93 .1 .3

Ineffective Flow num= 2  
 Sta L Sta R Elev Permanent  
 0 260.69 357.57 F  
 672.5 760.76 364 F

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 5

INPUT

Description:

Station Elevation Data num= 84

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	365.64	31.93	365.36	41.59	364.7	43.55	364.49	54.4	363.66
60.83	363.32	70.36	364.02	79.54	363.88	88.3	363.87	103.77	363.76
109.4	363.27	114.85	360.87	128.72	358.03	133.94	357.55	150.5	356.46
151.42	356.08	151.54	356.16	152.32	356.51	154.71	354.98	155.46	354.25
157.15	354.16	157.16	354.51	158.44	354.8	158.8	354.97	162.81	356.05
165.6	356.17	169.47	356.65	174.56	357.27	202.46	357.54	221.96	357.13
229.01	356.37	265.13	356.89	271.6	356.8	297.25	356.93	299.31	356.95
303.73	356.98	306.05	356.94	312.06	356.82	332.55	355.51	335.48	353.21
337.33	351.42	337.88	351.18	343.34	351.45	348.32	351.22	353.53	351.27
356.18	351.29	357.52	351.24	358.81	351.55	359.78	352.63	361.37	353.26
363.42	354.33	368.32	354.09	369.82	353.88	389.94	355.89	394.61	355.73
431.33	355.47	436.16	355.42	438.49	355.36	443.36	355.33	471.08	355.08

484.01	355.07	500.1	354.99	509.6	355.08	522.13	354.7	536.9	354.7
543.3	354.16	544.67	354.21	549.67	354.19	563.81	353.97	565.69	354.09
581.39	355.16	605.31	354.89	612.72	354.98	644.44	355	673.96	355.99
675.6	355.99	677.42	355.98	692.34	356.8	708.36	357.74	719.93	358.96
758.69	362	794.3	362.65	868.21	364	953.07	366		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	103.77	.11	332.55	.049	363.42	.11	794.3	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

332.55	389.94	132	149	230	.3	.5
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Ineffective Flow num= 6

Sta L	Sta R	Elev	Permanent
0	70.36	364.02	F
70.36	151.7	363	F
151.7	163.09	357.54	F
163.09	308.17	363	F
385.08	527.24	363	F
576.78	953.07	363	F

CULVERT

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 4.5

INPUT

Description: Bridge over Jones Falls on Park Heights Ave.  
 Distance from Upstream XS = 53  
 Deck/Roadway Width = 46  
 Weir Coefficient = 2.6

Upstream Deck/Roadway Coordinates

num= 14

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0		367			27.063		367			59.34		367		
124.7		366			201.23		365			305.74		364		
360.23		363			360.23		363			382.84		363		
382.84		363			464.76		363			736.72		363		
839		363			883.95		364							

Upstream Bridge Cross Section Data

Station Elevation Data num= 84

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	365.64	31.93	365.36	41.59	364.7	43.55	364.49	54.4	363.66
60.83	363.32	70.36	364.02	79.54	363.88	88.3	363.87	103.77	363.76
109.4	363.27	114.85	360.87	128.72	358.03	133.94	357.55	150.5	356.46
151.42	356.08	151.54	356.16	152.32	356.51	154.71	354.98	155.46	354.25
157.15	354.16	157.16	354.51	158.44	354.8	158.8	354.97	162.81	356.05
165.6	356.17	169.47	356.65	174.56	357.27	202.46	357.54	221.96	357.13

229.01	356.37	265.13	356.89	271.6	356.8	297.25	356.93	299.31	356.95
303.73	356.98	306.05	356.94	312.06	356.82	332.55	355.51	335.48	353.21
337.33	351.42	337.88	351.18	343.34	351.45	348.32	351.22	353.53	351.27
356.18	351.29	357.52	351.24	358.81	351.55	359.78	352.63	361.37	353.26
363.42	354.33	368.32	354.09	369.82	353.88	389.94	355.89	394.61	355.73
431.33	355.47	436.16	355.42	438.49	355.36	443.36	355.33	471.08	355.08
484.01	355.07	500.1	354.99	509.6	355.08	522.13	354.7	536.9	354.7
543.3	354.16	544.67	354.21	549.67	354.19	563.81	353.97	565.69	354.09
581.39	355.16	605.31	354.89	612.72	354.98	644.44	355	673.96	355.99
675.6	355.99	677.42	355.98	692.34	356.8	708.36	357.74	719.93	358.96
758.69	362	794.3	362.65	868.21	364	953.07	366		

Manning's n Values num= 5

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	103.77	.11	332.55	.049	363.42	.11	794.3	.045

Bank Sta: Left Right Coeff Contr. Expan.

332.55	389.94	.3	.5
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Ineffective Flow num= 6

Sta L	Sta R	Elev	Permanent
0	70.36	364.02	F
70.36	151.7	363	F
151.7	163.09	357.54	F
163.09	308.17	363	F
385.08	527.24	363	F
576.78	953.07	363	F

Downstream Deck/Roadway Coordinates num= 16

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
-67		370			-27		369			118.54		367		
183.84		366			260.22		365			364.65		364		
407.21		363.5			407.21		363.5			430.39		363.5		
430.39		363.5			523.52		363			793.58		363		
812.03		363			939.68		364			970		365		
1041.63		366												

Downstream Bridge Cross Section Data Station Elevation Data num= 27

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	362	77.33	360	130.4	358	170.95	356	257.66	355.34		
259.53	355.32	263.78	353.23	265.78	353.23	271.18	355.23	301.91	355		
363.95	355	388.75	354	400.53	352	422.68	352	429.75	354		
491.27	354	495.18	352.35	497.18	352.35	510.8	354	606.35	354.93		
620.43	355.07	716.33	356	847.36	358	928.57	360	1031.52	362		
1037.3	364	1040.91	366								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	77.33	.045	429.75	.049	716.33	.045

Bank Sta: Left Right Coeff Contr. Expan.  
 388.75 429.75 .3 .5

Ineffective Flow num= 4  
 Sta L Sta R Elev Permanent  
 0 259.53 363 F  
 271.18 391.95 363 F  
 449.03 456.73 363 F  
 516.56 1040.91 363 F

Upstream Embankment side slope = 4 horiz. to 1.0 vertical  
 Downstream Embankment side slope = 4 horiz. to 1.0 vertical  
 Maximum allowable submergence for weir flow = .98  
 Elevation at which weir flow begins = 363  
 Energy head used in spillway design =  
 Spillway height used in design =  
 Weir crest shape = Broad Crested

Number of Culverts = 3

Culvert Name Shape Rise Span  
 Culvert #4 Circular 2  
 FHWA Chart # 1 - Concrete Pipe Culvert  
 FHWA Scale # 1 - Square edge entrance with headwall  
 Solution Criteria = Highest U.S. EG  
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef  
 Exit Loss Coef  
 0 103.71 .013 .013 0 .5

1  
 Upstream Elevation = 354.16  
 Centerline Station = 156.31  
 Downstream Elevation = 353.23  
 Centerline Station = 264.78

Culvert Name Shape Rise Span  
 PHAve 24 N Box 9.2 22.5  
 FHWA Chart # 8 - flared wingwalls  
 FHWA Scale # 1 - Wingwall flared 30 to 75 deg.  
 Solution Criteria = Highest U.S. EG  
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef  
 Exit Loss Coef  
 28.7 52.9 .013 .013 0 .4

1  
 Upstream Elevation = 351.75  
 Centerline Station = 347.47  
 Downstream Elevation = 351.22  
 Centerline Station = 415.1

Culvert Name Shape Rise Span  
 PHAve 24 S Circular 2

FHWA Chart # 1 - Concrete Pipe Culvert

FHWA Scale # 1 - Square edge entrance with headwall

Solution Criteria = Highest U.S. EG

Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef  
Exit Loss Coef

18.14 128.15 .013 .013 0 .5

1

Upstream Elevation = 352.71  
Centerline Station = 548.25  
Downstream Elevation = 352.35  
Centerline Station = 496.18

CULVERT OUTPUT Profile #100 Yr Culv Group: Culvert #4

Q Culv Group (cfs)	38.92	Culv Full Len (ft)	103.71
# Barrels	1	Culv Vel US (ft/s)	12.39
Q Barrel (cfs)	38.92	Culv Vel DS (ft/s)	12.39
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	354.16
W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	353.23
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	3.07
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	1.62
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.19
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	361.60	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.67	Weir Sta Rgt (ft)	850.78
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	356.16	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	355.23	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	2.00	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	2.00	Min El Weir Flow (ft)	363.01

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

Note: Culvert critical depth exceeds the height of the culvert.

CULVERT OUTPUT Profile #100 Yr Culv Group: PHAve 24 N

Q Culv Group (cfs)	2366.86	Culv Full Len (ft)	
# Barrels	1	Culv Vel US (ft/s)	15.01
Q Barrel (cfs)	2366.86	Culv Vel DS (ft/s)	17.90
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	351.75
W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	351.22
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	0.18
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	4.29
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.42
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	363.67	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.66	Weir Sta Rgt (ft)	850.78

Culvert Control	Inlet	Weir Submerg	0.00
Culv WS Inlet (ft)	358.76	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	357.10	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	4.31	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	7.01	Min El Weir Flow (ft)	363.01

Warning: Since the culvert has supercritical flow, the program should be run in mixed flow in order to check if the cross section downstream of the culvert has supercritical flow.

Note: The flow in the culvert is entirely supercritical.

CULVERT OUTPUT Profile #100 Yr Culv Group: PHAve 24 S

Q Culv Group (cfs)	36.96	Culv Full Len (ft)	128.15
# Barrels	1	Culv Vel US (ft/s)	11.77
Q Barrel (cfs)	36.96	Culv Vel DS (ft/s)	11.77
E.G. US. (ft)	363.67	Culv Inv El Up (ft)	352.71
W.S. US. (ft)	363.67	Culv Inv El Dn (ft)	352.35
E.G. DS (ft)	357.78	Culv Frctn Ls (ft)	3.42
W.S. DS (ft)	357.02	Culv Exit Loss (ft)	1.39
Delta EG (ft)	5.89	Culv Entr Loss (ft)	1.08
Delta WS (ft)	6.64	Q Weir (cfs)	701.55
E.G. IC (ft)	359.56	Weir Sta Lft (ft)	323.09
E.G. OC (ft)	363.67	Weir Sta Rgt (ft)	850.78
Culvert Control	Outlet	Weir Submerg	0.00
Culv WS Inlet (ft)	354.71	Weir Max Depth (ft)	0.68
Culv WS Outlet (ft)	354.35	Weir Avg Depth (ft)	0.63
Culv Nml Depth (ft)	2.00	Weir Flow Area (sq ft)	334.27
Culv Crt Depth (ft)	2.00	Min El Weir Flow (ft)	363.01

Note: The normal depth exceeds the height of the culvert. The program assumes that the normal depth is equal to the height of the culvert.

Note: Culvert critical depth exceeds the height of the culvert.

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 4

INPUT

Description:

Station Elevation Data	num=	27							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	362	77.33	360	130.4	358	170.95	356	257.66	355.34
259.53	355.32	263.78	353.23	265.78	353.23	271.18	355.23	301.91	355
363.95	355	388.75	354	400.53	352	422.68	352	429.75	354



491.27	354	495.18	352.35	497.18	352.35	510.8	354	606.35	354.93
620.43	355.07	716.33	356	847.36	358	928.57	360	1031.52	362
1037.3	364	1040.91	366						

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.045	77.33	.045	429.75	.049	716.33	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

388.75	429.75	32	41	42	.3	.5
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Ineffective Flow num= 4

Sta L	Sta R	Elev	Permanent
0	259.53	363	F
271.18	391.95	363	F
449.03	456.73	363	F
516.56	1040.91	363	F

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 3.5

INPUT

Description:

Station Elevation Data num= 15

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	360	72.39	358	151.55	356	263.03	354	309.27	354
329.24	354	355.77	354	366.7	352	381.39	352	392.81	354
427.66	354	444.38	354	651.69	356	788.38	358	879.65	360

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.045	355.77	.049	392.81	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

355.77	392.81	129	124	114	.1	.3
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Ineffective Flow num= 3

Sta L	Sta R	Elev	Permanent
0	226.12	363	F
262.79	354.06	363	F
500.22	879.95	363	F

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 3 RS: 3

INPUT

Description:

Station Elevation Data									
num= 64									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	359.99	.34	360	4.87	359.83	24.26	359	26.66	359
52.98	358.47	65.8	358.24	73.86	358	123.46	357	126.73	356.9
147.04	356.21	153.61	356.09	156.52	356	189.59	355	248.58	354.22
255.72	354	272.26	354	279.04	353.8	287.01	353.16	289.84	353
305.32	353	321.86	354	365.33	354	386.15	352	393.78	352
405.44	354	495.82	354	627.55	356	636.55	356.74	640.53	356.29
646.19	355.47	650.59	355	655.67	355.54	659.28	356.05	664.62	356.59
670.81	356.56	672.36	356.45	678.66	356.48	685.44	356.76	685.95	356.76
692.7	357	698.53	358.25	701.09	358.21	706.73	359.24	746.51	357.92
748.84	357.15	762.88	357.08	763.95	357.11	776.92	358	790.11	357.97
790.96	358	791.93	357.96	803.2	357.94	804.99	357.86	807.07	357.93
816.28	357.9	819.03	358	833.07	358	837.35	358.15	842.45	358.5
852.49	358.81	855.53	358.8	861.14	359	882.29	359		

Manning's n Values					
num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.045	365.33	.049	405.44	.045

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	365.33	405.44		.1	.3

Ineffective Flow				
num= 1				
Sta L	Sta R	Elev	Permanent	
623.22	882.29	363	F	

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 4 RS: 2

INPUT

Description:

Station Elevation Data									
num= 67									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	359.14	7.06	358.91	17.67	358.06	19.63	358	34.93	357
38.91	356.87	47.9	356.15	50.24	356	70.75	355.83	79.5	355.13
81.37	355.02	91.99	354.86	157.37	354	172.68	353	186.5	353
187.53	352.98	189.15	352.89	198.15	352.17	203.29	352	356.33	352
389.23	352	394.41	351.02	399.85	350.31	402.25	350.23	408.45	351.42
410.46	351.84	417.55	352	498.36	352	506.01	351.89	509.38	352
567.66	352	569.7	351.97	570.6	351.92	572.93	351.92	580.32	351.82
585.91	352	631.82	352	633.4	352.05	636.96	352	647.13	352
662.43	353	706.27	353	708.35	352.94	709.99	352.95	718.33	352.83
723.65	353	738.96	353	739.56	352.98	740.92	352.87	754.26	352
769.57	353	775.57	353	782.02	353.09	792.64	353.75	800.17	354
803.25	354.3	813.87	355.84	815.48	356	819.64	356.14	824.48	356.21
830.78	356.59	844.87	358.84	845.72	358.94	846.09	358.96	847.05	358.97

856.33 358.83 857.88 358.89

Manning's n Values		num=		5					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.04	356.33	.045	389.23	.049	417.55	.045	498.36	.04

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	389.23	417.55		261	248	241		.1	.3

Ineffective Flow	num=		1					
Sta L	Sta R	Elev	Permanent					
662.43	857.88	353	F					

CROSS SECTION

RIVER: Jones Falls  
 REACH: Jones Falls 4 RS: 1

INPUT  
 Description:

Station Elevation Data		num=		34					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	356.15	2.31	356	17.6	356	23.22	355.82	26.04	355.67
36.66	355.63	45.22	355.2	48.19	355	50.53	354.92	57.9	354.55
77.84	354.03	78.78	354	109.37	353	124.67	353	132.26	352.75
139.96	352.73	149.5	352.38	153.5	352.06	155.26	352	201.14	351
262.33	350	338.83	350	349.44	349.18	351.13	348.88	352.83	349.24
363.44	350	434.53	350	636.49	350	664.25	350.27	667.02	350.05
668.56	350	717.1	350	793.76	352	821.31	354		

Manning's n Values		num=		4					
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val		
0	.045	338.83	.049	363.44	.045	434.53	.04		

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	338.83	363.44		.1	.3

SUMMARY OF MANNING'S N VALUES

River: Jones Falls

Reach		River Sta.	n1	n2	n3	n4	n5
n6	n7	n8	n9	n10	n11	n12	n13
n14	n15						

Jones Falls	15		.11	.049	.11	.04	
Jones Falls	14.5		.11	.049	.11	.04	
Jones Falls	14		.11	.049	.11	.04	
Jones Falls .11	13.6	.04	.11	.07	.055	.07	
Jones Falls .11	13.5	.04	.11	.07	.055	.07	
Jones Falls	13		.04	.07	.055	.07	.04
North Tributary .04	2		.04	.11	.07	.055	.07
North Tributary .04	1.5		.04	.11	.07	.055	.07
North Tributary	1		.04	.07	.055	.07	.04
South Tributary	8		.045	.057	.045		
South Tributary	7		.045	.057	.045		
South Tributary	6.5	Bridge					
South Tributary	6		.045	.057	.045		
South Tributary	5		.04	.07	.055	.07	.04
South Tributary	4		.04	.07	.055	.07	.04
South Tributary .04	3		.04	.055	.07	.055	.07
South Tributary	2		.04	.07	.055	.07	.055

	.07	.04							
IntsTrib			8	.013	.045	.046	.045	.013	
.045									
IntsTrib			7	.013	.045	.046	.045		
IntsTrib			6.5						
IntsTrib			6	.013	.04	.055	.04		
IntsTrib			5	.013	.04	.07	.055	.07	
.04									
IntsTrib			4.5	.013	.04	.07	.055	.07	
.04									
IntsTrib			4	.013	.04	.07	.055	.07	
.04									
IntsTrib			3	.04	.055	.04			
IntsTrib			2.5						
IntsTrib			2	.045	.046	.045	.013		
IntsTrib			1	.04	.046	.045			
Jones Falls 2			12	.04	.07	.055	.07	.055	
.07	.055	.07		.04					
Jones Falls 2			11.2	.04	.07	.055	.07	.055	
.07	.055	.07		.04					
Jones Falls 3			11.1	.04	.07	.055	.07	.055	
.07	.055	.07		.055	.07	.055	.07	.055	
.07	.04								
Jones Falls 3			11	.04	.07	.055	.07	.055	
.07	.055	.07		.055	.07	.055	.07	.04	
Jones Falls 3			10.5	.04	.07	.055	.07	.055	
.07	.055	.07		.055	.07	.04			

Jones Falls 3	10	.04	.07	.055	.055	.055	
.07	.055	.07	.04				
Jones Falls 3	9	.04	.07	.055	.07	.055	
.07	.04						
Jones Falls 3	8	.04	.07	.055	.07	.055	
.07	.04						
Jones Falls 3	7.2	.04	.07	.055	.07	.04	
Jones Falls 3	7.1	.04	.07	.055	.07	.04	
Jones Falls 3	7	.04	.07	.055	.07	.04	
Jones Falls 3	6.5	.04	.07	.055	.07	.04	
Jones Falls 3	6	.04	.11	.049	.11		
Jones Falls 3	5	.04	.11	.049	.11	.045	
Jones Falls 3	4.5	Culvert					
Jones Falls 3	4	.045	.045	.049	.045		
Jones Falls 3	3.5	.045	.049	.045			
Jones Falls 3	3	.045	.049	.045			
Jones Falls 4	2	.04	.045	.049	.045	.04	
Jones Falls 4	1	.045	.049	.045	.04		

SUMMARY OF REACH LENGTHS

River: Jones Falls

Reach	River Sta.	Left	Channel	Right
Jones Falls	15	306	304	297
Jones Falls	14.5	422	398	305
Jones Falls	14	145	146	147
Jones Falls	13.6	133	146	114
Jones Falls	13.5	196	219	219
Jones Falls	13			
North Tributary	2	164	165	175
North Tributary	1.5	173	177	182
North Tributary	1			
South Tributary	8	75	80	75
South Tributary	7	65	56	49
South Tributary	6.5	Bridge		
South Tributary	6	98.7695	77.1617	53.891
South Tributary	5	200.66	173.213	148
South Tributary	4	254.6731	254.1741	253.6741
South Tributary	3	220	242	244
South Tributary	2			
IntsTrib	8	135	134	134
IntsTrib	7	150	152	154
IntsTrib	6.5	Culvert		
IntsTrib	6	116	112	107
IntsTrib	5	179	181	181
IntsTrib	4.5	162	172	179
IntsTrib	4	202	222	233
IntsTrib	3	112	116	119
IntsTrib	2.5	Culvert		
IntsTrib	2	209	187	167
IntsTrib	1			
Jones Falls 2	12	71	119	122
Jones Falls 2	11.2			
Jones Falls 3	11.1	112	91	53
Jones Falls 3	11	80	116	60
Jones Falls 3	10.5	78	62	43
Jones Falls 3	10	140	165	170
Jones Falls 3	9	93	123	130
Jones Falls 3	8	109	102	67
Jones Falls 3	7.2	220	203	229
Jones Falls 3	7.1	147	177	171
Jones Falls 3	7	213	234	230
Jones Falls 3	6.5	273	271	241
Jones Falls 3	6	122	82	93
Jones Falls 3	5	132	149	230
Jones Falls 3	4.5	Culvert		

Jones Falls 3	4	32	41	42
Jones Falls 3	3.5	129	124	114
Jones Falls 3	3			
Jones Falls 4	2	261	248	241
Jones Falls 4	1			

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: Jones Falls

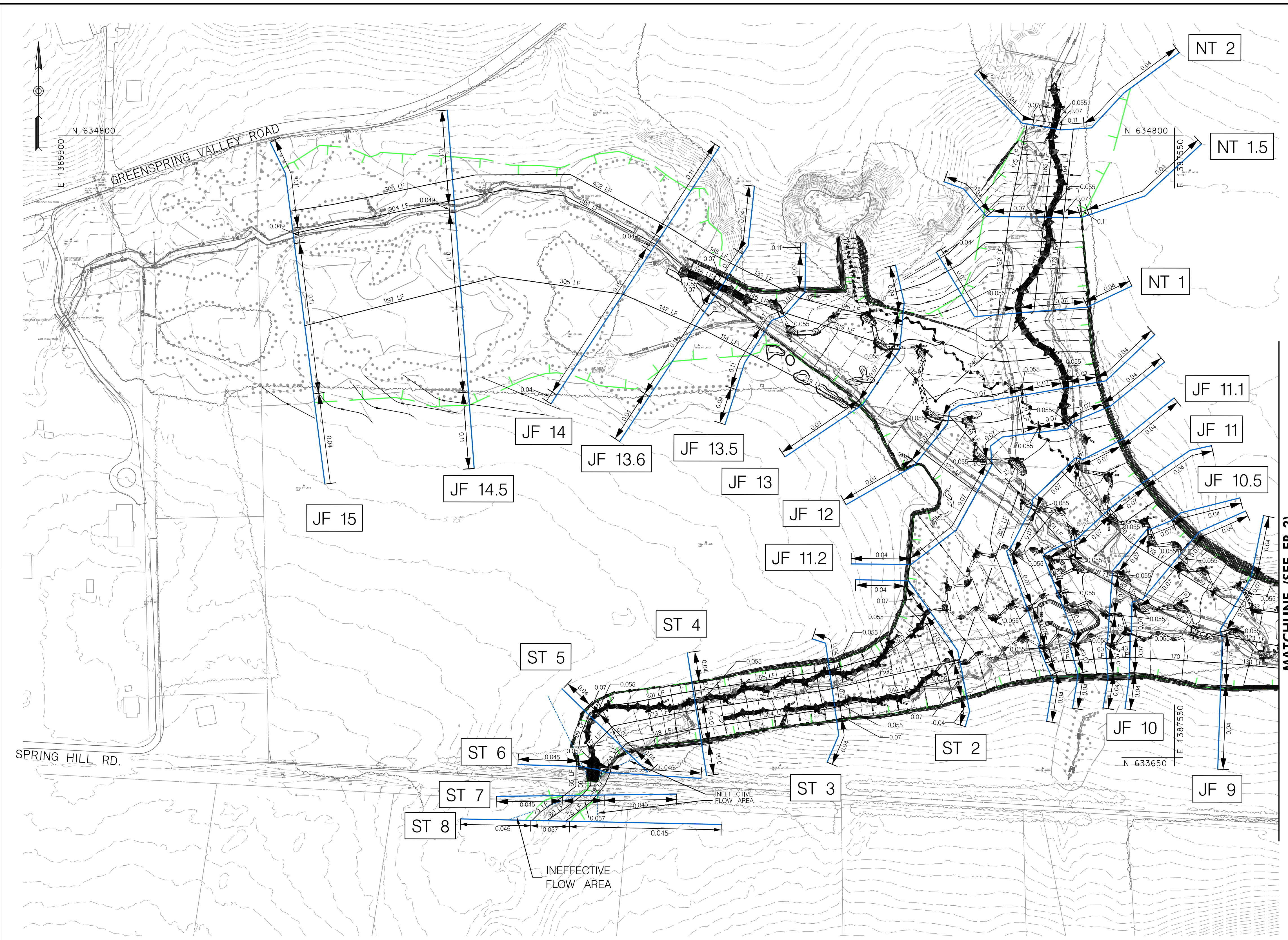
Reach	River Sta.	Contr.	Expan.
Jones Falls	15	.1	.3
Jones Falls	14.5	.1	.3
Jones Falls	14	.1	.3
Jones Falls	13.6	.1	.3
Jones Falls	13.5	.1	.3
Jones Falls	13	.1	.3
North Tributary	2	.1	.3
North Tributary	1.5	.1	.3
North Tributary	1	.1	.3
South Tributary	8	.1	.3
South Tributary	7	.3	.5
South Tributary	6.5	Bridge	
South Tributary	6	.3	.5
South Tributary	5	.1	.3
South Tributary	4	.1	.3
South Tributary	3	.1	.3
South Tributary	2	.1	.3
IntsTrib	8	.1	.3
IntsTrib	7	.3	.5
IntsTrib	6.5	Culvert	
IntsTrib	6	.3	.5
IntsTrib	5	.1	.3
IntsTrib	4.5	.1	.3
IntsTrib	4	.1	.3
IntsTrib	3	.3	.5
IntsTrib	2.5	Culvert	
IntsTrib	2	.3	.5
IntsTrib	1	.1	.3
Jones Falls 2	12	.1	.3
Jones Falls 2	11.2	.1	.3
Jones Falls 3	11.1	.1	.3
Jones Falls 3	11	.1	.3
Jones Falls 3	10.5	.1	.3
Jones Falls 3	10	.1	.3
Jones Falls 3	9	.1	.3



Jones Falls 3	8	.1	.3
Jones Falls 3	7.2	.1	.3
Jones Falls 3	7.1	.1	.3
Jones Falls 3	7	.1	.3
Jones Falls 3	6.5	.1	.3
Jones Falls 3	6	.1	.3
Jones Falls 3	5	.3	.5
Jones Falls 3	4.5	Culvert	
Jones Falls 3	4	.3	.5
Jones Falls 3	3.5	.1	.3
Jones Falls 3	3	.1	.3
Jones Falls 4	2	.1	.3
Jones Falls 4	1	.1	.3

**LEGEND**

- REACH LENGTHS —
- PROPOSED HEC-RAS CROSS SECTIONS —
- PROPOSED FLOODPLAIN —
- INEFFECTIVE FLOW AREA - - -



MATCHLINE (SEE FP-2)

OWNER / DEVELOPER INFORMATION  
 MARYLAND TRANSPORTATION AUTHORITY  
 2310 BRIDGING HWY  
 BALTIMORE, MD 21224

MARYLAND COORDINATE SYSTEM - HOR. NAD 8391 MD STATE PLANE VERT. NAVD 88

GREENSPRING VALLEY ROAD  
 SW CORNER PARK HEIGHTS AVE  
 OWINGS MILLS, MD 21117

**FLOODPLAIN MAP - PROPOSED CONDITIONS**

SCALE AS SHOWN	DATE JANUARY, 2021	PROJECT NO. 17-10977-002
MDE PROJECT NO. 21-SF-0044		CONTRACT NO. KH-3038-0000
DESIGNED BY PVC		COUNTY BALTIMORE COUNTY
DRAWN BY PVC		LOGMILE
CHECKED BY JJM /MRG		HORIZONTAL SCALE N/A
F.A.P. NO. N/A		VERTICAL SCALE N/A

REVISIONS  
 95%  
 SUBMISSION  
 NOT FOR  
 CONSTRUCTION

DRAWING NO. **FP - 1** OF **2** SHEET NO. OF 86

**ECCLESTON MITIGATION SITE**

DESIGN PROFESSIONAL  
 JEREMY KOSER  
 JOHNSON, MIRMIRAN & THOMPSON INC.  
 40 WIGHT AVENUE, HUNT VALLEY, MD 21030  
 TEL: 410-316-2360  
 EMAIL: JKoser@jmt.com

PROFESSIONAL CERTIFICATION  
 I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 31183, EXPIRATION DATE: 1/13/2023.





**PLAN**  
 SCALE: 1"=100'  
 100' 0 100' 200'  
 SCALE: 1"=100'

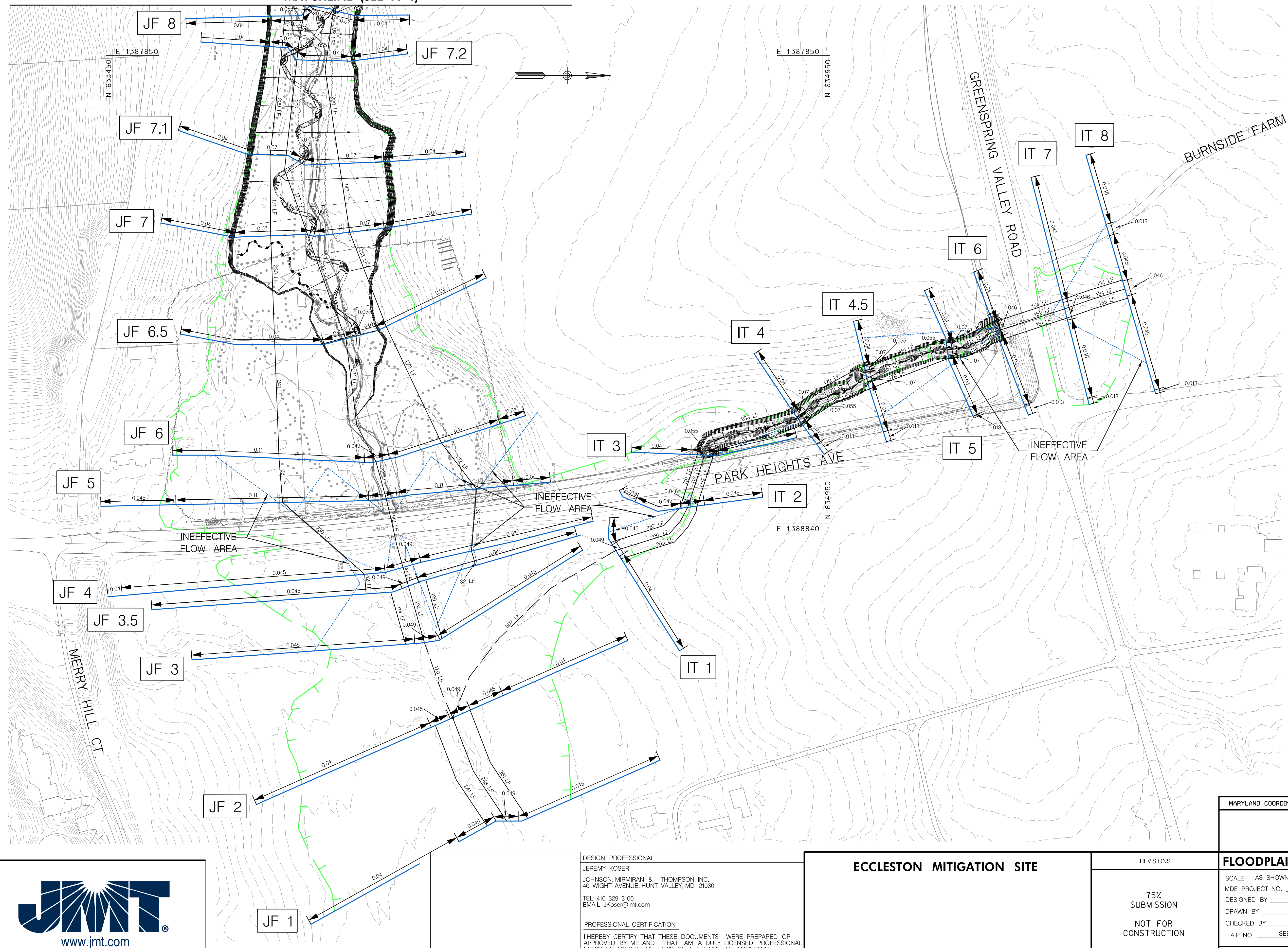


BY: PCrawford

MATCHLINE (SEE FP-1)

LEGEND

- REACH LENGTHS 
- PROPOSED HEC-RAS CROSS SECTIONS 
- PROPOSED FLOODPLAIN 
- INEFFECTIVE FLOW AREA 



PLAN  
SCALE: 1"=100'  
100' 0 100' 200'  
SCALE: 1"=100'

OWNER / DEVELOPER INFORMATION  
MARYLAND TRANSPORTATION AUTHORITY  
2310 BRACING HWY  
BALTIMORE, MD 21224

MARYLAND COORDINATE SYSTEM - HOR. NAD 83/91 MD STATE PLANE VERT. NAVD 88  
GREENSPRING VALLEY ROAD  
SW CORNER PARK HEIGHTS AVE  
OWINGS MILLS, MD 21117



BY: MOCCA

DESIGN PROFESSIONAL  
JEREMY KOSER  
JOHNSON, MIRIRAN & THOMPSON, INC.  
40 WIGHT AVENUE, HUNT VALLEY, MD 21030  
TEL: 410-329-3100  
EMAIL: JKoser@jmt.com

PROFESSIONAL CERTIFICATION  
I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 31183, EXPIRATION DATE: 1/13/2021.

ECCLESTON MITIGATION SITE

REVISIONS

75%  
SUBMISSION

NOT FOR  
CONSTRUCTION

FLOODPLAIN MAP - PROPOSED CONDITIONS

SCALE AS SHOWN DATE SEPTEMBER 2020 PROJECT NO. 17-10977-002  
MDE PROJECT NO. 21-SF-0044 CONTRACT NO. KH-3038-0000  
DESIGNED BY PVC COUNTY BALTIMORE COUNTY  
DRAWN BY PVC LOGMILE  
CHECKED BY JJM /MRG HORIZONTAL SCALE N/A  
F.A.P. NO. SEE TITLE SHEET VERTICAL SCALE N/A

DRAWING NO. **FP - 2** OF **2** SHEET NO. OF 49

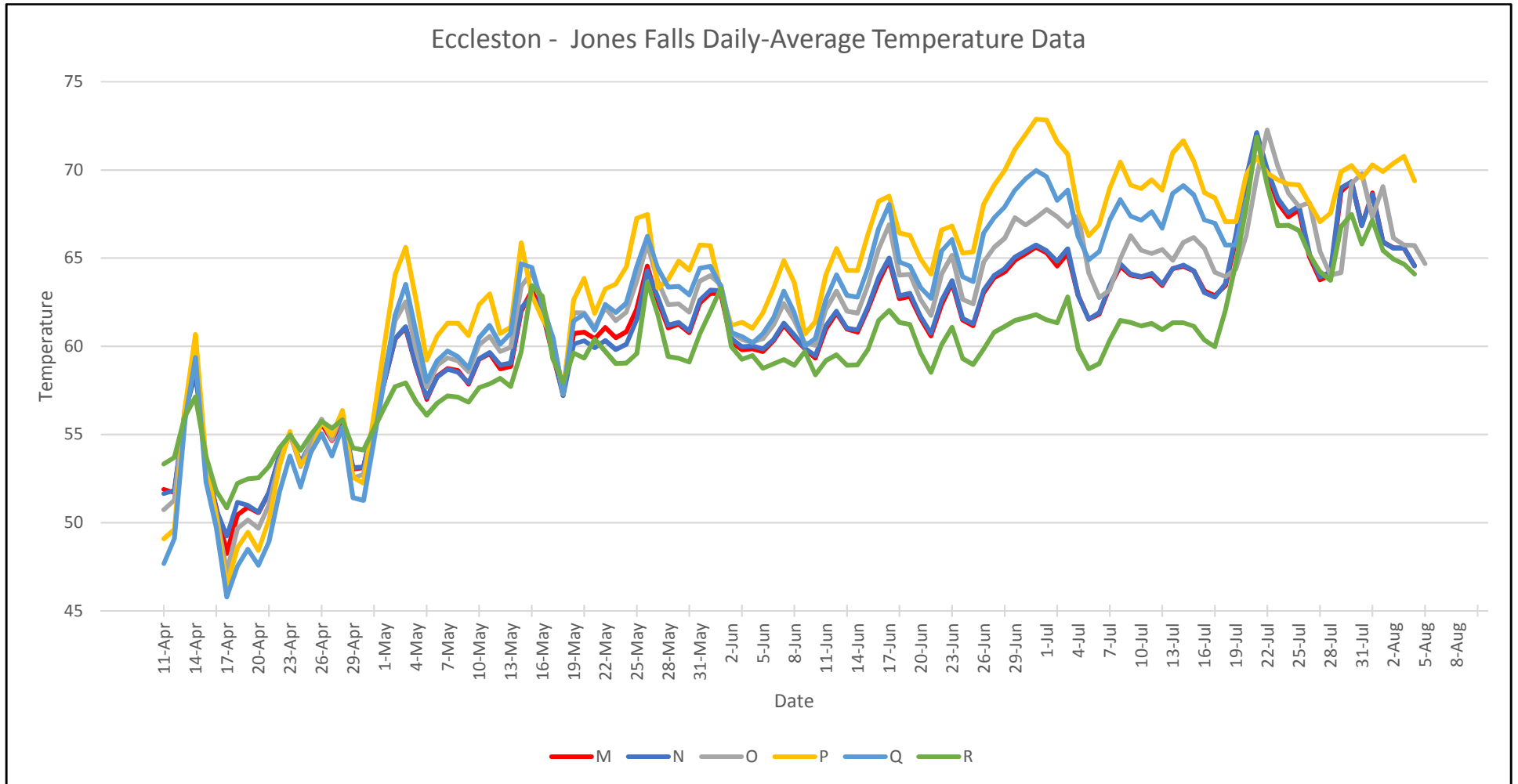
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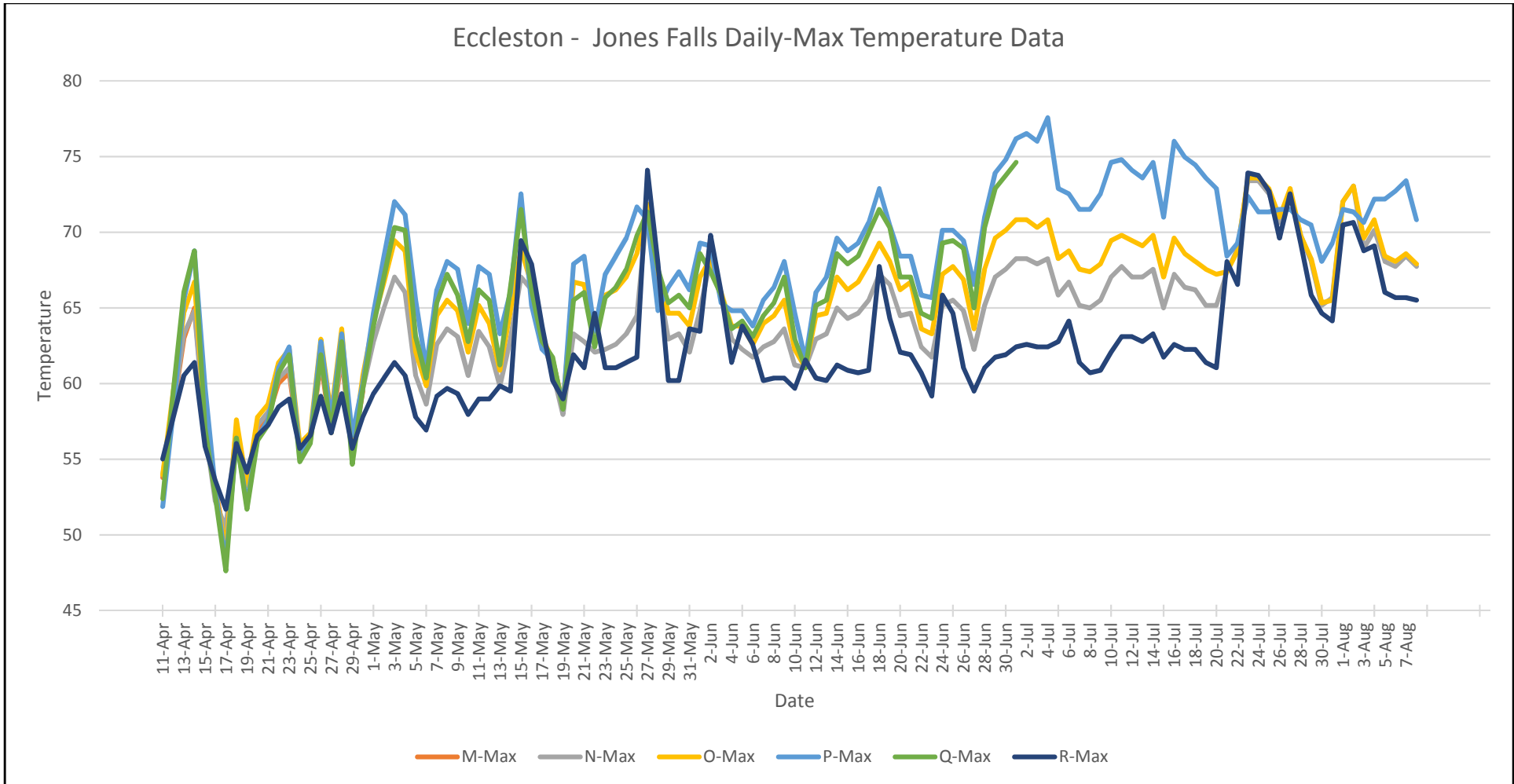
# APPENDIX I

## Temperature Gauge Locations Map and Data

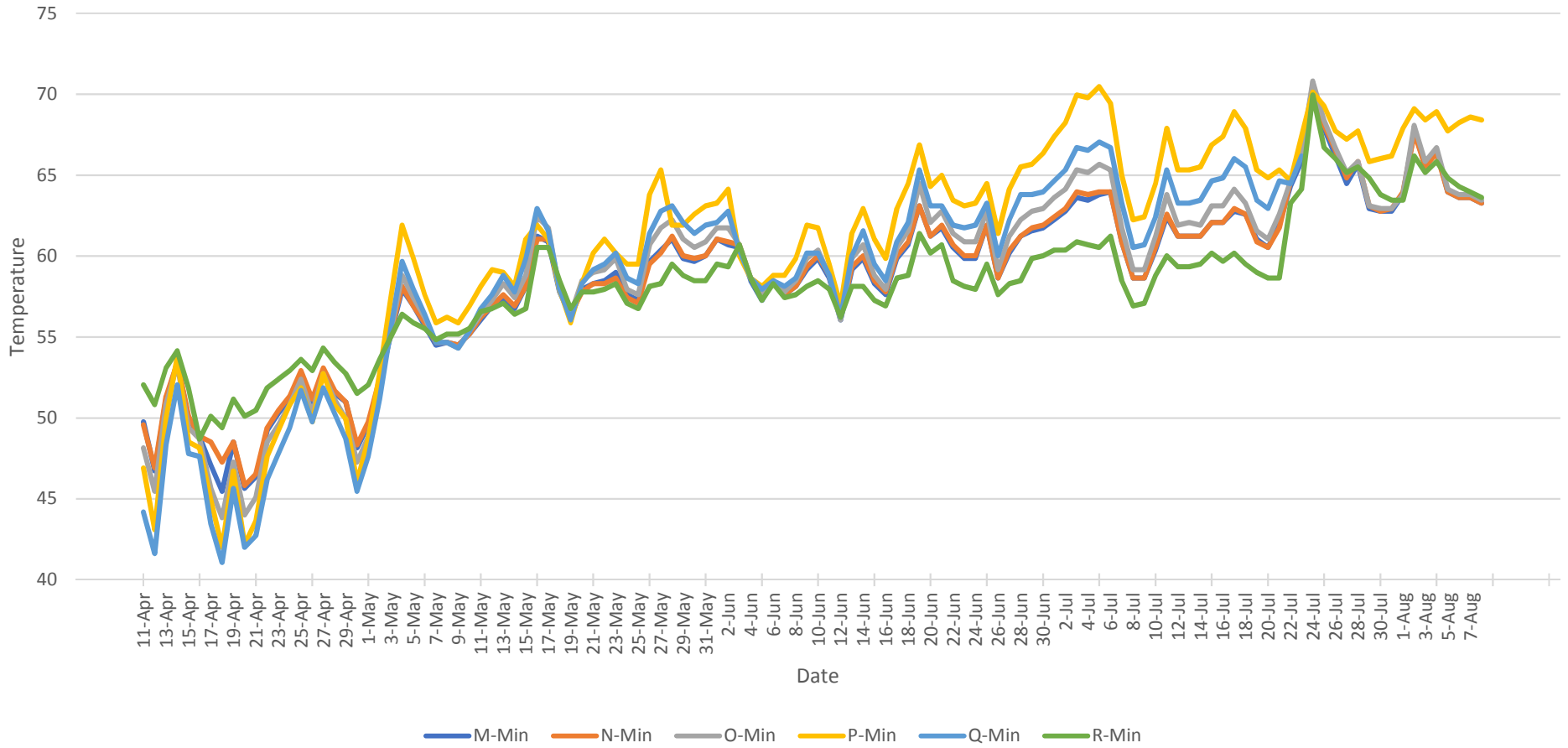
Eccleston - Jones Falls Daily-Average Temperature Data



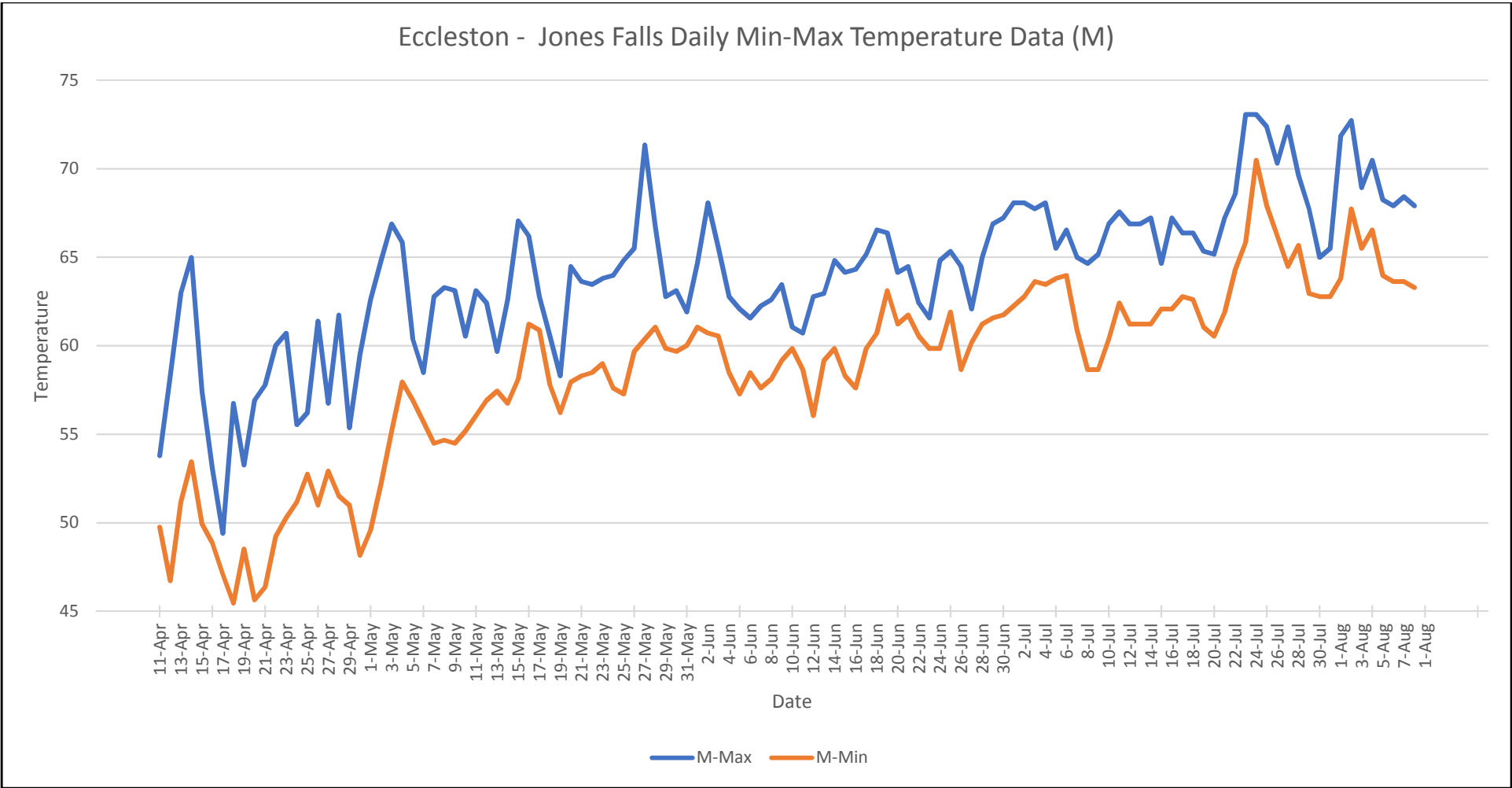
Eccleston - Jones Falls Daily-Max Temperature Data



Eccleston - Jones Falls Daily-Min Temperature Data

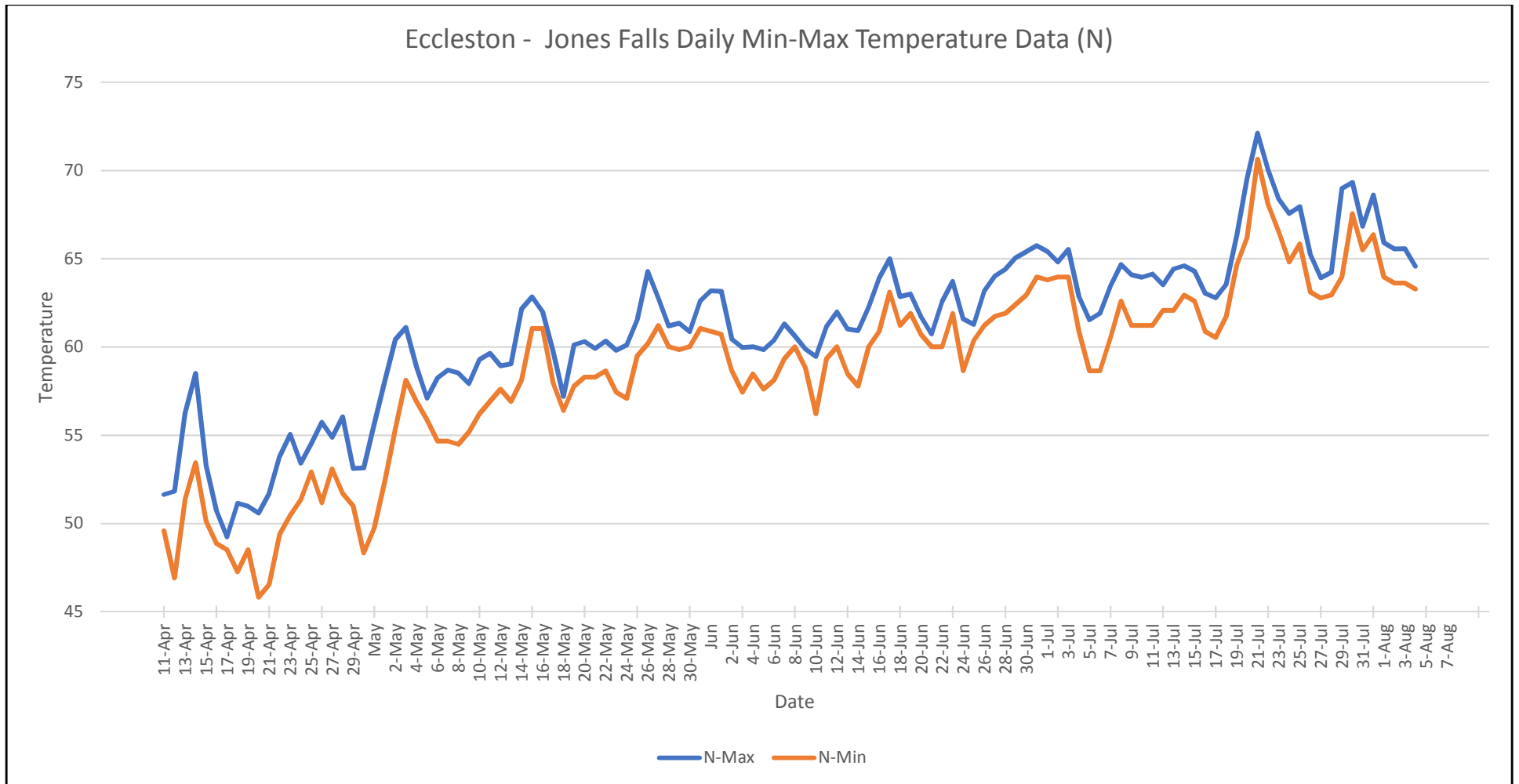


Eccleston - Jones Falls Daily Min-Max Temperature Data (M)

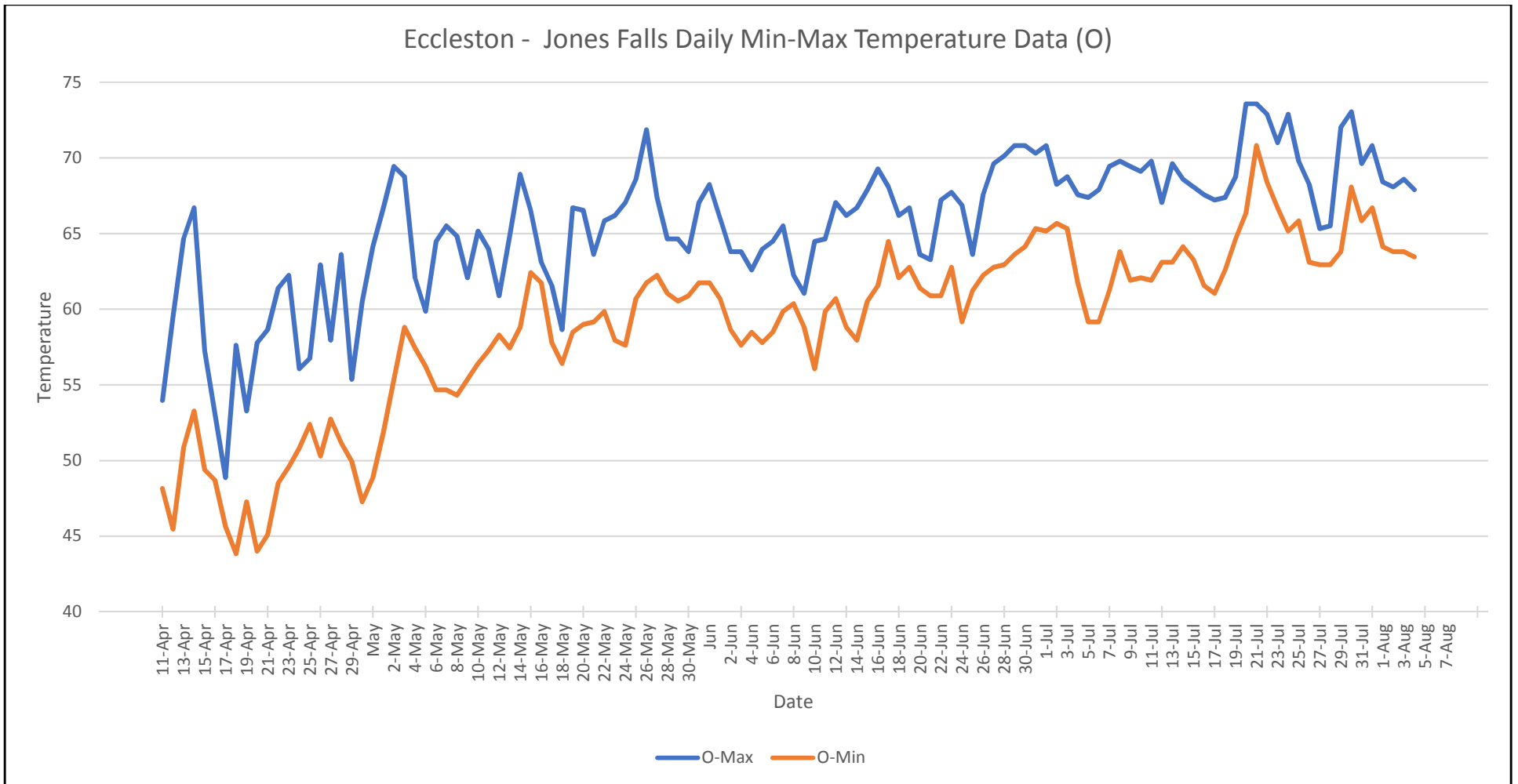




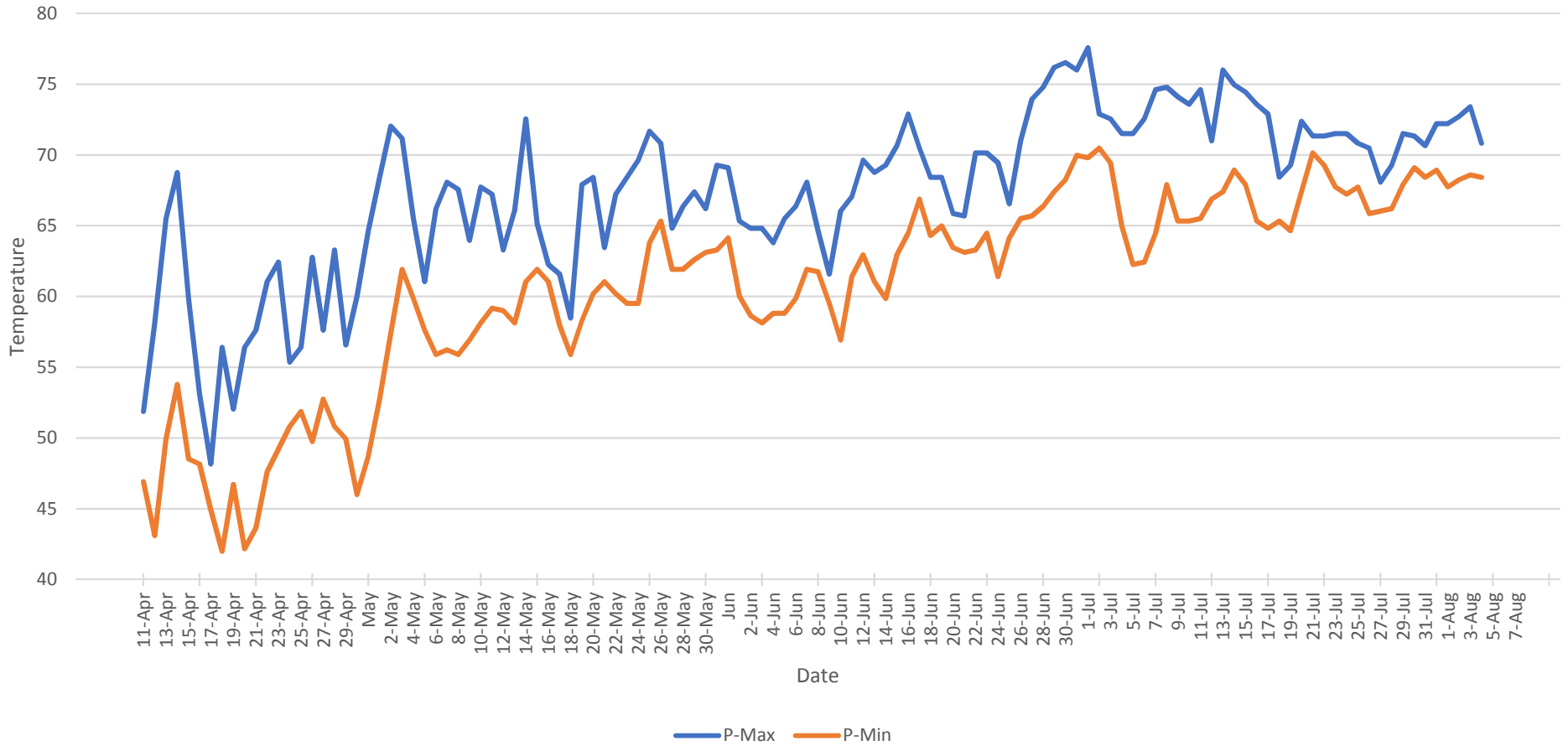
Eccleston - Jones Falls Daily Min-Max Temperature Data (N)



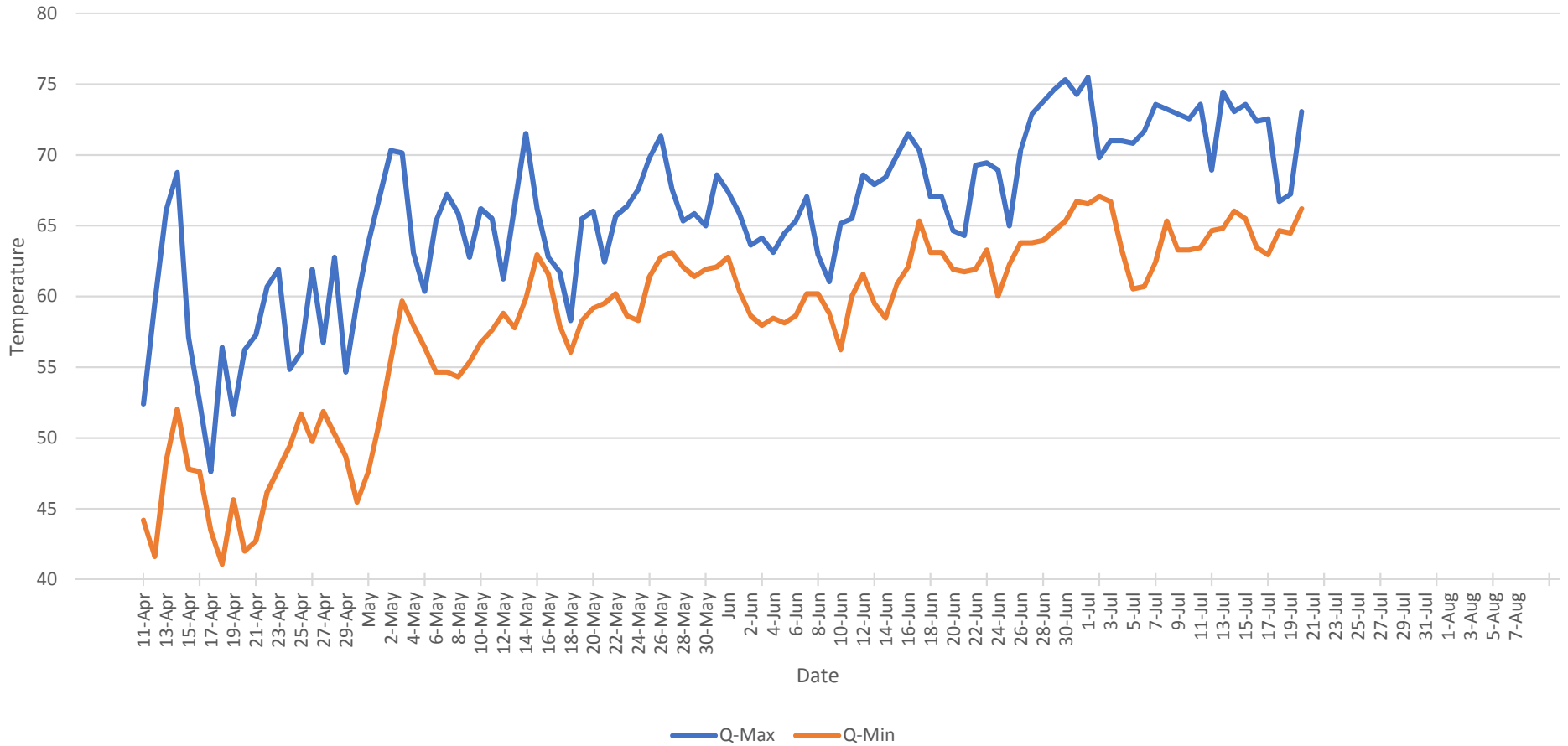
Eccleston - Jones Falls Daily Min-Max Temperature Data (O)



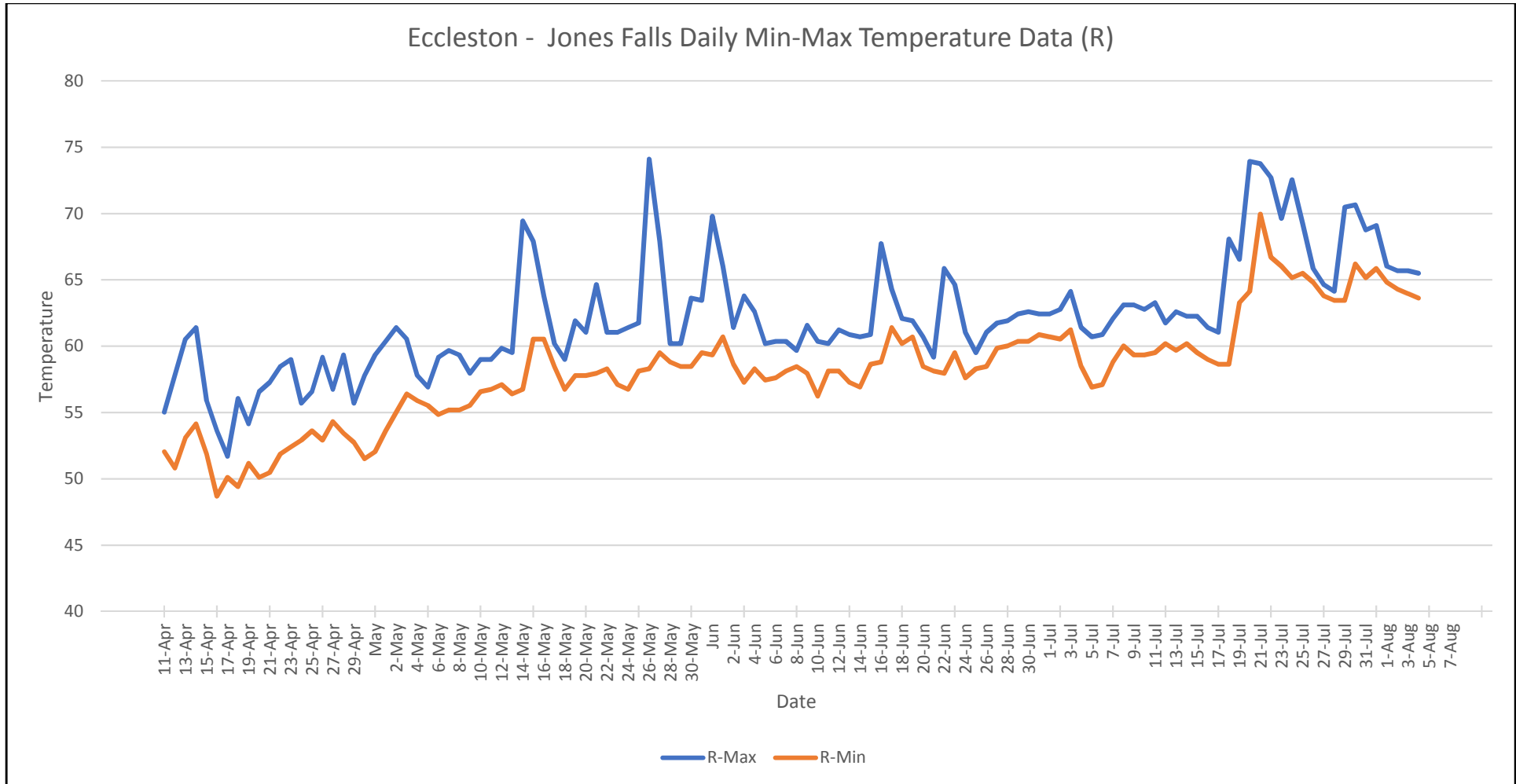
Eccleston - Jones Falls Daily Min-Max Temperature Data (P)



Eccleston - Jones Falls Daily Min-Max Temperature Data (Q)



Eccleston - Jones Falls Daily Min-Max Temperature Data (R)



Eccleston Temperature Gauge Locations





APPENDIX J  
Design Plans  
(under separate cover)



# APPENDIX K MAINTENANCE PLAN





## MAINTENANCE PLAN

The Maintenance Plan is a description and schedule of maintenance requirements to ensure the continued viability of the mitigation resources. The site owner shall continue with such maintenance activities until the Long-Term Steward assumes their responsibilities.

The following regular maintenance and bookkeeping will be conducted for the site, at a minimum:

- Conduct regular inspections of all mitigation areas, particularly during non-reporting years of site operation (annual inspections recommended, at a minimum).
- Maintain and repair all mitigation areas to meet or exceed the objectives and functions of the site, including all mitigation-related berms and structures.
- Proactively manage invasive species on the property.
- Ensure that no trespass, illegal dumping, or trash accumulation occurs on the property.
- Post and repair site / property limit and conservation easement signs.
- Maintain, repair, and/or replace gates and fences, as necessary.
- Maintain and repair direct access roads, as necessary.
- Other maintenance responsibilities to site operation and adaptive management.
- Remediation of Wetland Areas
  - i. Describe any problems observed within the mitigation site, such as excessive inundation, insufficient hydrology, seasonal drought conditions, invasion by undesirable species of plants or wildlife, disease condition for plants, poor plant establishment, adverse water quality impacts (i.e. excessive sediment loading, water pollution, etc.), human encroachment, and slope failures or erosion problems.
  - ii. Describe the proposed remedial measures to address the problems noted above.
  - iii. Remedial measures proposed are subject to review and approval by the regulatory agencies prior to implementation. In the event that remedial measures are implemented, the monitoring period may be extended on a case-by-case basis. The treatment of non-native invasive plant species does not need the approval of the agencies but should be completed at the correct time of year by someone with a current pesticide applicator certification and the required MDE toxic materials permit.
- Remediation of Buffer Areas
  - i. Describe any problems observed within the upland buffer, such as invasion by undesirable species of plants or wildlife, disease condition for plants, poor plant establishment, human encroachment, and slope failures or erosion problems.
  - ii. Describe the proposed remedial measures to address the problems noted above.
  - iii. Remedial measures proposed are subject to review and approval by the regulatory agencies prior to implementation. In the event that remedial measures are implemented, the monitoring period may be extended on a case-by-case basis. The treatment of non-native invasive plant species does not need the approval of the agencies but should be completed at the correct time of year by someone with a current pesticide applicator certification and the required MDE toxic materials permit.



# APPENDIX L MONITORING REQUIREMENTS AND PERFORMANCE STANDARDS

ECOLOGICAL PERFORMANCE STANDARDS AND MONITORING PROTOCOL FOR  
PERMITTEE-RESPONSIBLE NONTIDAL WETLAND MITIGATION SITES IN MARYLAND

October 30, 2020

Nontidal wetland permittee-responsible mitigation sites shall conform to the following performance standards by the end of the monitoring period, unless otherwise determined by the Maryland Department of the Environment (MDE) and the U.S. Army Corps of Engineers (Corps).

**I. Performance Standards:** All required documentation, including monitoring reports, construction completion reports, and as-built surveys shall be submitted to MDE and the Corps. MDE and the Corps will use visual observations during site visits and monitoring reports to evaluate attainment of performance standards and performance-based milestones and in determining whether part of or the entire mitigation site is successful or whether corrective actions are warranted. With the exception of standards for Invasive Species and Wetland Species Richness, success will be determined on a plot, well, field, or cell basis. Presenting averages or means of plot data across a site is not satisfactory to demonstrate success. All of the following standards and milestones will be used to assess project success and must be achieved each monitoring year.

**A. Wetland Area(s):**

- 1. Wetland Vegetation Dominance:** Wetland vegetation dominance, defined as a vegetation community where more than 50% of all dominant plant species across all strata are rated obligate (“OBL”), facultative wet (“FACW”), or facultative (“FAC”), using the vegetation sampling procedures as described in the appropriate regional supplement to the Corps of Engineers Wetland Delineation Manual, must be achieved; and
  
- 2. Aerial Cover Vegetative Standards:**
  - a) For sites that require monitoring in year one, a minimum of 50% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native (FAC or wetter) species.
  - b) By the end of year two, a minimum of 60% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native (FAC or wetter) species.
  - c) By the end of year three, a minimum of 70% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native (FAC or wetter) species.
  - d) By the end of year five and each monitoring year thereafter, a minimum of 85% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native (FAC or wetter) species.
  - e) Volunteer species should support functions consistent with the project design goals; and
  
- 3. Non-Native and Invasive Species:** The goal of any mitigation site is to have no non-native or invasive species. However, if non-native or invasive species are present, no more than 10% of relative plant cover<sup>1</sup> over the entire mitigation site shall be made up by non-native or invasive species, with no individual colony greater than or equal to 5% of relative plant cover. No more than 5% of relative plant cover over the entire site shall be made up of

---

<sup>1</sup> “Relative plant cover” is defined as the cover of a particular species as a percentage of total plant cover. Thus, relative cover will always total 100%, even when total absolute cover is quite low.

*Phragmites australis*<sup>2</sup>, *Persicaria perfoliata*, or *Lythrum salicaria*. Invasive species are identified on the 2010 National Park Service/U.S. Fish and Wildlife Service document *Plant Invaders of Mid Atlantic Natural Areas*<sup>3</sup> and the Maryland Invasive Species Council Invasive Species of Concern in Maryland<sup>4</sup>. Native status will be based on the Natural Resources Conservation Service Plants Database<sup>5</sup>. *Phalaris arundinacea* and *Typha* spp. may also be considered as invasive species by MDE and the Corps. Alternatively, the Permittee may propose appropriate reference sites as justification for MDE and the Corps to consider different site-specific standards for non-native and invasive species; and

**4. Wetland Species Richness:**

- a) For scrub/shrub wetlands, establish a minimum of three species of native wetland shrubs (FAC or wetter) with no more than 75% relative cover of one species, over the entire mitigation site. Loblolly pine cannot be more than 35% relative cover.
- b) For forested wetlands, establish a minimum of three species of native wetland trees and two species of native wetland shrubs (FAC or wetter) with no more than 75% relative cover of one species, over the entire mitigation site. Loblolly pine cannot be more than 35% relative cover; and

**5. Wetland Vegetation Density for Scrub-Shrub and Forested Wetlands:** For scrub-shrub or forested wetlands, native wetland (FAC or wetter) plant density of at least 435 living trees/shrubs per acre with a minimum height of 10 inches shall be achieved by the end of the first year a monitoring report is required and maintained each monitoring year thereafter through the end of the monitoring period; and

**6. Wetland Vegetation Cover for Forested Wetlands:** For forested wetlands, average tree height of tallest five native wetland (FAC or wetter) trees within each sample plot shall be at least three feet in height at year three and at least five feet in height at year five and each monitoring year thereafter. Canopy cover<sup>6</sup> of native wetland (FAC or wetter) trees and shrubs must be at least 30% by the end of the monitoring period; and

**7. Wetland Hydrology:** Wetland hydrology, defined as 14 consecutive days of flooding or ponding, or a water table 12 inches (30 cm) or less below the soil surface, during the growing season at a minimum frequency of 5 years in 10 (50 percent or higher probability). For the purpose of this determination, the growing season should be based on median dates (i.e., 50 percent probability) of 28°F air temperatures in spring and fall, based on the long-term data for the nearest appropriate weather station, as recorded in the WETS tables available from the NRCS National Water and Climate Center ([https://www.wcc.nrcs.usda.gov/climate/navigate\\_wets.html](https://www.wcc.nrcs.usda.gov/climate/navigate_wets.html)), or as specified in the appropriate regional supplement to the Corps of Engineers Wetland Delineation Manual; and

**8. Wetland Soils:** The entire wetland restoration or creation area must meet the Hydric Soil Technical Standard (Technical Note 11) developed by the National Technical Committee for

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<sup>2</sup> American Common Reed, *Phragmites australis* subsp. *americanus*, while uncommon, is not considered to be an invasive plant.

<sup>3</sup> <https://www.invasive.org/alien/pubs/midatlantic/midatlantic.pdf>

<sup>4</sup> <http://mdinvasives.org/species-of-concern/>

<sup>5</sup> <https://plants.sc.egov.usda.gov/>

<sup>6</sup> “Canopy cover” is defined as the percentage of ground covered by tree and shrub leaves, when the edges of the leaves are mentally projected down to the ground surface.

Hydric Soils for saturated conditions and anaerobic conditions:

- a) Free water must exist within 10 inches (25 cm) of the ground surface for at least 14 consecutive days; and
- b) Anaerobic conditions must exist within 10 inches (25 cm) of the ground surface for at least 14 consecutive days. Anaerobic conditions may be determined by one of the following methods, as detailed in the Hydric Soil Technical Standard:
  - (1) Positive reaction to alpha-alpha dipyrldyl, determined as least weekly.
  - (2) Reduction of iron determined with IRIS devices (tubes or films) installed for 30 days.
  - (3) Measurement of redox potential (Eh) using platinum electrodes, determined at least weekly.

**9. Delineation of Aquatic Resources:** At the mid-term monitoring year (year 3 for a 5-year monitoring period and year 5 for a 10-year monitoring period) and at the final year of the monitoring period, the wetland boundary area (established/re-established/restored/enhanced/preserved) as shown on the approved mitigation plan, shall be delineated using the wetland criteria outlined in the Corps of Engineers Wetlands Delineation Manual (1987) and appropriate regional supplement(s). Delineated wetlands shall be broken into projected vegetative type (e.g., emergent, scrub-shrub, forested) based on species present and density. In addition, all special aquatic sites, other waters, such as lakes and ponds, and all streams, within the approved mitigation site shall be identified and delineated. The delineated aquatic resource mitigation areas as verified by the MDE and/or Corps shall be consistent with the approved mitigation plan and contain at least as much wetland acreage and waterway linear feet as required in the permit. Deep water habitats and unvegetated areas that do not meet wetland criteria shall not be included in area measurements.

**10. Wetland function assessment:** An assessment of the specific wetland functions and values being provided should be conducted.

**B. Buffer Area(s):** The Buffer Area Performance Standards are only required to be met if the buffer is getting mitigation credit:

**1. Aerial Cover Vegetative Standards:**

- a) For sites that require monitoring in year one, a minimum of 50% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native species.
- b) By the end of year two, a minimum of 60% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native species.
- c) By the end of year three, a minimum of 70% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native species.
- d) By the end of year five and each monitoring year thereafter, a minimum of 85% of the mitigation site shall be vegetated (either by planted or volunteer plants) by native species.
- e) Volunteer species should support functions consistent with the project design goals; and

**2. Non-Native and Invasive Species:** The goal of any mitigation site is to have no non-native or invasive species. However, if non-native or invasive species are present, no more than 10% of relative plant cover<sup>1</sup> over the entire site shall be made up by non-native or invasive species, with no individual colony greater than or equal to 5% of relative plant cover. No more than 5% of relative plant cover over the entire site shall be made up of *Phragmites australis*<sup>2</sup>, *Persicaria perfoliata*, or *Pueraria montana*. Invasive species are identified on the

2010 National Park Service/U.S. Fish and Wildlife Service document *Plant Invaders of Mid Atlantic Natural Areas*<sup>7</sup> and the Maryland Invasive Species Council Invasive Species of Concern in Maryland<sup>8</sup>. Native status will be based on the Natural Resources Conservation Service Plants Database<sup>9</sup>. Alternatively, the Permittee may propose appropriate reference sites as justification for the IRT to consider different site-specific standards for non-native and invasive species; and

3. **Vegetation Density for Forested Buffers:** For forested buffers, native plant density of at least 435 living trees/shrubs per acre with a minimum height of 10 inches shall be achieved by the end of the first year a monitoring report is required and maintained each monitoring year thereafter through the end of the monitoring period.

## II. Monitoring Timeframe:

- A. The Permittee will be responsible for monitoring the mitigation site for a period of time specified in the Phase II Mitigation Plan Approval Letter, approved final mitigation plan, or the special conditions of the Permit. The Corps of Engineers' 2008 Mitigation Rule requires compensatory mitigation projects to have a minimum monitoring period of five years (33 CFR 332.6(b)). However, longer monitoring periods of more than 5 years are warranted for aquatic resources with slow development rates (e.g., vernal pools, riparian forest, forested wetlands, and coastal salt marsh).
- B. The monitoring period begins the year the mitigation planting occurs, unless planting occurs after April 15, in which case the monitoring period will not begin until the following year. For each monitoring report, vegetative monitoring shall be conducted between May 1 and September 30 for forested/scrub-shrub systems and between June 15 and September 30 for emergent systems. Site visits should preferably be during a period with normal precipitation and groundwater levels.
- C. Monitoring must be conducted a minimum of once per year during the years that monitoring reports are required. Certain mitigation projects may require more frequent monitoring (e.g., twice a year during spring and fall) and reporting during the early stages of development to quickly identify and address problems and/or concerns. The extent of monitoring may be terminated or reduced no earlier than the end of the fifth monitoring year over part or the entire site upon a determination by the Corps and MDE (as applicable) that the wetland mitigation site has achieved all performance-based milestones each monitoring year and all final performance standards for two consecutive monitoring events. Conversely, MDE and the Corps (as applicable) may extend the original monitoring period upon a determination that performance standards have not been met, the mitigation site is not on track to meet them (e.g., remediation or adaptive management required), or in consideration of the amount and distribution of precipitation prior to and during the growing season compared with a "typical year"<sup>10</sup>. If a natural disaster occurs during the monitoring period, remediation or adaptive management may

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<sup>7</sup> <https://www.invasive.org/alien/pubs/midatlantic/midatlantic.pdf>

<sup>8</sup> <http://mdinvasives.org/species-of-concern/>

<sup>9</sup> <https://plants.sc.egov.usda.gov/>

<sup>10</sup> "Typical year" is calculated by considering normal precipitation and other climatic factors over a rolling 30-year period at or near the project location. Determination of "typical year" should follow the methods described in the U.S. Environmental Protection Agency/Corps factsheet entitled "Typical Year and the Navigable Waters Protection Rule," including use of the antecedent precipitation tool (APT) (<https://github.com/jDeters-USACE/Antecedent-Precipitation-Tool>).

be required and the monitoring period may be extended.

**III. Monitoring Reports:** Monitoring reports should be concise and effectively provide the information necessary to assess the status of the compensatory mitigation project. Reports should provide information necessary, including supporting data such as plans, maps, and photographs, to illustrate site conditions and whether the compensatory mitigation project is meeting its objectives and performance standards. In accordance with federal requirements, all monitoring of mitigation sites required by the Corps must adhere to the minimum standards provided in Regulatory Guidance Letter 08-03 (attached).

**A.** Monitoring reports, a paper copy and an electronic version, must be submitted to MDE and the Corps by December 31 of each monitoring year. If five years of monitoring is required, monitoring reports shall be submitted annually. If ten years of monitoring is required, monitoring reports shall be submitted for years 2, 3, 5, 7, and 10 (“monitoring years”) following completion of construction and planting of the mitigation site or phase thereof.

**B. Content:** The following information must be included with the monitoring report:

**1. Monitoring and Performance Standards Summary Table** comparing the required performance standards to the conditions and status of the developing mitigation site must be completed and attached to the beginning of the Monitoring Report. The table will list the monitoring requirements and performance standards, as specified in the Phase II Wetland Mitigation Plan Approval Letter, and/or special conditions of the permit and evaluate whether the compensatory mitigation project site, including each area (plot, well, field or cell as appropriate), is successfully achieving the approved performance standards or trending towards success.

**2. Project Overview / Background Data:**

- a) Title page indicating the permit name, permit tracking number (Corps permit number and MDE NT number), mitigation site name (if applicable), phase (if applicable), monitoring year, and preparer identification (name, address, phone number, and email address).
- b) Written description of the location, any identifiable landmarks of the mitigation site, including information to locate the site perimeter(s), and coordinates of the mitigation site (expressed as latitude and longitude).
- c) Date(s) of site inspections.
- d) A brief paragraph describing the goals and objectives of the mitigation site, including the authorized impact acreage and type of aquatic resource impacted, proposed mitigation acreage and aquatic resource type approved as part of the Phase II Wetland Mitigation Plan or Permit. Include the dates the mitigation construction was started and the planting was completed.
- e) A brief narrative description of the mitigation site addressing its position in the landscape, adjacent waterbodies, and adjacent land use.
- f) Describe methods used to evaluate performance standards. Plot locations should be

clearly identified on the appropriate maps.

- g) A short statement on whether the performance standards are being met.
  - h) A narrative description of existing mitigation site conditions and functions and how the mitigation site has or has not achieved the goals, objectives and performance standards established for the project.
  - i) Dates of any recent corrective or maintenance activities conducted since the previous report submission.
  - j) If monitoring or site inspections were conducted between years of required monitoring (e.g., year four in a 10-year monitoring period), this data should also be included.
  - k) Specific recommendations for any additional corrective or remedial actions.
  - l) Estimate the percent of the mitigation site that is establishing into wetland and the type of wetland system (ex: forested, scrub-shrub, emergent). If this differs from what was planned, show the boundaries of the actual wetland area/types on the plans or maps.
  - m) Estimate the percent of the mitigation site buffer that is establishing into forested buffer. If this differs from what was planned, show the boundaries of the actual forested buffer area on the plans or maps.
  - n) Discussion of growing season and how it was determined for this site.
- 3. Summary data:** Summary data should be provided to substantiate the success and/or potential challenges associated with the compensatory mitigation project. Refer to Section IV below for monitoring report measurements to include for the overall site.
- 4. Photographs:** Take one set of photographs from established photographic points any time between May 1 and September 30 of each monitoring year (pictures should be taken at the same time of year when possible). Photo location points should be identified on the appropriate maps and labeled with the direction in which the photo was taken. Submitted photos should be formatted to print on a standard 8.5 by 11-inch piece of paper, dated, and clearly labeled with the direction from which the photo was taken. It is highly recommended that aerial photos are also provided, as these are good indicators of hydrology and vegetative cover.
- 5. Maps and Plans:** Maps should be provided to show the location of the compensatory mitigation site relative to other landscape features, habitat types, locations of photographic reference points, transects, sampling data points (e.g., vegetation plots, wells, soil samples, etc.), and/or other features pertinent to the mitigation plan. GPS coordinates should be shown on the plans for each photographic reference point and sample plot. In addition, the submitted maps and plans should clearly delineate the mitigation site perimeter(s), which will assist the project managers in locating the mitigation area(s) during subsequent site inspections. Each map or diagram should be formatted to print on a standard 8.5 by 11-inch piece of paper and include a legend and the location of any photos submitted for review. As-built plans should be included if they were not already submitted to MDE and the Corps.



6. **Conclusions:** A general statement shall be included that describes the conditions of the compensatory mitigation project. If performance standards are not being met, a brief explanation of the difficulties and potential remedial actions proposed by the Permittee, including a timetable, must be provided. MDE and the Corps will ultimately determine if the mitigation site is successful for a given monitoring period.

**IV. Monitoring Report Measurements.** Monitoring reports should include all of the following information for the overall site, and each plot, well, field or cell:

**A. Wetland Area(s):**

**1. Vegetation:**

- a) Estimate the actual and relative percent cover by plant species, in order of dominance, across all strata for each plot. Include this information in a table. For each species listed in the table include native/non-native status and wetland indicator status. Summarize the data by plot, field/cell and overall site.
- b) For scrub-shrub or forested wetlands, estimate the percent survival of planted trees and number of native wetland (FAC or wetter) trees/shrubs per acre (including volunteer woody species at least ten inches). Data should be summarized for each plot and also by field/cell and overall site. Please note that projects where the vegetation is inconsistent throughout the site may not meet the performance standards (e.g. a site where some portions have high densities of woody species but other portions have low densities).
- c) For scrub-shrub or forested wetlands, measure the height of the tallest five trees within each sample plot in each monitoring year. In the final year of monitoring, measure canopy cover of native wetland (FAC or wetter) trees and shrubs.
- d) Summarize the results from the vegetation plot study, including how the vegetation meets/does not meet performance standards. Data should be summarized for each plot, by field/cell, and for the entire site. Include a discussion of water movement into and through the site. **Do not include the raw plot data in your monitoring report.**

**2. Hydrology:**

- a) Estimate percent of site that is inundated or saturated to the surface on the dates of the site visits.
- b) Monitoring data for surface water and groundwater, including hydrograph of measured depth to water table, after calibrating for above-ground height of well. Data should be included for each well separately.
- c) Discuss how precipitation during this monitoring year compares with historical precipitation data for that location.
- d) Summarize results of the hydrology monitoring for each well, by field/cell, and for the entire site, including if each meets/does not meet the performance standards. Estimate percent of site that has wetland hydrology.

**3. Soils:**

- a) Monitoring data to determine if hydric soils are actively developing. Data should be included for each sample location. This must include evidence that saturated and anaerobic soil conditions are being met, as measured by alpha-alpha dipyrindyl, IRIS devices (tubes or films), or platinum electrodes.

- b) Provide a soil profile description with accompanying soil photos for each soil location tested above.
- c) Summarize results of the soil monitoring for each sample location, by field/cell, and for the entire site, including if each meets/does not meet the performance standards.

**4. Remediation:**

- a) Describe any problems observed within the mitigation site, such as: excessive inundation, insufficient hydrology, seasonal drought conditions, invasion by undesirable species of plants or wildlife, disease condition for plants, poor plant establishment, adverse water quality impacts (i.e., excessive sediment loading, water pollution, etc.), human encroachment, and slope failures or erosion problems.
- b) Describe any proposed or implemented remedial measures to address the problems noted above.
- c) Remedial measures proposed by the Permittee are subject to review and approval by MDE and the Corps prior to implementation. In the event that remedial measures are implemented, the monitoring period may be extended on a case-by-case basis. The treatment of non-native invasive plant species does not need the approval of MDE and the Corps, but should be completed at the correct time of year by someone with a current pesticide applicator certification and the required MDE toxic materials permit.

**B. Buffer Area(s):**

**1. Vegetation:**

- a) Estimate the actual and relative percent cover by plant species across all strata for each plot. Include this information in a table. For each species listed in the table, include native/non-native status. Summarize the data by plot, field/cell and overall site.
- b) For scrub-shrub or forested buffers, estimate the percent survival of trees and the number of native trees/shrubs per acre (including planted or volunteer woody species at least ten inches). Data should be summarized for each plot and also by field/cell and overall site. Please note that projects where the vegetation is inconsistent throughout the site may not meet the performance standards (e.g. a site where some portions have high densities of woody species but other portions have low densities).
- c) Measurements of vegetation based upon performance standard and methods used to evaluate the vegetative success of the mitigation site. **Do not include the raw plot data in your monitoring report.**

**2. Remediation:**

- a) Describe any problems observed within the upland buffer, such as: invasion by undesirable species of plants or wildlife, disease condition for plants, poor plant establishment, human encroachment, and slope failures or erosion problems.
- b) Describe the proposed remedial measures to address the problems noted above.
- c) Remedial measures proposed by the Permittee are subject to review and approval by MDE and the Corps prior to implementation. In the event that remedial measures are implemented, the monitoring period may be extended on a case-by-case basis. The treatment of non-native invasive plant species does not need the approval of MDE and the Corps, but should be completed at the correct time of year by someone with a current pesticide applicator certification and the required MDE toxic materials permit.

## **V. Adaptive Management Review**

- A.** The Permittee assumes all liability for performing approved measures through adaptive management strategies or alternative mitigation should MDE and the Corps or the Permittee determine the compensatory mitigation project is not meeting performance standards or satisfying the objectives of the compensatory mitigation project. The approved adaptive management plan will guide decisions for revising mitigation plans and implementing measures to address circumstances (foreseeable and unforeseen) that adversely affect compensatory mitigation success. Any deviations from the approved mitigation plan requires approval from the Corps and MDE.
- B.** The Permittee must include appropriate information in the monitoring reports about performance issues and implementation of approved adaptive management measures to allow the Corps and MDE to assess how the project is progressing. The Permittee must notify the Corps and MDE as soon as possible if the compensatory mitigation project is not achieving its performance standards as anticipated. The Corps and MDE, in coordination with the Permittee and other appropriate agencies, will evaluate any deficiencies and determine if proposed measures will address those deficiencies and/or require modification of the approved mitigation plan(s). The proposed measures must be designed to ensure that the modified compensatory mitigation project provides aquatic resource functions comparable to those described in the mitigation plan objectives. The Permittee shall implement the strategies in the adaptive management plan until the compensatory mitigation project has been determined by the Corps and MDE to have met its goals, objectives, and performance standards and the long-term management plan is initiated.

## **RECOMMENDED METHODS FOR MONITORING VEGETATION, HYDROLOGY, AND SOILS IN WETLAND MITIGATION SITES IN MARYLAND**

Below are the recommended techniques for monitoring mitigation sites. Alternate techniques may be considered, but must be approved in writing by the MDE and the Corps prior to the commencement of the monitoring period.

### Recommended Wetland Vegetation Density Measurement Technique

- a. The following method for measuring the success of the vegetative colonization should be conducted once between May 1 and September 30 for forested/shrub-shrub systems and between June 15 and September 30 for emergent systems during each year requiring submittal of a monitoring report, unless an alternate schedule is agreed upon by MDE and the Corps.
- b. Vegetation sample plots shall be located on a stratified random basis over the site in order to sample all areas of wetlands at locations adjacent to each photo location marker. Plots should be located within each planned and actual vegetative type and hydrologic regime. Plot locations should be determined prior to construction and shown on the mitigation plan. Once the sample plots are approved as part of the mitigation plan, they should be stationary, unless the Permittee recommends, and MDE and the Corps agree to moving the sample plots. Potential justification for moving sample plots may include that the plot location is an outlier or the actual vegetative type/hydrologic regime differs from what was planned, resulting in some representative areas not being monitored. The following minimum numbers of samples will be required:
  - i. If the site is < 5 acres, then a minimum of 3 plots/acre is necessary.
  - ii. If the site is > 5 acres but less than 20 acres, then a minimum of 3 plots/acre is required for the first 5 acres, then 2 plots/acre is required for the remaining acreage.
  - iii. If the site is > 20 acres, then a minimum of 2 plots/acre is required for the first 20 acres, then 1 plot/acre is required for the remaining acreage.
  - iv. All cells, fields, or blocks shall be sampled. A targeted vegetation monitoring approach that correlates monitoring stations with vegetative signatures on aerial photography may be useful for larger mitigation sites.
- c. Each plot shall be of a size no less than 400 square feet for woody plants and 3'x3' for herbaceous plants (or circular with approximately the same surface area). The vegetation data shall be collected during the growing season and shall include:
  - i. Dominant vegetative species identification
  - ii. Percent ground cover assessment
  - iii. Number of woody plant stems greater than 10 inches in height (total and #/acre)
  - iv. The percentage of dominant species FAC or wetter
  - v. Percent survival by planted species
  - vi. A non-native/invasive species assessment including percent cover

### Recommended Buffer Vegetation Density Measurement Technique

- a. The following method for measuring the success of the vegetative colonization should be conducted once between May 1 and September 30 of each year requiring submittal of a monitoring report, unless an alternate schedule is agreed upon by MDE and the Corps.
- b. Vegetation sample plots shall be located on a stratified random basis over the site in order to sample all areas of wetland buffer at locations adjacent to each photo location marker. Plots should be located within each planned and actual vegetative type and hydrologic regime. Plot locations should be

determined prior to construction and shown on the mitigation plan. Once the sample plots are approved as part of the mitigation plan, they should be stationary, unless the Permittee recommends, and MDE and the Corps agree to moving the sample plots. Potential justification for moving sample plots may include that the plot location is an outlier or the actual vegetative type differs from what was planned, resulting in some representative areas not being monitored. The following minimum numbers of samples will be required:

- i. If the site is < 5 acres, then a minimum of 3 plots/acre is necessary.
  - ii. If the site is > 5 acres but less than 20 acres, then a minimum of 3 plots/acre is required for the first 5 acres, then 2 plots/acre is required for the remaining acreage.
  - iii. If the site is > 20 acres, then a minimum of 2 plots/acre is required for the first 20 acres, then 1 plot/acre is required for the remaining acreage.
  - iv. All cells, fields, or blocks shall be sampled. A targeted vegetation monitoring approach that correlates monitoring stations with vegetative signatures on aerial photography may be useful for larger mitigation sites.
- c. Each plot shall be of a size no less than 400 square feet for woody plants (or circular with approximately the same surface area). The vegetation data shall be collected during the growing season and shall include:
- i. Total actual and relative percent cover of native plant species.
  - ii. Number of native woody plant stems greater than 10 inches in height (total and #/acre).
  - iii. A non-native/invasive species assessment including relative percent cover.

#### Recommended Groundwater Well Placement and Data Collection

- a. Determine if this wetland is groundwater fed or has a perched water table. Soil profile descriptions must be assessed prior to well installation to identify any restrictive layers to downward water movement. Wells should be installed so they do not penetrate the restrictive layer, but are instead no deeper than the top of the restrictive layer (as discussed in the 2005 Corps document entitled *Technical Standard for Water-Table Monitoring of Potential Wetland Sites ERDC TN-WRAP-05-02*). In most cases, a standard monitoring well installed to 15 inches below the soil surface should be used. Shallower installation depths should be utilized if restrictive soil depths are located within 15 inches of the soil surface. Well design and installation shall be consistent with current Corps' guidance.
- b. Specific details on the groundwater monitoring wells and locations shall be provided in the mitigation plan, and must be approved by MDE and the Corps.
- c. The following minimum numbers of groundwater wells will generally be required. The Permittee may propose alternate well requirements as part of the mitigation plan, based on justification from the proposed mitigation design:
  - i. If the site is < 10 acres, then a minimum of 1 well/acre is necessary.
  - ii. If the site is 10 to 20 acres, then a minimum of 1 well/acre is necessary for the first 10 acres, then 1 well/2 acres is necessary for the remaining acreage.
  - iii. If the site is > 20 acres, then a minimum of 1 well/acre is necessary for the first 10 acres, 1 well/2 acres is necessary for the next 10 acres, and 1 well/5 acres is necessary for the remaining acreage.
  - iv. Hydrologic zones differentiated by a 1-foot change in elevation should have a minimum of one groundwater monitoring well installed.
  - v. For sites with multiple cells, each cell should have at least one well.
- d. Begin the collection of groundwater well data within fourteen days of the start of the growing season. Take groundwater well readings once every 7 days for the first two months of the growing season and every 30 days for the remainder of the growing season. Record to the nearest inch. Well data should be collected every year during the monitoring period. If well data confirms the presence of wetland hydrology during multiple years of monitoring, the Permittee may request that well data not be required

every year. MDE and the Corps will consider the evidence of hydrology, based on the monitoring reports, site visits, and local precipitation, to approve or deny this request.

- e. The growing season should be based on median dates (i.e., 50 percent probability) of 28°F air temperatures in spring and fall, based on the long-term data for the nearest appropriate weather station, as recorded in the WETS tables available from the NRCS National Water and Climate Center ([https://www.wcc.nrcs.usda.gov/climate/navigate\\_wets.html](https://www.wcc.nrcs.usda.gov/climate/navigate_wets.html)), or as specified in the appropriate regional supplement to the Corps of Engineers Wetland Delineation Manual.
- f. Measure and record any surface water present at the monitoring wells.
- g. Include a copy of the plan showing the location of the wells and surface elevation beside each well. Summarize the information regarding groundwater and surface water elevations, and provide monthly rainfall data for the areas.

#### Indicator of Saturated and Anaerobic Conditions to Demonstrate the Presence of Active Hydric Soil Conditions

- a. The Hydric Soil Technical Standard (HSTS) developed by the National Technical Committee for Hydric Soils (Technical Note 11) requires documentation of anaerobic conditions and saturated conditions for a soil to be considered hydric:
  - i. For a soil to meet the Saturated Conditions part of the HSTS, free water must exist within 10 inches (25 cm) of the ground surface for at least 14 consecutive days; and
  - ii. Anaerobic conditions must exist within 10 inches (25 cm) of the ground surface for at least 14 consecutive days. Anaerobic conditions may be determined by one of the following methods, as detailed in the HSTS:
    - (1) Positive reaction to alpha-alpha-dipyridyl, determined at least weekly.
    - (2) Reduction of iron determined with IRIS devices (tubes or films) installed for 30 days.
    - (3) Measurement of redox potential (Eh) using platinum electrodes, determined at least weekly.

Methods to demonstrate the presence of anaerobic conditions are outlined at ([https://www.nrcs.usda.gov/Internet/FSE\\_DOCUMENTS/nrcs142p2\\_051608.pdf](https://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_051608.pdf)).

- b. If using alpha-alpha dipyridyl to show soil reduction, soils should be measured at least weekly during the growing season, at a depth of six inches. Note that alpha-alpha dipyridyl is also available as paper strips for easier measurement.
- b. Soil testing should be conducted during the time of the growing season anticipated to have the highest amount of soil reduction (often in the early growing season).
- c. Samples should be taken in a representative portion of the mitigation site with similar micro topography, vegetative community, etc., rather than in the lowest/wettest areas. Some samples should also include the areas with higher elevations. Additional tests should be taken for larger sites and sites with higher changes in elevation.
- d. Plot locations shall be determined after baseline hydrology data are collected for two years to select areas that represent various hydroperiods. At least one soil sample plot location should be established for each hydroperiod present at the mitigation site. Soil sample plots shall be located within five feet of the monitoring well, and shall be performed during each monitoring year. Additional soil monitoring plots may need to be established where saturation occurs between 5% and 12.5% of the growing season to provide corroborative evidence that wetland hydrology is present. Additional soil monitoring may also be required if soil monitoring occurs during extremely wet or dry years.
- d. Include a copy of the plan showing the location of the soil data collection, summarize the information, and provide monthly rainfall data for the area.
- e. If soil testing confirms the presence of actively reducing soil conditions during multiple years of monitoring, the Permittee may request that soil testing not be required every year. MDE and the

Corps will consider the evidence of anaerobic soil conditions, based on the monitoring reports, site visits, and local precipitation, to approve or deny this request.

Recommended Method of Indicator of Reduction in Soils (IRIS) Film Placement and Data Collection.

- a. Label Fe-coated films.
- b. Roll one Fe-coated film into 1” clear polycarbonate delivery tube, with Fe-coating facing out.
- c. Create a pilot hole in the soil using a 1” push probe. The hole should be slightly deeper (1-2”) than final depth of film.
- d. Insert rod into the delivery tube, being sure to hook the rod into the hole at the bottom of the film.
- e. Insert the “loaded” delivery tube into the hole until the mark on the tube is at the soil surface (50 cm).
- f. Holding the rod to ensure the film stays in the soil, pull out the delivery tube.
- g. Pull out the rod, being careful not to pull out the film.
- h. Insert foam plug into the top of the film, using two O-rings to secure the film around the plug.
- i. If the films are installed to shallower depths (e.g., gravel layer inhibits full depth for pilot hole), mark the depth of the soil surface on the films with a permanent marker.
- j. Install five replicates, up to a meter apart, within the study area.
- k. Films should be left in place for two to four weeks and then should be removed and replacement films can be installed in the same holes for an additional two to four weeks. Films left in for longer than four weeks cannot be used to meet required performance standards.
- l. Gently wash off any adhering soil from the films.
- m. Estimate the amount of paint removed from each film by overlaying with a mylar grid and marking and counting the grid<sup>11</sup>, or by using some other IRT-approved procedure.
- n. Find a six-inch area on the film, entirely within the upper 12 inches, with the most paint removed. Estimate the percentage of paint removed from this six-inch area and document the depth of this six-inch area.
- o. To meet the Technical Standard for reducing soil conditions as currently specified in the National Technical Committee on Hydric Soils, 30% or more of paint within this six-inch section must be removed.
- p. At least three of the five replicates must show this paint removal for the soil to demonstrate that it is reducing.

Recommended Method of Indicator of Reduction in Soils (IRIS) Tube Placement and Data Collection  
(summarized from the 2008 document entitled *Protocol for Using and Interpreting IRIS Tubes*).

- a. Create a pilot hole in the soil using a 7/8” push probe. The hole should be slightly deeper (1-2”) than final depth of tube.
- b. Be sure tubes are labeled.
- c. Insert the IRIS tube into the hole until the mark on the tube is at the soil surface (50 cm). If they are installed to shallower depths, mark the depth of the soil surface with a permanent marker.
- d. Install five replicates, up to a meter apart, within the study area.
- e. Tubes should be left in place for two to four weeks and then should be removed and replacement tubes can be installed in the same holes for an additional two to four weeks. Tubes left in for longer than four weeks cannot be used to meet required performance standards.
- f. Gently wash off any adhering soil from the tubes.

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<sup>11</sup> Rabenhorst, M.C. 2012. Simple and Reliable Approach for Quantifying IRIS Tube Data. *Soil Sci. Soc. Am. J.* 76: 307-308.

- g. Estimate the amount of paint removed from each tube by wrapping a mylar grid around tube and by marking and counting the grid, or by using some other IRT-accepted procedure.
- h. If visual estimations are used, to improve accuracy, have two (or more) people estimate the amount of paint removed, then average the two sets of data.
- i. Find a six-inch area on the tube, entirely within the upper 12 inches, with the most paint removed. Estimate the percentage of paint removed from this six-inch area and document the depth of this six-inch area.
- j. To meet the Technical Standard for reducing soil conditions as currently specified in the National Technical Committee on Hydric Soils, 30% or more of paint within this six-inch section must be removed.
- k. At least three of the five replicates must show this paint removal for the soil to demonstrate that it is reducing.

#### Recommended Method of Application of the Alpha-Alpha Dipyridyl Paper Test Strips

- a. To meet the anaerobic condition requirement using alpha-alpha dipyridyl test strips, tests should show positive reaction to alpha-alpha dipyridyl at least three times in a row (e.g., sample on Day 1, sample a week later, sample another week later).
- b. Excavate a soil pit to a depth of at least 14-16 inches\*. A fresh slice of the profile should be cut from the side of the pit and laid out for observation and characterization. Apply the test strips to the targeted layer(s) at several locations within the representative area to ensure that the majority of the layer is reduced. Document at what depth the positive reaction(s) to the test occurred. The procedure for problematic soils (Step 4d) discussed in Chapter 5 of the Regional Supplements requires that at least 60% of a layer 4 inches or more thick and located within 12 inches of the surface, react positively from liquid alpha-alpha dipyridyl solution. *\*Note: The depth of soil excavations for profile characterization can be much deeper depending upon the required depth and thickness requirements of some hydric soil indicators.*
- c. It is important that the test strips are applied only to a fresh, broken face of the desired layer(s). Do not add moisture to soil samples or rub soil against or on to the paper, simply press the paper against a fresh, broken ped face on the soil sample(s). Be sure not to test soil samples that have been exposed to digging equipment to prevent false positive reactions. Record all observations of soil moisture, limit of saturation and the depth to water table on a data form and or in your notes.
- d. A positive reaction on the paper (turning pink or red) should occur in a few moments but can take longer especially during colder periods. The manufacturer indicates that the reaction normally takes place within about 30 seconds.
- e. To increase the validity of your findings, test the targeted layers at several different locations within the same representative area and any other layers which meet an indicator.
- f. Testing multiple samples can exhaust your supply quickly but you can double your reserves by cutting the strips in half. Be careful not to use cutting instruments that could contaminate a sample.
- g. The test should be performed as soon as you remove the sample and all information (depths, layers, etc.) recorded in the appropriate fields of the data form (i.e. hydrology remarks, soil layer comments, soil remarks, etc.). Your soil profile description should also be performed as soon as possible using one of the representative pits. In addition to photo documenting your soil profile, document the application of the strips before and after any potential reaction.
- h. If the soil is allowed to dry before implementing the test strips or characterization of the profile, dig another representative pit and start over.





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## **STREAM ECOLOGICAL MONITORING REQUIREMENTS AND PERFORMANCE STANDARDS FOR ECCLESTON MITIGATION SITE**

The submission of monitoring reports will be required for a period of ten years from the completion of the construction of the mitigation site or phase thereof. Monitoring reports should be concise and effectively provide the information necessary to assess the status of the compensatory mitigation project. Reports should provide information necessary, including supporting data such as plans, maps, and photographs, to illustrate site conditions and whether the compensatory mitigation project is meeting its objectives and performance standards. Monitoring reports shall be submitted for years 2, 3, 5, 7, and 10 (“monitoring years”) following completion of construction and planting of the mitigation site or phase thereof. Monitoring reports, both paper copies and an electronic version, must be submitted to the agencies by December 31 of each monitoring year. Monitoring must be conducted a minimum of once per year during the monitoring years following construction of any phase of the project site.

Monitoring may be terminated or the extent of monitoring may be reduced over part or the entire site at the discretion of MDE and the Corps. Conversely, the agencies may extend the original monitoring period upon a determination that performance standards have not been met or the project is not on track to meet them.

If all performance criteria have not been met in the tenth year, then a monitoring report shall be required for each consecutive year until two sequential annual reports indicate that all criteria have been successfully satisfied.

An as-built report/survey will be provided to the agencies within 60 days following completion of construction and planting.

### **Monitoring Report Requirements:**

In addition to the wetland monitoring report requirements, stream monitoring report requirements much detail the following:

**As-Built Report:** The as-built report shall include comparisons of the design plan to the as-built plan, using the following report components:

1. Plan view of the constructed/restored wetlands, streams, habitat features, and adjacent buffers with location of all permanent sampling stations, photo stations, monitoring wells, instream and stream bank structures, and all permanent cross-section and profile locations
2. Photographs of the completed mitigation taken from permanent photo stations. Photos from each station must be grouped with corresponding photos from previous monitoring reports
3. Longitudinal stream profiles taken from permanent locations, and inlaid with and compared to design profiles
4. Cross-sections taken at permanent locations and inlaid with and compared to design cross-sections



5. Pebble counts from each station that are grouped with corresponding pebble counts from previous monitoring reports
6. Summary stream geomorphologic data presented in a side by side analysis for the design, reference, and as-built channels
7. Planting composition, locations, phases, and densities
8. Stream gage locations
9. Site boundaries
10. As-built elevations

### **Monitoring Report Measurements:**

#### **Stream Area(s)**

After Year 2, physical monitoring of stream condition (e.g. Longitudinal profiles, cross-sections, channel Width and Depth) may be conducted outside of the growing season.

Stream chemical (i.e. temperature) monitoring will occur continuously, year-round. For stream biological monitoring, event shall occur consistently within the index period as required by the Maryland Biological Stream Survey (MBSS); between March 1 and April 30 for benthos sampling and between June 1 and September 30 for fish sampling. For any year in which planting was conducted, monitoring of woody vegetation shall take place no earlier than October and at least 6 months following planting. Monitoring of vegetation (herbaceous and woody species) shall be conducted during the growing season.

- a. Stream Channel Preservation: Limits will be visually monitored and documented with photographs on an annual basis.
- b. Stream Restoration: For the linear footage of stream with stream enhancement activities, the following monitoring will occur in addition to those outlined for Stream Preservation areas:
  - i. Reach-wide data will be collected including the following:
    - Bank Erodibility Hazard Index (BEHI) will be assessed to provide a representative reach assessment.
    - Beginning with Year 3, The U.S. Forest Service Stream Reach Inventory and Channel Stability Evaluation (Pfankuch, 1975) will be performed on each reach to provide a representative assessment.
    - Monitoring of the pre- and post-restoration instream habitat using the Rapid Bioassessment Method
    - Radius of curvature shall be assessed within a representative reach.
    - Sinuosity shall be assessed in a representative reach.
    - Percent estimated canopy coverage over restored streams
  - ii. Permanent cross-sections shall be established to ensure that the same locations



are used each monitoring year. Representative cross-sections (with permanent markers established during the first monitoring interval) will be surveyed at intervals on a representative sample of riffles and pools. The total number required will vary depending on project length and complexity. Additional cross-sections may be required to show areas where aggradation, degradation, erosion, and mid-channel bars have developed. The following cross-section data will be collected:

- Bankfull width/depth ratio
  - Bank/height ratio and entrenchment ratio
  - Baseflow discharge measured at least once during both the wet and dry seasons
  - Photographs documenting the structural integrity and function at each habitat structure
  - Documentation of structure use by intended species. Each instream structure shall have the following data collected:
    - Photographs documenting structural integrity and function.
    - Surveyed profile documenting invert elevation
- iii. A surveyed longitudinal profile of the stream within the thalweg with measurements of the locations, depths, and slopes of riffles, runs, pools, and glides. A separate profile will be prepared depicting all previous longitudinal profiles superimposed.
- iv. Stream classification pebble count
- v. Bar sample or pavement sample
- vi. Wetted-perimeter cross-section pebble count of representative riffles
- vii. The D50 analysis of the pebble count data

Chemical and Biological Monitoring: Monitoring events shall occur in years 2, 3, 5, 7, and 10 and consistently in either spring or fall of each monitoring year. Stream chemical (i.e. temperature) monitoring will occur continuously, year-round. For stream biological monitoring, event shall occur consistently within the index period as required by the MBSS; between March 1 and April 30 for benthos sampling and between June 1 and September 30 for fish sampling. The number and location of monitoring stations shall be determined, and approved by the agencies, on a case-specific basis and shall remain consistent throughout the monitoring period. Surveys of other biota (e.g. fish, waterfowl, amphibians, etc.) should occur on a case-by-case basis.

Scientific Collection permits for conducting benthic and fish sampling will be coordinated with the Maryland Department of Natural Resources (DNR). All field sampling as well as laboratory sample processing shall be performed by or under supervision of a professional aquatic biologist.

- i. Chemistry – Temperature shall be collected at each designated monitoring location site using a HOBO data logger.
- ii. Biological – A quantitative survey for benthic macroinvertebrates and fish habitat assessment shall be conducted at designated monitoring locations using a modified MBSS methodology.

Habitat Assessment: A habitat assessment shall be conducted at each identified site using the Rapid Bioassessment methodology.



## **Performance Standards**

All required documentation, including monitoring reports, semi-annual ledgers, and as-built surveys shall be submitted to Maryland Department of the Environment (MDE) and the Army Corps of Engineers (Corps). MDE and the Corps will use best professional judgment, visual observation, and monitoring reports to evaluate attainment of performance standards and in determining whether part of or the entire Project is successful or whether corrective actions are warranted. Success will be determined on a plot, well, field, cell, area, or reach basis. Presenting averages or means of plot data across a project site is not satisfactory to demonstrate success. All of the following standards will be used to assess project success and credit releases and must be achieved each monitoring year.

The project shall conform to the following performance standards by the end of the monitoring period, unless otherwise determined by MDE and the Corps.

Performance Standards and success criteria have been put into table format to easily determine if performance standards have been met. Tables 1 and 2 can be found below and should be used to determine if performance standards have been met for each monitoring year.

## **Stream Area(s)**

The overall objective for the stream compensation is to ensure that the dimension, pattern, and profile of the stream enhancement and restoration areas: 1) remain within the natural range of variability present in the reference data obtained for the design; 2) remain stable; 3) exhibit appropriate habitat diversity; 4) have healthy viable riparian buffers; and 5) improve biological communities.

## **Stream Restoration/Enhancement Areas**

For the linear footage of stream with stream restoration activities, the following Performance Standards will apply:

- a. **Dimension:** The analysis of representative riffle cross-sections shall indicate that they have not aggraded, degraded, widened, or narrowed to the point where they have become unstable or will cause instability. The following measurements will be used to aid in making this determination each monitoring year:
  - i. The Width/Depth Ratio Stability Rating (measured Width/Depth Ratio divided by the approved as-built Width/Depth Ratio) shall not be greater than 1.3. If the channel is incising, then the Width/Depth Ratio Stability Rating shall not be less than 0.7.
  - ii. The Bank Height Ratio shall not increase or decrease by an amount greater than 0.2 of the approved as-built Bank Height Ratio.
- a. **Stream Reach Stability:** The analysis of the streambank from the top of the bank to the ordinary high-water mark presence of natural protection to prevent streambank erosion that could jeopardize the stability of the streambank or the stream reach. The following measurements will be used to aid in making this determination each monitoring year:
  - i. The individual Index Values of Bank Erodibility Hazard Index (BEHI) rating for any identified reach shall be equal to or less than the previous year's Index Value. In

- addition, the Total Score shall be equal to or less than the previous year's Total Score, and shall have a Total Score of "Moderate" by monitoring Year 3, and a Total Score of "Low" by monitoring Year 5, and maintained at "Low" throughout the remainder of the monitoring period.
- ii. The U.S Forest Service Stream Reach Inventory and Channel Stability Evaluation (Pfankuch, 1975) rating shall be "Good" each monitoring year, beginning with Year 3.
- b. **Pattern:** The analysis of the plan-view survey or field measurements shall indicate that the stream is not migrating to the point where it will cause bank erosion and instability. The following criteria will be used to aid in making this determination each monitoring year:
- i. The sinuosity of the stream does not increase or decrease by an amount greater than 0.1 of the approved as-built pattern.
  - ii. The thalweg of each channel cross-section does not move by more than 10% of the width of the approved as-built channel width in any given year.
  - iii. The radius of Curvature/Bankfull Width Ratio does not increase or decrease by an amount greater than 0.2 of the as-built condition. For instance, if the as-built ratio is 3.0, the acceptable ratio shall be 2.8 to 3.2 to remain within the range of variability present in the reference data.
- c. **Structures:** The analysis of each instream structure shall indicate that it is performing its intended function, and not adversely affecting the stream. The following measurements will be used to aid in making this determination each monitoring year:
- i. Absence of significant under cutting, washing around, or erosion of the bank or streambed associated with any instream structure, excluding any minor channel scour within the thalweg immediately downstream of a structure caused by its intended redirection of flow.
  - ii. The invert elevation (controlling elevation) of the header rocks or logs of any vane, j-hook, cross-vane, W-weir, or other structure shall not vary more than 0.2 feet from the approved as-built.
- d. **Materials:** The analysis of the pebble count data shall not show a significant change in streambed materials to the point that indicates a shift in bedload material due to stream instability. The following measurement will be used to aid in making this determination each monitoring year:
- i. The D50 size particle remains within its approved as-built size class (silt, sand, gravel, cobble, or boulder).
- e. **Profile:** The analysis of the longitudinal profile shall indicate that the bed elevation has neither aggraded nor degraded to the point where it will cause instability. The following criteria will be used to aid in making this determination each monitoring year:
- i. The analysis of the Longitudinal Profile does not indicate significant alterations in the locations, depths, and slopes of stream features (riffle, run, pool, and glide).

### **Functional Uplift Goals**

The overall project should meet functional uplift goals. These are directly and indirectly related to the wetland and stream performance standards. The corresponding Functional Uplift Goals

table has anticipated goals and actual goals for each monitoring year. MDE and the Corps will use best professional judgment, visual observations and monitoring reports to evaluate attainment of wetland and stream functional uplift goals and whether corrective actions are warranted.

- 1. Increase Stream Sinuosity**
  - a. Sinuosity
- 2. Reconnect Stream to Basal Gravel Layer**
  - a. Pebble Counts (D50)
- 3. Reconnect Streams to Floodplain**
  - a. Bank/ height ratio
  - b. Entrenchment ratio
- 4. Reconnect Streams to Floodplain, Increase Flood flow Attenuation**
  - a. Frequency of flooding (modeled with corroborating visual observations)
- 5. Increase Flood flow Attenuation**
  - a. Area of floodplain
- 6. Reconnect Streams to Floodplain, Increase Flood flow Attenuation, Remove Wetland Drains, Improve Habitat**
  - a. Acreage of floodplain wetlands
- 7. Restore Diverted Stream Baseflow**
  - a. Baseflow discharge and physical destruction of pipe
- 8. Increase Upland Riparian Buffer, Improve Upland Habitat**
  - a. Acreage of upland riparian buffer
- 9. Improve In-Stream Habitat**
  - a. Bed diversity
  - b. Riffle habitat area
  - c. Pool habitat area
  - d. LF of stream
  - e. Channel overhead cover area/ LWD frequency
- 10. Invasive Species Controls**
  - a. Decrease in invasive species relative cover
- 11. Improve Aquatic Organism Passage**
  - a. Remove Dam(s)
- 12. Decrease Stream Bank Erosion**
  - a. BEHI score
- 13. Maintain or Improve Fauna Presence**
  - a. Trout (presence, absence, age classes)
  - b. Macroinvertebrates (B-IBI scores, taxa diversity)

**Table 1: Eccleston Stream Performance Standards**

Eccleston Stream Performance Standards - Restoration											
Parameters	Measurement	Year 2 Monitoring		Year 3 Monitoring		Year 5 Monitoring		Year 7 Monitoring		Year 10 Monitoring	
		Antic.	Actual	Antic.	Actual	Antic.	Actual	Antic.	Actual	Antic.	Actual
Dimension	Width/depth ratio stability rating (measured w/d divided by as-built w/d)	≥ 0.7, ≤ 1.3		≥ 0.7, ≤ 1.3		≥ 0.7, ≤ 1.3		≥ 0.7, ≤ 1.3		≥ 0.7, ≤ 1.3	
	Bank height ratio	≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built	
Stability	BEHI (Index value for any identified reach)	Not applicable		≤ Previous year's index value		≤ Previous year's index value		≤ Previous year's index value		≤ Previous year's index value	
	BEHI (Total score)	Not applicable		≤ Previous year's total score; Moderate		≤ Previous year's total score; Low		≤ Previous year's total score; Low		≤ Previous year's total score; Low	
	USFS Stream Reach Inventory and Channel Stability Evaluation	Not applicable		Good		Good		Good		Good	
Pattern	Sinuosity	≤ 0.1 ± as-built		≤ 0.1 ± as-built		≤ 0.1 ± as-built		≤ 0.1 ± as-built		≤ 0.1 ± as-built	
	Thalweg movement of each channel cross-section	≤ 10% width of approved as-built channel per year		≤ 10% width of approved as-built channel per year		≤ 10% width of approved as-built channel per year		≤ 10% width of approved as-built channel per year		≤ 10% width of approved as-built channel per year	
	Radius of curvature/bankfull width ratio	≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built		≤ 0.2 ± as-built	
Structures	"Significant undercutting/erosion of bank or streambed associated with structure "	Absent		Absent		Absent		Absent		Absent	
	Invert elevation of header rocks/logs of structure (eg vane, j-hook)	≤ 0.2 ft ± as-built		≤ 0.2 ft ± as-built		≤ 0.2 ft ± as-built		≤ 0.2 ft ± as-built		≤ 0.2 ft ± as-built	
Materials	D50	Remains within approved as-built size class		Remains within approved as-built size class		Remains within approved as-built size class		Remains within approved as-built size class		Remains within approved as-built size class	
Profile	Significant alterations in locations, depths, and slopes of stream features	Absent		Absent		Absent		Absent		Absent	



**Table 2: Eccleston Functional Uplift Goals**

Functional Uplift Goals*												
Functional Uplift/Impairment Reduction Goal	Measurement	Baseline	Year 2 Monitoring		Year 3 Monitoring		Year 5 Monitoring		Year 7 Monitoring		Year 10 Monitoring	
			Antic.	Actual.	Antic.	Actual.	Antic.	Actual.	Antic.	Actual.	Antic.	Actual.
Increase stream sinuosity	Sinuosity											
Reconnect streams to basal gravel layer	Pebble Counts (D50)											
Reconnect streams to floodplain	Bank/height ratio											
	Entrenchment ratio											
Reconnect streams to floodplain, increase floodflow attenuation	Frequency of flooding (modeled w/ corroborating observations)											
Increase floodflow attenuation	Area of floodplain											
Reconnect streams to floodplain, increase floodflow attenuation, remove wetland drains, improve habitat	Acreage of floodplain wetlands											
Restore diverted stream baseflow	Baseflow discharge and physical destruction of pipe											
Increase upland riparian buffer, improve upland habitat	Acreage of upland riparian buffer											
Improve in-stream habitat	Bed diversity											
	Riffle habitat area											
	Pool habitat area											
	LF of stream											
Channel overhead cover area / LWD frequency												
Invasive species control	Decrease in invasive species relative cover											
Improve aquatic organism passage	Remove Dam											
Decrease stream bank erosion	BEHI score											
Maintain or improve fauna presence	Trout (presence, absence, age classes)											
	Macroinvertebrates (B-IBI scores, taxa diversity)											

\* Anticipated quantitative goals for each measurement type are TBD.





# APPENDIX M LONG-TERM MANAGEMENT PLAN

**LONG-TERM MANAGEMENT PLAN**

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1. Introduction

A. Purpose of Establishment

The Eccleston Mitigation Project (Site) was established to compensate for unavoidable impacts to, and to conserve and to protect, waters of the U.S. The site property includes preserved wetlands enhanced wetlands, created or restored wetlands, out-of-kind stream mitigation, created or restored stream channel, preserved stream channel, enhanced buffer, and preserved buffer. Table 1 – Site Resources includes the total acreage and linear footage per resource type requiring long-term management. The Baltimore District of the U.S. Army Corps of Engineers and the Maryland Department of the Environment (MDE) constitute the regulatory agencies with authority over this mitigation site, and will be consulted concerning site adjustments. Terms used in this management plan have the same meaning as defined in the Compensatory Mitigation Plan (CMP).

**Table 1 - Site Resources**

<b>Wetlands</b>	
Preserved	5.68 acres
Enhanced	3.09 acres
Restored	13.08 acres
<b>Total</b>	<b>21.85 acres</b>
<b>Streams</b>	
Preserved	1,437 linear feet
Restored	7,486 linear feet
Created	1,118 linear feet
<b>Total</b>	<b>10,041 linear feet</b>
<b>Buffer</b>	
Preserved	8.61 acres
Enhanced	18.29 acres
<b>Total</b>	<b>26.90 acres</b>

B. Purpose of this Long-term Management Plan

The purpose of this long-term management plan is to ensure the Site is managed, monitored, and maintained in perpetuity. This management plan establishes objectives, priorities and tasks to monitor, manage, maintain and report on the waters of the U.S., covered species and covered habitat on the Site. This management plan is a binding and enforceable instrument, implemented in accordance with the CMP and the real estate protection instrument (conservation easement or declaration of restrictions) covering the Site property.

C. Long-Term Steward and Responsibilities

The Long-Term Steward is North American Land Trust. The Long-Term Steward, and subsequent Long-Term Stewards upon transfer, shall implement this long-term management plan, managing and monitoring the site property in perpetuity to preserve its habitat and conservation values in accordance with the Site’s plan and conservation easement and/or declaration of restrictions, and the long-term management plan. Long-term management tasks shall be funded through the Long-Term



Management Fund. The Long-Term Steward must maintain a copy of the final plan including all deed restrictions and easements. The Long-Term Steward shall be responsible for providing an annual report to the regulatory agencies detailing the time period covered, an itemized account of the management tasks and total amount expended. Any subsequent grading, or alteration of the site's hydrology and/or topography by the Long-Term Steward or its representatives must be approved by the regulatory agencies and the necessary permits, such as a Section 404 permit and/or Maryland Nontidal Wetlands Permit, must be obtained if required.

#### D. Eminent Domain

If the site is taken in whole or in part through eminent domain, the Long-Term Steward shall use all monies received as compensation for lands and all associated services and values taken to provide replacement compensation within the same service area, subject to regulatory agency approval. The agencies shall have the right to participate in any proceeding associated with the determination of the amount of such compensation. Replacement compensation may be determined in consultation with the regulatory agencies.

## 2. Property Description

#### A. Setting and Location

The Site is located at the southwest corner of Park Heights Avenue and Greenspring Valley Road in Baltimore County, in the State of Maryland, designated as Parcel No. 154. The Property is shown on the Vicinity and Location Map (Exhibit A) of the Mitigation Site. Exhibit A shows the Site location in relation to cities, towns, or major roads, and other distinguishable landmarks. The Site easement plat is included as **Appendix A** to this plan.

#### B. History and Land Use

Publicly available historic aerials have been researched for this project site, including 1927 and 1953 photography. Previous land use for the site was high-density agricultural, with dairy cows and livestock grazing. Agriculture has shifted to grain farming with no livestock component. Current surrounding land uses include agricultural and low-density residential. The project site and uses are well-depicted in these data and demonstrate that the Jones Falls was straightened multiple times and was almost completely deforested approximately 60 years ago. Other evidence of drain tile, wetland drainage, dams, stream diversion and piping, and construction activities is noted in these photographs. Grazing and land clearing are evident in these aerials, supporting the documentation that the site was used for a dairy operation.

#### C. Cultural Resources

No known protected historic, archaeological, or cultural resources are present on the site within the proposed work areas. Per standard protocols, if the discovery of resources on the site is made, MDE/MHT will immediately be contacted. No additional studies are proposed at this time.

#### D. Hydrology and Topography

Investigation of the site has shown that the lowland floodplain areas of the site were most likely

Piedmont emergent/scrub shrub/forested mosaic wetlands highly connected to a basal gravel layer within the valley. A buried hydric soil layer is present, indicating persistent groundwater at that location. This was the dominant, climax hydrological surface/groundwater regime until European settlement, deforestation, and impoundment of Jones Falls. Groundwater historically would have been at or near the ground surface, and is impacted now by channel straightening, legacy sediment, and drain tile, resulting in sporadic wetland conditions adjacent to the stream. Groundwater has been characterized through observation of redoximorphic features observed during the basal gravel investigation. Basal gravel at the site along with the buried hydric soil layer above it is known to be in the ground water table seasonal range based on soil indicators. Streams classify as perennial B4 or C4 Rosgen streams.

#### E. Soils

Existing soils on the site and specific to the Site include MmA, QM, and WhA. MmA (Merrimac fine sandy loam) has slopes of 0 to 3 percent and is commonly found in major stream valleys.

#### F. Existing Easements

Current zoning of the property and proposed Site lands is RC-2. Surrounding lands of the proposed Site are also zoned RC-2. A title report was obtained, and additional title research has been conducted for the property revealing that no encumbrances, liens, or easements noted affect the Site. The edges of the property contact road, utility, and railroad right of ways that will not impact or affect the proposed Site. No known mortgages, liens, rights-of-ways, servitudes, easements, mineral rights, etc., other than those previously stated, are known on the property.

#### G. Adjacent Land Uses

Current surrounding land uses include agricultural and low density residential. Though previous agricultural usage included dairy cows and livestock grazing, current agriculture is strictly grain farming.

### 3. Habitat and Species Descriptions

#### A. Baseline Description of Biological Resources on Site

The Rapid Bioassessment Protocol was used to assess the baseline for each stream resource. MBSS Protocols have also been conducted to assess the baseline for macroinvertebrates and fish species within the project limits.

Existing site wetlands functions and values have been assessed using New England Highway methodology.

Baseline invasive plant species and cover percentages have been assessed and documented at the site.

#### B. Summary of Site Development Plan

Mitigation Credit Map and/or As-built drawings are included as **Appendix B** to this plan.

### C. Endangered and Threatened Species

There are no State or Federally Endangered or Threatened Species that currently exist on the Site; however, any species that may eventually occupy the site will be identified and documented on a yearly basis. Elements A.2 and A.3 below may need to be updated should a new species be identified on the site after construction/during the post-construction monitoring period.

### D. Rare Species and Species of Special Concern

There are no State or Federally Rare or Species of Special Concern that currently exist on the Site; however, any species that may eventually occupy the site will be identified and documented on a yearly basis. Elements A.2 and A.3 below may need to be updated should a new species be identified on the site after construction/during the post-construction monitoring period.

## 4. Management and Monitoring

The overall goal of long-term management is to foster the long-term viability of the site's waters of the U.S., and any listed species/habitat. Routine monitoring and minor maintenance tasks are intended to assure the viability of the site in perpetuity.

### A. Biological Resources

The approach to the long-term management of the site's biological resources is to conduct annual site examinations and monitoring of selected characteristics to determine stability and ongoing trends of the preserved, restored, enhancement, and created waters of the U.S., including wetlands and streams. Annual monitoring will assess the Site's condition, degree of erosion, establishment of invasive or non-native species, water quality, fire hazard, and/or other aspects that may warrant management actions. While it is not anticipated that major management actions will be needed, an objective of this long-term management plan is to conduct monitoring to identify any issues that arise and using adaptive management to determine what actions might be appropriate. Those chosen to accomplish monitoring responsibilities will have the knowledge, training, and experience to accomplish monitoring responsibilities.

Adaptive management means an approach to natural resource management which incorporates changes to management practices, including corrective actions as determined to be appropriate by the regulatory agencies in discussion with the Long-Term Steward. Adaptive management includes those activities necessary to address the effects of climate change, fire, flood, or other natural events. Before considering any adaptive management changes to the long-term management plan, the regulatory agencies will consider whether such actions will help ensure the continued viability of Site's biological resources.

The Long-Term Steward for the site shall implement the following:

#### **Element A. 1 Waters of the U.S., including wetlands**

**Objective:** Monitor, conserve and maintain the site's waters of the U.S., including wetlands and streams. Limit any impacts to waters of the U.S. from vehicular travel or other adverse impacts.

**Task:** At least one annual walk-through survey will be conducted to qualitatively monitor the

general condition of these habitats. General topographic conditions, hydrology, general vegetation cover and composition, invasive species, and erosion will be noted, evaluated, and mapped during a site examination. Notes to be made will include observations of species encountered, water quality, general extent of wetlands and streams, and any occurrences of erosion, structure failure, or invasive or non-native species establishment.

Task: Establish reference sites for photographs and prepare a site map showing the reference sites for the Site file. Alternatively, utilize photographic reference sites, if any, developed during the active and interim site management period. Reference photographs will be taken of the overall site at least every five years from the beginning of the long-term management, with selected reference photos taken on the ground more frequently, one time per year (*if applicable*).

Special attention should be paid to any area adjacent to or draining from non-site lands. Streams and wetlands should be observed near site boundaries to observe if increased sediment deposition has occurred. The report should provide a discussion of any recent changes in the watershed (i.e., subdivision being developed upstream of stream site).

**Element A. 2 Threatened/Endangered Plant Species Monitoring (*if applicable*)**

Objective: Monitor population status and trends.

Objective: Manage to maintain habitat for *Not Applicable (N/A)*

Task: Monitor status every year by conducting population assessment surveys. The annual survey dates will be selected during the appropriate period as identified by the regulatory agencies and will generally occur from     N/A     through     N/A     each year. Occupied habitat will be mapped and numbered to allow repeatable data collection over subsequent survey years. Abundance will be assessed semi-quantitatively using broad abundance categories, i.e., 0, 1 - 100, 101 - 500, 501 - 1,000, and >1,000 plants.

Task: Visually observe for changes to occupied habitat, such as changed hydrology or vegetation composition. Record any observed changes. Size of population (1 acre, etc).

Task: Implement other tasks that enhance or monitor habitat characteristics     N/A    .

**Element A. 3 Threatened/Endangered Animal Species Monitoring (*if applicable*)**

Objective: Monitor population status and trends.

Objective: Manage to maintain habitat for     N/A    .

Task: Monitor status every year by conducting population assessment surveys. [*The annual survey dates will be selected during the appropriate period each year.*]

Task: Implement other tasks that enhance or monitor habitat characteristics for     N/A    .

**Element A. 4 Invasive Species**

Invasive species threaten the diversity or abundance of native species through competition for



resources, predation, parasitism, interbreeding with native populations, transmitting diseases, or causing physical or chemical changes to the invaded habitat.

Objective: Monitor and maintain control over invasive species that diminish site quality for which the site was established. The Long-Term Steward shall consult the Maryland Department of Natural Resources at [dnr.maryland.gov](http://dnr.maryland.gov) for guidance on what species may threaten the site and on management of those species.

Task: Mapping of invasive species cover or presence shall occur before the site is constructed to establish a baseline. During the active monitoring period, performance standards include an invasive species cover component. Maps and other data will be available that will detail where invasive species presently occur. If a map is not available, mapping shall be accomplished through use of available technologies, such as GIS and aerial photography.

Task: Each year's annual walk-through survey (or a supplemental survey) will include a qualitative assessment (e.g. visual estimate of cover) of invasive species. Additional actions to control invasive species will be evaluated and prioritized in coordination with the regulatory agencies

#### **Element A. 5 Vegetation Management**

Objective: Analyze effects of any authorized silvicultural manipulations on the wetland, streams, and buffers on the site. If determined appropriate, develop and implement specific silvicultural manipulations (e.g. selective thinning) in coordination with the regulatory agencies. (Site specific targets for vegetation may be specified here and task revised or added to achieve those targets.)

Objective: Adaptively manage vegetation based on site conditions and data acquired through monitoring to maintain biological values.

Task: Review and explore potential vegetation management regimes as proposals and/or opportunities and funding arise. If determined to potentially maintain site quality, develop specific silvicultural practices, amend this long-term management plan with the agencies' approval to reflect those practices, and implement silvicultural actions as funding allows.

Task: Implement vegetation management techniques, if determined beneficial and as funding allows, to allow development of vegetation as identified in the CMP. Implementation of vegetation management techniques must be approved by the regulatory agencies.

#### **B. Security, Safety, and Public Access**

The Site will be fenced or appropriately marked and shall have no general public access, nor any regular public use. Research and/or other educational programs or efforts, hunting, fishing, birdwatching and passive recreational activities may be allowed on the site as deemed appropriate by the regulatory agencies, but are not specifically funded or a part of this long-term management plan.

Potential mosquito abatement issues will be addressed through the development of a plan by the Long-Term Steward and any local mosquito control district or local health department in coordination



with and approved by the regulatory agencies.

Potential wildfire fuels will be reduced as needed where approved by the regulatory agencies.

### **Element B.1 Trash and trespass**

Objective: Monitor sources of trash and trespass.

Objective: Collect and remove trash, repair vandalized structures, and rectify trespass impacts.

Task: During each site visit, record occurrences of trash and/or trespass. Record type, location, and management mitigation recommendations to avoid, minimize, or rectify a trash and/or trespass impact.

Task: At least once yearly collect and remove as much trash as possible and repair and rectify vandalism and trespass impacts.

### **Element B.2 Fire Hazard Reduction**

Objective: Maintain the site as required for fire control while limiting impacts to biological values.

Task: Reduce vegetation in any areas recommended by authorities, and as approved by the agencies, for fire control.

## **C. Infrastructure and Facilities**

*[Fence and gate maintenance and repair frequency will be dependent on trespass and access control issues, as well as whether grazing is utilized as a vegetation management technique and to what extent.]*

### **Element C.1 Fences, Gates, Signage, Crossings, and Property Boundaries**

Objective: Monitor condition of fences, gates, signage, crossings, and property boundaries.

Objective: Maintain fences, gates, signage, crossings and property boundaries to prevent casual trespass, allow necessary access, and *[if applicable: facilitate management.]*

Task: During each site visit, record condition of fences, gates, signs, crossings, and property boundaries. Record location, type, and recommendations to implement repair or replacement to fence, gate, signage, crossings or property boundary markers, if applicable.

Task: Maintain fences, gates, signs, crossings, and property boundary markers as necessary by replacing posts, wire, gates, and signs. Replace fences and/or gates, as necessary, and as funding allows. Note any trespass by livestock.

### **Element C.2 Berms, Structures, and Roads**

Objective: Monitor condition of berms, structures, and roads.

Objective: Maintain berms, structures, and roads to facilitate management and maintain conditions of wetlands and streams.

Task: During each site visit, record condition of berms, structures, and roads. Record location, type, and recommendations to implement repair or replacement to berms, structures, and roads, if applicable.

Task: Maintain berms, structures, and roads as necessary. Replace berms, structures, and roads as necessary, and as funding allows.

#### D. Reporting and Administration

##### **Element D. 1 Annual Report**

Objective: Provide annual report on all management tasks conducted and general site conditions to the regulatory agencies and any other appropriate parties. Each report shall include a cover page with the following information: the site name; Long-Term Steward (name, address, phone number, and email address); monitoring year; and any requested action (e.g. funding release, maintenance recommendations requiring agency approval).

Task: Prepare annual report and any other additional documentation. Include a summary. Complete and circulate to the regulatory agencies and other parties by December 31 of each year. Reports should be distributed electronically.

Task: Make recommendations with regard to (1) any enhancement measures deemed to be warranted; (2) any problems that need near-, short-, and long-term attention (e.g., weed removal, fence repair, erosion control); and (3) any changes in the monitoring or management program that appear to be warranted based on monitoring results to date. Provide documentation of the cost of any recommended maintenance and repairs.

## **5. Transfer, Replacement, Amendments, and Notices**

### A. Transfer

Any subsequent transfer of responsibilities under this long-term management plan to a different Long-Term Steward shall be requested by the Long-Term Steward in writing to the regulatory agencies, shall require written approval by the agencies, and shall be incorporated into this long-term management plan by amendment.

The Long-Term Steward shall be required to ensure that any subsequent property owners (if not identified as the long-term steward) are notified of the deed restriction, conservation easement,

purpose and location of the site lands, and requirement for long-term stewardship.

#### B. Replacement

If the Long-Term Steward fails to implement the tasks described in this long-term management plan and is notified of such failure in writing by any of the regulatory agencies, the Long-Term Steward shall have 90 days to cure such failure. If failure is not cured within 90 days, the Long-Term Steward may request a meeting with the regulatory agencies to resolve the failure. Such meeting shall occur within 30 days or a longer period if approved by the regulatory agencies. Based on the outcome of the meeting, or if no meeting is requested, the regulatory agencies may designate a replacement Long-Term Steward in writing by amendment of this long-term management plan. If the Long-Term Steward fails to designate a replacement Long-Term Steward, then such public or private land or resource management organization acceptable to and as directed by the regulatory agencies may enter onto the Site property in order to fulfill the purposes of this long-term management plan.

#### C. Amendments

The Long-Term Steward, property owner, and the regulatory agencies may meet and confer from time to time, upon the request of any one of them, to revise the long-term management plan to better meet management objectives and preserve the conservation values of the Site property. Any proposed changes to the long-term management plan shall be discussed with the regulatory agencies and the Long-Term Steward. Any proposed changes will be designed with input from all parties. Amendments to the long-term management plan shall be approved by the regulatory agencies in writing shall be required management components and shall be implemented by the Long-Term Steward.

If the MD DNR or United States Fish and Wildlife Service (USFWS) determine, in writing, that continued implementation of the long-term management plan would jeopardize the continued existence of a state or federally listed species, any written amendment to this long-term management plan, determined by either the MD DNR or USFWS as necessary, shall be a required management component and shall be implemented by the Long-Term Steward.

#### D. Notices

Any notices regarding this long-term management plan shall be directed as follows:

Long-Term Steward: **North American Land Trust**  
 100 Hickory Hill Road  
 PO Box 467  
 Chadds Ford, PA 19317  
 610-388-3670 (phone)  
 610-388-3673 (fax)

Property Owner: **Eccleston Land Company**  
 112 Castlewood Road  
 Baltimore, MD 21210  
 410-435-2664 (phone)

USACE:	<b>U.S. Army Corps of Engineers, Baltimore District</b> 2 Hopkins Plaza Baltimore, MD 21201 1-800-434-0988 (phone)
MDE:	<b>Maryland Department of the Environment</b> 1800 Washington Boulevard Baltimore, MD 21230 410-537-3000 (phone)

## 6. Funding and Task Prioritization

### A. Funding

The Long-Term Management Fund Derivation table included with this plan summarizes the anticipated costs of long-term management for the Site. These costs include estimates of time and funding needed to conduct the basic monitoring site visits and reporting, trash removal, fence repair, etc. and a prorated calculation of funding needed to fully repair and/or replace fences and other structures every year. The total annual funding anticipated is approximately \$ 2,508.00, therefore, with the current annual estimated capitalization rate of 4% the total endowment amount (the Long-Term Management Fund) required will be \$ 62,700.00. A detailed breakdown of the cost can be found in **Appendix C**.

North American Land Trust shall hold the endowment principal and interest monies (the Long-Term Management Fund) as required in the CMP, which consists of monies that are paid into it in trust, and is appropriated to fulfill the purposes for which payments into it are made. These interest monies will fund the long-term management, enhancement, and monitoring activities on Site lands in a manner consistent with this long-term management plan.

### B. Task Prioritization

Due to unforeseen circumstances, prioritization of tasks, including tasks resulting from new requirements, may be necessary if insufficient funding is available to accomplish all tasks. The Long-Term Steward and the regulatory agencies shall discuss task priorities and funding availability to determine which tasks will be implemented. In general, tasks are prioritized in this order: 1) required by a local, state, or federal agency; 2) tasks necessary to maintain or remediate the Site (including unauthorized impacts); and 3) tasks that monitor resources, particularly if past monitoring has not shown downward trends. Equipment and materials necessary to implement priority tasks will also be considered priorities. Final determination of task priorities in any given year of insufficient funding will be determined in consultation with the regulatory agencies and as authorized by the regulatory agencies in writing.

### C. Enforcement

The regulatory agencies and its authorized agents shall have the right to inspect the Property and take actions necessary to verify compliance with this Long-Term Management Plan. The Long-Term Management Plan herein shall be enforceable by any proceeding at law or in equity or administrative



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proceeding by the regulatory agencies. Failure by any agency (or owner) to enforce the Long-Term Management Plan contained herein shall in no event be deemed a waiver of the right to do so thereafter.

